

# **Appendix D-1 Traffic Impact Analysis**

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## MEMORANDUM

To: Ara Mihranian, AICP JN 10104089

From: Paul Martin – RBF Consulting

Date: January 19, 2010

Subject: Revised Marymount College Project Traffic & Parking Analysis

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RBF prepared the *Marymount College Facilities Expansion Project Traffic & Parking Impact Analysis (September 28, 2007)*, and the *Revised Marymount College Project Traffic & Parking Analysis (May 15, 2009)*, which were utilized for environmental review of the proposed project. This technical memorandum summarizes traffic and parking operations associated with the designation of up to 250 students from an Associates of Arts degree program (AA Program) to a Bachelor of Arts degree program (BA Program). For purposes of this analysis, the AA Program and BA Program are defined as follows:

- **AA Program** – For traffic purposes defined as educational programs under the Junior/Community College land use, which usually last about two-years, but may vary depending on the student; and
- **BA Program** – For traffic purposes defined as educational programs under the University land use, which usually last about four-years, but may vary depending on the student.

The *Revised Marymount College Project Traffic & Parking Analysis (May 15, 2009)*, included analysis for the following modifications:

- Removal of residence halls;
- Revised number of existing student seats from 578 to 648;
- Reduce net new seats from 131 to 5 seats;
- Reduce new employees/faculty from 12 to 7 staff; and
- Increase weekend student enrollment from 83 to 150 students.

The traffic and parking operations associated with the current project modifications are analyzed to determine if impacts are greater, the same, or less than those identified in the *Marymount College Facilities Expansion Project Traffic & Parking Impact Analysis (September 28, 2007)*, and the *Revised Marymount College Project Traffic & Parking Analysis (May 15, 2009)*.

## 1. TRAFFIC IMPACT ANALYSIS

### Analysis Methodology

Level of service (LOS) is commonly used as a qualitative description of intersection operation and is based on the capacity of the intersection and the volume of traffic using the intersection. The *Intersection Capacity Utilization (ICU)* analysis methodology is utilized in this study to determine the operating LOS of the signalized study intersections; the *2000 Highway Capacity Manual (HCM)* analysis methodology is utilized to determine the operating LOS of the unsignalized study intersections. Intersection LOS calculations are determined using the Traffix software except at the Miraleste Drive/Via Colinita intersection which is evaluated using the Highway Capacity Software (HCS). HCS is utilized at the Miraleste Drive/Via Colinita intersection to take into account the large median and effective refuge area when crossing Miraleste Drive from Via Colinita known as a Two-stage Gap Acceptance.

The ICU analysis methodology describes the operation of a signalized intersection using a range of LOS from LOS A (free-flow conditions) to LOS F (severely congested conditions), based on corresponding Volume/Capacity (V/C) ratios shown in Table 1.

**Table 1**  
**LOS & V/C Ratio Ranges**  
**Signalized Intersections**

LOS	V/C Ratio (2 decimals)	V/C Ratio (3 decimals)
A	$\leq 0.60$	$\leq 0.600$
B	$\geq 0.61 \leq 0.70$	$\geq 0.601 \leq 0.700$
C	$\geq 0.71 \leq 0.80$	$\geq 0.701 \leq 0.800$
D	$\geq 0.81 \leq 0.90$	$\geq 0.801 \leq 0.900$
E	$\geq 0.91 \leq 1.00$	$\geq 0.901 \leq 1.000$
F	$> 1.00$	$> 1.000$

Source: 1990 Transportation Research Board

The *2000 Highway Capacity Manual (HCM)* Operational Analysis Methodology describes the operation of an unsignalized intersection using a range of LOS from LOS A (free-flow conditions) to LOS F (severely congested conditions), based on delay experienced per vehicle as shown in Table 2.

**Table 2  
LOS & Delay Ranges  
Unsignalized Intersections**

LOS	Delay (seconds)
A	≤ 10.0
B	≥ 10.1 ≤ 15.0
C	≥ 15.1 ≤ 25.0
D	≥ 25.1 ≤ 35.0
E	≥ 35.1 ≤ 50.0
F	> 50.0

**Source:** 2000 Highway Capacity Manual

Level of service is based on the average stopped delay per vehicle for all movements of all-way stop-controlled unsignalized intersections; for one-way or two-way stop-controlled intersections, LOS is based on the worst stop-controlled approach.

**Thresholds of Significance**

The County of Los Angeles has established thresholds of significance to determine whether a project traffic impact at a study intersection is considered significant and thus requires mitigation. Table 3 identifies the County of Los Angeles thresholds of significance for signalized intersections as defined in the *Los Angeles County Traffic Impact Analysis Report Guidelines (January 1, 1997)*, based on V/C ratios calculated to “2” decimals.

**Table 3  
County of Los Angeles  
Signalized Intersection Thresholds of Significance**

Pre-Project		Project V/C Increase
LOS	V/C	
C	0.71 - 0.80	0.04 or more
D	0.81 - 0.90	0.02 or more
E/F	0.91 or more	0.01 or more

**Source:** *Los Angeles County Traffic Impact Analysis Report Guidelines (January 1, 1997)*.

The County of Los Angeles traffic thresholds of significance for signalized intersections are utilized by the City of Rancho Palos Verdes.

To determine whether the addition of project-generated trips at an unsignalized study intersection results in a significant impact, the City of Rancho Palos Verdes has established the following thresholds of significance:

- A significant impact would occur at an unsignalized study intersection when the addition of project-generated trips causes the peak hour level of service of the study intersection to change from acceptable operation (LOS D or better) to deficient operation (LOS E or F); or
- A significant impact would occur at an unsignalized study intersection if the addition of project-generated trips changes the delay by the value shown in Table 4.

**Table 4**  
**City of Rancho Palos Verdes**  
**Unsignalized Intersection Thresholds of Significance**

Pre-Project		Project Delay Increase (seconds)
LOS	Delay (seconds)	
E/F	35.1 or more	2.0 or more

**Source:** *City of Rancho Palos Verdes.*

Table 5 identifies the City of Los Angeles thresholds of significance for signalized intersections as defined in the *Los Angeles Department of Transportation (LADOT) Traffic Study Policies and Procedures (Revised August 2003)*, based on V/C ratios calculated to “3” decimals.

**Table 5**  
**City of Los Angeles**  
**Signalized Intersection Thresholds of Significance**

LOS	Final V/C Ratio	Project V/C Increase
C	> 0.700 - 0.800	Equal to or greater than 0.040
D	> 0.800 - 0.900	Equal to or greater than 0.020
E, F	> 0.900	Equal to or greater than 0.010

**Source:** *LADOT Traffic Study Policies and Procedures (Revised August 2003).*

Where significant traffic impacts are identified, mitigation measures are identified to reduce the traffic impact to a level considered less than significant. Mitigation measures would be the full responsibility of the Project Applicant when the project causes a significant impact for existing with proposed project conditions and may be eligible for potential reimbursement by future projects that result in impacts at the same intersection. Further, mitigation measures would be a proportionate share contribution by the Project Applicant when the project causes a significant impact for cumulative with proposed project conditions.

Traffic analysis has been prepared for the following four scenarios:

- Existing Conditions;
- Forecast Existing With Project Conditions;
- Forecast Year 2012 Without Project Conditions; and
- Forecast Year 2012 With Project Conditions.

Traffic analysis has been prepared consistent with the 2007 and 2009 analyses, however, refinements to analysis software may produce slightly varying delay values. For ease in reading this focused analysis, exhibits showing intersection volumes and forecast trip assignment of the proposed project are contained in Appendix A.

**Existing Weekday Conditions AM & PM Peak Hour Intersection Level of Service**

Table 6 summarizes existing weekday conditions a.m. and p.m. peak hour LOS of the City of Rancho Palos Verdes study intersections; detailed LOS analysis sheets are contained in Appendix B.

**Table 6  
City of Rancho Palos Verdes  
Existing Weekday AM & PM Peak Hour Intersection LOS**

Study Intersection	Weekday AM Peak Hour (7 AM to 10 AM)			Weekday PM Peak Hour (4 PM to 6 PM)		
	V/C	Delay	LOS	V/C	Delay	LOS
Palos Verdes Dr East/Miraleste Dr	N/A	287.9	F	N/A	414.9	F
Palos Verdes Dr East/Crest Dr-College Entrance	0.44	N/A	A	0.33	N/A	A
Palos Verdes Dr East/Palos Verdes Dr South	N/A	190.0	C	N/A	17.8	C
Miraleste Dr/Via Colinita	N/A	21.7	C	N/A	18.3	C
Miraleste Dr/1 <sup>st</sup> St	N/A	14.7	B	N/A	14.6	B
Western Ave (SR-213)/Toscanini Dr	0.81	N/A	D	0.70	N/A	B
Western Ave (SR-213)/Trudie Dr-Capitol Dr	0.91	N/A	E	0.80	N/A	C
Western Ave (SR-213)/Crestwood St	0.86	N/A	D	0.81	N/A	D

**Note:** V/C = volume to capacity ratio; N/A = not applicable since delay is shown at unsignalized intersections and V/C ratio is shown at signalized intersections; delay shown in seconds per vehicle.

Table 7 summarizes existing weekday conditions a.m. and p.m. peak hour LOS of the City of Los Angeles study intersections; detailed LOS analysis sheets are contained in Appendix B.

**Table 7  
City of Los Angeles  
Existing Weekday AM & PM Peak Hour Intersection LOS**

Study Intersection	Weekday AM Peak Hour (7 AM to 10 AM)		Weekday PM Peak Hour (4 PM to 6 PM)	
	V/C	LOS	V/C	LOS
Western Ave (SR-213)/Trudie Dr-Capitol Dr	0.912	E	0.788	C
Western Ave (SR-213)/Crestwood St	0.809	D	0.759	C
Western Ave (SR-213)/1 <sup>st</sup> St	1.414	F	1.317	F
Western Ave (SR-213)/9 <sup>th</sup> St	0.607	B	0.804	D
Western Ave (SR-213)/25 <sup>th</sup> St	0.681	B	0.622	B

**Note:** V/C = volume to capacity ratio.

**Existing Weekday Mid-day & Afternoon Conditions Peak Hour Intersection Level of Service**

Table 8 summarizes existing weekday conditions mid-day peak hour and afternoon peak hour LOS of the City of Rancho Palos Verdes study intersections; detailed LOS analysis sheets are contained in Appendix B.

**Table 8  
City of Rancho Palos Verdes  
Existing Weekday Mid-day Peak Hour & Afternoon Peak Hour Intersection LOS**

Study Intersection	Weekday Mid-day Peak Hour (11 AM to 1 PM)			Weekday Afternoon Peak Hour (2 PM to 4 PM)		
	V/C	Delay	LOS	V/C	Delay	LOS
Palos Verdes Dr East/Miraleste Dr	N/A	169.3	F	N/A	250.5	F
Palos Verdes Dr East/Crest Dr-College Entrance	0.31	N/A	A	0.48	N/A	A
Palos Verdes Dr East/Palos Verdes Dr South	N/A	13.5	B	N/A	20.4	C
Miraleste Dr/Via Colinita	N/A	16.5	C	N/A	17.3	C

**Note:** V/C = volume to capacity ratio; N/A = not applicable since delay is shown at unsignalized intersections and V/C ratio is shown at signalized intersections; delay shown in seconds per vehicle.

**Existing Saturday Mid-day Conditions Peak Hour Intersection Level of Service**

Table 9 summarizes existing Saturday conditions mid-day peak hour LOS of the City of Rancho Palos Verdes study intersections; detailed LOS analysis sheets are contained in Appendix B.

**Table 9**  
**City of Rancho Palos Verdes**  
**Existing Saturday Mid-day Peak Hour Intersection LOS**

Study Intersection	Saturday Mid-day Peak Hour (11 AM to 1 PM)		
	V/C	Delay	LOS
Palos Verdes Dr East/Miraleste Dr	N/A	25.9	D
Palos Verdes Dr East/Crest Dr-College Entrance	0.20	N/A	A
Palos Verdes Dr East/Palos Verdes Dr South	N/A	14.9	B
Miraleste Dr/Via Colinita	N/A	16.3	C

**Note:** V/C = volume to capacity ratio; N/A = not applicable since delay is shown at unsignalized intersections and V/C ratio is shown at signalized intersections; delay shown in seconds per vehicle.

**PROPOSED PROJECT**

As indicated, this technical memorandum summarizes traffic and parking operations associated with the designation of 250 students from an AA Program to a BA Program. For purposes of this analysis, the AA Program and BA Program are defined as follows:

- **AA Program** – For traffic purposes defined as educational programs under the Junior/Community College land use, which usually last about two-years, but may vary depending on the student; and
- **BA Program** – For traffic purposes defined as educational programs under the University land use, which usually last about four-years, but may vary depending on the student.

As previously noted, the *Revised Marymount College Project Traffic & Parking Analysis (May 15, 2009)*, included analysis for the following modifications:

- Removal of residence halls;
- Revised number of existing student seats from 578 to 648;
- Reduce net new seats from 131 to 5 seats;
- Reduce new employees/faculty from 12 to 7 staff; and
- Increase weekend student enrollment from 83 to 150 students.

Refer to the *Marymount College Facilities Expansion Project Traffic & Parking Impact Analysis (September 28, 2007)* for additional proposed project descriptions.

## Forecast Project Trip Generation

To determine forecast trip generation of the proposed project, *ITE Trip Generation* published trip generation rates were used for specific land uses. *ITE* trip rates are based on surveys of representative facilities throughout the United States. Forecast project trip generation is determined through consideration of the following project components:

- Increase in junior college buildings through construction of 77,504 square feet and demolition of 18,022 square feet; and
- Designation of 250 students from an AA Program to a BA Program.

Consistent with *ITE*, the analysis assumes the project components consisting of the construction and demolition of campus facilities and buildings as the *ITE* Junior/Community College land use category. *ITE* describes the Junior/Community College land use as including two-year junior, community, or technical colleges. *ITE* describes the University/College land use as including four-year universities or colleges that may or may not offer graduate programs. The *ITE* Junior/Community College and University/College categories are assumed to include buildings serving administration and instruction, as well as ancillary uses such as library, cafeteria, athletic facilities, etc., but no on-campus dormitories. *ITE* trip rates for the Junior/Community College and University categories are assumed to account for trips associated with students, faculty, and support staff.

Consistent with *ITE*, traffic generation associated with the BA Program designation project component is quantified through comparison of trip generation per student in a BA Program versus an AA Program. The net increase in *ITE* trip generation through the BA Program designation of up to 250 students is then added to the trip generation identified for expansion of buildings on the campus. It is worth noting the *ITE* University land use category, which is utilized for the BA Program, does not include trip rates based on building square footage.

Table 10 summarizes the *ITE* weekday trip generation rates for the proposed project.

**Table 10**  
**Weekday *ITE* Trip Rates for Proposed Project**

Land Use ( <i>ITE</i> Code)	Units	AM Peak Hour Rates (7 AM to 10 AM)			Mid-Day Peak Hour Rates <sup>1</sup> (11 AM to 1 PM)			Afternoon Peak Hour Rates <sup>2</sup> (2 PM to 4 PM)			PM Peak Hour Rates (4 PM to 6 PM)			Daily Trip Rate
		In	Out	Total	In	Out	Total	In	Out	Total	In	Out	Total	
Junior/Community College (540)	tsf	2.21	0.78	2.99	1.55	1.54	3.09	1.16	1.48	2.64	1.47	1.07	2.54	27.49
Junior/Community College (540)	stu	0.10	0.02	0.12	0.08	0.04	0.12	0.07	0.05	0.12	0.08	0.04	0.12	1.20
University (550)	stu	0.17	0.04	0.21	0.15	0.05	0.20	0.07	0.17	0.24	0.06	0.15	0.21	2.38

**Source:** 2008 *ITE Trip Generation Manual, 8<sup>th</sup> Edition*.

**Note:** tsf = thousand square feet; stu = students.

1 = AM Peak Hour of Generator rates used.

2 = PM Peak Hour of Generator rates used.

Table 11 summarizes weekday trips forecast to be generated by the proposed project utilizing the *ITE* trip generation rates contained in Table 10.

**Table 11  
Forecast Weekday Trip Generation of Proposed Project**

Land Use	AM Peak Hour Trips (7 AM to 10 AM)			Mid-Day Peak Hour Trips (11 AM to 1 PM)			Afternoon Peak Hour Trips (2 PM to 4 PM)			PM Peak Hour Trips (4 PM to 6 PM)			Daily Trips
	In	Out	Total	In	Out	Total	In	Out	Total	In	Out	Total	
Junior/Community College Building Expansion	171	60	231	120	119	239	90	115	205	114	83	197	2,131
- Proposed 77.504-tsf <sup>1</sup>													
- Demolished 18.022-tsf	<u>- 40</u>	<u>- 14</u>	<u>- 54</u>	<u>- 28</u>	<u>- 28</u>	<u>- 56</u>	<u>- 21</u>	<u>- 27</u>	<u>- 48</u>	<u>- 26</u>	<u>- 19</u>	<u>- 45</u>	<u>- 495</u>
<b>Subtotal</b>	<b>131</b>	<b>46</b>	<b>177</b>	<b>92</b>	<b>91</b>	<b>183</b>	<b>69</b>	<b>88</b>	<b>157</b>	<b>88</b>	<b>64</b>	<b>152</b>	<b>1,636</b>
University	43	10	53	38	13	52	18	43	61	15	38	53	595
- 250 stu BA Program													
- 250 stu AA Program	<u>- 25</u>	<u>- 5</u>	<u>- 30</u>	<u>- 20</u>	<u>- 10</u>	<u>- 30</u>	<u>- 18</u>	<u>- 13</u>	<u>- 31</u>	<u>- 20</u>	<u>- 10</u>	<u>- 30</u>	<u>- 300</u>
<b>Subtotal</b>	<b>18</b>	<b>5</b>	<b>23</b>	<b>18</b>	<b>3</b>	<b>22</b>	<b>0</b>	<b>30</b>	<b>30</b>	<b>- 5</b>	<b>28</b>	<b>23</b>	<b>295</b>
<b>Total Forecast Trip Generation of Proposed Project</b>	<b>149</b>	<b>51</b>	<b>200</b>	<b>110</b>	<b>94</b>	<b>204</b>	<b>69</b>	<b>118</b>	<b>187</b>	<b>83</b>	<b>92</b>	<b>175</b>	<b>1,931</b>

**Note:** tsf = thousand square feet; stu = student.

As shown in Table 11, the proposed project is forecast to generate approximately 1,931 weekday daily trips, which include approximately 200 weekday a.m. peak hour trips, approximately 204 weekday mid-day peak hour trips, approximately 187 weekday afternoon trips, and approximately 175 weekday p.m. peak hour trips.

It should be noted, *ITE* also publishes Junior/Community College trip generation rates based on enrolled students; therefore, if *ITE* trip generation rates based on enrolled students are used, the junior college component of the proposed project would generate no new weekday trips because enrollment is not proposed to change.

Since the *Marymount College Facilities Expansion Project Traffic & Parking Impact Analysis (September 28, 2007)*, did not assume increased weekend student enrollment, the *ITE* Junior/Community College land use category trip rate is utilized based on either building square footage or student enrollment, where applicable.

Table 12 summarizes the *ITE* Saturday trip generation rates for the proposed project without residence halls.

**Table 12**  
**Saturday *ITE* Trip Rates for Proposed Project**

Land Use ( <i>ITE</i> Code)	Units	Mid-day Peak Hour Rates			Daily Trip Rate
		In	Out	Total	
Junior/Community College (540)	tsf	0.81	0.61	1.42	11.23
Junior/Community College (540)	stu	0.03	0.02	0.05	0.42
University (550)	stu	0.09	0.06	0.15	1.30

**Source:** 2008 *ITE Trip Generation Manual, 8<sup>th</sup> Edition*.

**Note:** tsf = thousand square feet; stu = students.

Table 13 summarizes Saturday trips forecast to be generated by the proposed project utilizing the *ITE* trip generation rates contained in Table 12.

**Table 13  
Forecast Saturday Trip Generation of Proposed Project**

Land Use	Mid-day Peak Hour Trips			Daily Trips
	In	Out	Total	
Junior/Community College Building Expansion - Proposed 77.504-tsf - Demolished 18.022-tsf	63 <u>- 15</u>	47 <u>- 11</u>	110 <u>- 26</u>	870 <u>- 202</u>
<b>Subtotal</b>	<b>48</b>	<b>36</b>	<b>84</b>	<b>668</b>
Junior/Community College - 67 Students <sup>1</sup>	<b>2</b>	<b>1</b>	<b>3</b>	<b>28</b>
University - 250 stu BA Program - 250 stu AA Program	23 <u>- 8</u>	15 <u>- 5</u>	38 <u>- 13</u>	325 <u>- 105</u>
<b>Subtotal</b>	<b>15</b>	<b>10</b>	<b>25</b>	<b>220</b>
<b>Total Forecast Trip Generation of Proposed Project</b>	<b>65</b>	<b>47</b>	<b>112</b>	<b>888</b>

**Note:** tsf = thousand square feet; stu = student.

1 = Represents increased weekend student enrollment (150 - 83 = 67).

As shown in Table 13, the proposed project is forecast to generate approximately 888 additional Saturday daily trips, which include approximately 112 additional Saturday mid-day peak hour trips.

### **Trip Distribution of Proposed Project**

The trip distribution utilized in this analysis is consistent with that identified in the *Marymount College Facilities Expansion Project Traffic & Parking Impact Analysis (September 28, 2007)* and is assumed to remain unchanged with project description modifications because origination and destinations of students, faculty, and staff are not expected to be substantially different due to the inclusion of the BA programs.

### **Trip Assignment of Proposed Project**

Exhibit A-1 shows the corresponding assignment of project-generated weekday a.m. and p.m. peak hour trips assuming the project trip percent distribution. Exhibit A-2 shows the corresponding assignment of project-generated weekday mid-day and afternoon peak hour trips assuming the project trip percent distribution. Exhibit A-3 shows the corresponding assignment of project-generated Saturday mid-day peak hour trips assuming the project trip percent distribution.

## **EXISTING PLUS MARYMOUNT PROJECT CONDITIONS**

This section analyzes the impact of adding trips forecast to be generated by the proposed project to existing traffic conditions.

## Existing Plus Marymount Project Conditions Peak Hour Traffic Volumes

Existing plus Marymount project traffic volumes were derived by adding project-generated trips to existing traffic volumes.

Exhibit A-4 shows existing plus Marymount project weekday conditions a.m. and p.m. peak hour volumes at the study intersections. Exhibit A-5 shows existing plus Marymount project weekday conditions mid-day peak hour and afternoon peak hour volumes. Exhibit A-6 shows existing plus Marymount project Saturday conditions mid-day peak hour volumes.

## Existing Plus Marymount Project Conditions Weekday AM & PM Peak Hour Intersection Level of Service

Table 14 summarizes existing plus Marymount project weekday conditions a.m. and p.m. peak hour LOS of the City of Rancho Palos Verdes study intersections; detailed LOS analysis sheets are contained in Appendix B.

**Table 14**  
**City of Rancho Palos Verdes**  
**Existing Plus Marymount Project Weekday**  
**Conditions AM & PM Peak Hour Intersection LOS**

Study Intersection	Existing Weekday Conditions		Existing Plus Marymount Project Weekday Conditions		Significant Impact?
	AM Peak Hour (7 AM to 10 AM)	PM Peak Hour (4 PM to 6 PM)	AM Peak Hour (7 AM to 10 AM)	PM Peak Hour (4 PM to 6 PM)	
	V/C – Delay – LOS	V/C – Delay – LOS	V/C – Delay – LOS	V/C – Delay – LOS	
Palos Verdes Dr East/Miraleste Dr	N/A – 287.9 – F	N/A – 414.9 – F	N/A – 724.1 – F	N/A – 643.0 – F	Yes
Palos Verdes Dr East/Crest Dr-College Entrance	0.44 – N/A – A	0.33 – N/A – A	0.54 – N/A – A	0.41 – N/A – A	No
Palos Verdes Dr East/Palos Verdes Dr South	N/A – 19.0 – C	N/A – 17.8 – C	N/A – 21.9 – C	N/A – 21.2 – C	No
Miraleste Dr/Via Colinita	N/A – 21.7 – C	N/A – 18.3 – C	N/A – 18.7 – C	N/A – 27.1 – D	No
Miraleste Dr/1 <sup>st</sup> St	N/A – 14.7 – B	N/A – 14.6 – B	N/A – 14.9 – B	N/A – 15.1 – B	No
Western Ave (SR-213)/Toscanini Dr	0.81 – N/A – D	0.70 – N/A – B	0.82 – N/A – D	0.71 – N/A – C	No
Western Ave (SR-213)/Trudie Dr-Capitol Dr	0.91 – N/A – E	0.80 – N/A – C	0.93 – N/A – E	0.83 – N/A – D	Yes
Western Ave (SR-213)/Crestwood St	0.86 – N/A – D	0.81 – N/A – D	0.86 – N/A – D	0.82 – N/A – D	No

**Note:** V/C = volume to capacity ratio; N/A = not applicable since delay is shown at unsignalized intersections and V/C ratio is shown at signalized intersections; delay shown in seconds per vehicle.

As shown in Table 14, based on City of Rancho Palos Verdes established thresholds of significance, the addition of project-generated trips is forecast to result in a significant impact at the following study intersections for existing plus Marymount project weekday conditions:

- Palos Verdes Drive East/Miraleste Drive (a.m. and p.m. peak hours); and
- Western Avenue (SR-213)/Trudie Drive-Capitol Drive (a.m. and p.m. peak hours).

Table 15 summarizes existing plus Marymount project weekday conditions a.m. and p.m. peak hour LOS of the City of Los Angeles study intersections; detailed LOS analysis sheets are contained in Appendix B.

**Table 15**  
**City of Los Angeles**  
**Existing Plus Marymount Project Weekday**  
**Conditions AM & PM Peak Hour Intersection LOS**

Study Intersection	Existing Weekday Conditions		Existing Plus Marymount Project Weekday Conditions		Significant Impact?
	AM Peak Hour (7 AM to 10 AM)	PM Peak Hour (4 PM to 6 PM)	AM Peak Hour (7 AM to 10 AM)	PM Peak Hour (4 PM to 6 PM)	
	V/C – LOS	V/C – LOS	V/C – LOS	V/C – LOS	
Western Ave (SR-213)/Trudie Dr-Capitol Dr	0.912 – E	0.788 – C	0.935 – E	0.822 – D	Yes
Western Ave (SR-213)/Crestwood St	0.809 – D	0.759 – C	0.812 – D	0.764 – C	No
Western Ave (SR-213)/1 <sup>st</sup> St	1.414 – F	1.317 – F	1.367 – F	1.321 – F	No
Western Ave (SR-213)/9 <sup>th</sup> St	0.607 – B	0.804 – D	0.609 – B	0.804 – D	No
Western Ave (SR-213)/25 <sup>th</sup> St	0.681 – B	0.622 – B	0.703 – C	0.635 – B	No

**Note:** V/C = volume to capacity ratio.

As shown in Table 15, based on City of Los Angeles established thresholds of significance, the addition of project-generated trips is forecast to result in a significant impact at the following intersection for existing plus Marymount project weekday conditions:

- Western Avenue (SR-213)/Trudie Drive-Capitol Drive (a.m. and p.m. peak hours).

**Existing Plus Marymount Project Weekday Mid-day & Afternoon Peak Hour Conditions Intersection Level of Service**

Table 16 summarizes existing plus Marymount project weekday conditions mid-day peak hour and afternoon peak hour LOS of the City of Rancho Palos Verdes study intersections; detailed LOS analysis sheets are contained in Appendix B.

**Table 16**  
**City of Rancho Palos Verdes**  
**Existing Plus Marymount Project Weekday Conditions**  
**Mid-day Peak Hour & Afternoon Peak Hour Intersection LOS**

Study Intersection	Existing Weekday Conditions		Existing Plus Marymount Project Weekday Conditions		Significant Impact?
	Mid-day Peak Hour (11 AM to 1 PM)	Afternoon Peak Hour (2 PM to 4 PM)	Mid-day Peak Hour (11 AM to 1 PM)	Afternoon Peak Hour (2 PM to 4 PM)	
	V/C – Delay – LOS	V/C – Delay – LOS	V/C – Delay – LOS	V/C – Delay – LOS	
Palos Verdes Dr East/Miraleste Dr	N/A – 169.3 – F	N/A – 250.5 – F	N/A – 345.5 – F	N/A – 437.4 – F	Yes
Palos Verdes Dr East/Crest Dr-College Entrance	0.31 – N/A – A	0.48 – N/A – A	0.44 – N/A – A	0.57 – N/A – A	No
Palos Verdes Dr East/Palos Verdes Dr South	N/A – 13.5 – B	N/A – 20.4 – C	N/A – 14.6 – B	N/A – 28.2 – C	No
Miraleste Dr/Via Colinita	N/A – 16.5 – C	N/A – 17.3 – C	N/A – 23.6 – C	N/A – 20.5 – C	No

**Note:** V/C = volume to capacity ratio; N/A = not applicable since delay is shown at unsignalized intersections and V/C ratio is shown at signalized intersections; delay shown in seconds per vehicle.

As shown in Table 16, based on City of Rancho Palos Verdes established thresholds of significance, the addition of project-generated trips is forecast to result in a significant impact at the following study intersection for existing plus Marymount project weekday conditions:

- Palos Verdes Drive East/Miraleste Drive (mid-day and afternoon peak hours).

**Existing Plus Marymount Project Saturday Mid-day Peak Hour Conditions Intersection Level of Service**

Table 17 summarizes existing plus Marymount project Saturday conditions mid-day peak hour LOS of the City of Rancho Palos Verdes study intersections; detailed LOS analysis sheets are contained in Appendix B.

**Table 17  
City of Rancho Palos Verdes  
Existing Plus Marymount Project Saturday  
Conditions Mid-day Peak Hour Intersection LOS**

Study Intersection	Existing Saturday Conditions	Existing Plus Marymount Project Saturday Conditions	Significant Impact?
	Mid-day Peak Hour (11 AM to 1 PM)	Mid-day Peak Hour (11 AM to 1 PM)	
	V/C – Delay – LOS	V/C – Delay – LOS	
Palos Verdes Dr East/Miraleste Dr	N/A – 25.9 – D	N/A – 48.5 – E	Yes
Palos Verdes Dr East/Crest Dr-College Entrance	0.20 – N/A – A	0.26 – N/A – A	No
Palos Verdes Dr East/Palos Verdes Dr South	N/A – 14.9 – B	N/A – 15.9 – C	No
Miraleste Dr/Via Colinita	N/A – 16.3 – C	N/A – 17.3 – C	No

**Note:** V/C = volume to capacity ratio; N/A = not applicable since delay is shown at unsignalized intersections and V/C ratio is shown at signalized intersections; delay shown in seconds per vehicle.

As shown in Table 17, based on City of Rancho Palos Verdes established thresholds of significance, the addition of project-generated trips is forecast to result a significant impact at the following study intersection for existing plus Marymount project Saturday conditions:

- Palos Verdes Drive East/Miraleste Drive (mid-day peak hour).

**Existing Plus Marymount Project Conditions Recommended Mitigation Measures**

The following feasible mitigation measures are recommended to eliminate significant traffic impacts for existing plus Marymount project conditions:

**Mitigation Measure No. 1:**

**Palos Verdes Drive East/Miraleste Drive** – Prior to issuance of any Certificate of Occupancy, the applicant shall signalize the intersection. Additionally, the applicant shall design the Palos Verdes Drive East/Miraleste Drive intersection traffic signal to include a westbound right-turn overlap, which will preclude u-turn movement from southbound to northbound Palos Verdes Drive East. The applicant may be eligible for reimbursement from future projects that result in impacts at this intersection.

**Mitigation Measure No. 2:**

**Western Avenue (SR-213)/Trudie Drive-Capitol Drive** – Prior to issuance of any Certificate of Occupancy, the applicant shall implement the re-striping of the eastbound Trudie Drive approach from one shared left-turn/through lane and one de-facto right-turn lane to consist of one left-turn lane and one shared through/right-turn lane. Implementation of this mitigation measure likely requires coordination with City of Los Angeles and Caltrans staff. The applicant may be eligible for reimbursement from future projects that result in impacts at this intersection.

**Mitigation Measure No. 3:**

The traffic impacts and corresponding mitigation measures assume the Marymount College student enrollment as a maximum of 793 weekday students (based on the formula allowing 750 full-time students, 20 part-time students, and a marginal difference of 3.0 percent) and 150 weekend students. Additionally, it is assumed, Marymount College student enrollment as a maximum of 250 weekday students enrolled in the BA Program and a maximum of 793 weekday students minus current BA Program weekday students enrolled in the AA Program. Therefore, prior to issuance of any Certificate of Occupancy, student enrollment shall be limited to a maximum of 793 weekday students and 150 weekend students, including full- and part-time students, and maximum of 250 weekday students enrolled in the BA Program and a maximum of 793 weekday students minus current BA Program weekday students enrolled in the AA Program.

Assuming modifications to the project description as described, Mitigation Measures 1 through 3 are the same as those identified in the *Marymount College Facilities Expansion Project Traffic & Parking Impact Analysis (September 28, 2007)*, and the *Revised Marymount College Project Traffic & Parking Analysis (May 15, 2009)*.

**Mitigated Existing Plus Marymount Project Weekday Conditions Intersection Level of Service**

Table 18 summarizes existing plus Marymount project weekday conditions a.m. and p.m. peak hour LOS of the City of Rancho Palos Verdes study intersections assuming full implementation of the recommended mitigation measures; detailed LOS analysis sheets are contained in Appendix B.

**Table 18  
City of Rancho Palos Verdes  
Mitigated Existing Plus Marymount Project Weekday  
Conditions AM & PM Peak Hour Intersection LOS**

Study Intersection	Existing Conditions		Mitigated Existing Plus Marymount Project Conditions		Significant Impact?
	AM Peak Hour (7 AM to 10 AM)	PM Peak Hour (4 PM to 6 PM)	AM Peak Hour (7 AM to 10 AM)	PM Peak Hour (4 PM to 6 PM)	
	V/C – Delay – LOS	V/C – Delay – LOS	V/C – Delay – LOS	V/C – Delay – LOS	
Palos Verdes Dr East/Miraleste Dr	N/A – 287.9 – F	N/A – 414.9 – F	0.95 – N/A – E	0.88 – N/A – D	No
Western Ave (SR-213)/Trudie Dr-Capitol Dr	0.91 – N/A – E	0.80 – N/A – C	0.86 – N/A – D	0.78 – N/A – C	No

**Note:** V/C = volume to capacity ratio; N/A = not applicable since delay is shown at unsignalized intersections and V/C ratio is shown at signalized intersections; delay shown in seconds per vehicle.

As shown in Table 18, no significant impacts are forecast to occur at City of Rancho Palos Verdes study intersections assuming full implementation of the recommended mitigation measures (1 and 2) for the existing plus Marymount project weekday conditions a.m. and p.m. peak hour.

Table 19 summarizes existing plus Marymount project weekday conditions a.m. and p.m. peak hour LOS of the Western Avenue (SR-213)/Trudie Drive-Capitol Drive intersection assuming full implementation of the recommended mitigation measure; detailed LOS analysis sheets are contained in Appendix B.

**Table 19**  
**City of Los Angeles**  
**Mitigated Existing Plus Marymount Project Weekday**  
**Conditions AM & PM Peak Hour Intersection LOS**

Study Intersection	Existing Conditions		Mitigated Existing Plus Marymount Project Conditions		Significant Impact?
	AM Peak Hour (7 AM to 10 AM)	PM Peak Hour (4 PM to 6 PM)	AM Peak Hour (7 AM to 10 AM)	PM Peak Hour (4 PM to 6 PM)	
	V/C – LOS	V/C – LOS	V/C – LOS	V/C – LOS	
Western Ave (SR-213)/Trudie Dr-Capitol Dr	0.912 – E	0.788 – C	0.855 – D	0.764 – C	No

**Note:** V/C = volume to capacity ratio.

As shown in Table 19, no significant impacts are forecast to occur at City of Los Angeles study intersections assuming full implementation of the recommended mitigation measure (2) for the existing plus Marymount project weekday conditions a.m. and p.m. peak hour.

Table 20 summarizes existing plus Marymount project weekday conditions mid-day and afternoon peak hour LOS of the City of Rancho Palos Verdes study intersections assuming full implementation of the recommended mitigation measures; detailed LOS analysis sheets are contained in Appendix B.

**Table 20**  
**City of Rancho Palos Verdes**  
**Mitigated Existing Plus Marymount Project Weekday Conditions**  
**Mid-day Peak Hour & Afternoon Peak Hour Intersection LOS**

Study Intersection	Existing Weekday Conditions		Mitigated Existing Plus Marymount Project Weekday Conditions		Significant Impact?
	Mid-day Peak Hour (11 AM to 1 PM)	Afternoon Peak Hour (2 PM to 4 PM)	Mid-day Peak Hour (11 AM to 1 PM)	Afternoon Peak Hour (2 PM to 4 PM)	
	V/C – Delay – LOS	V/C – Delay – LOS	V/C – Delay – LOS	V/C – Delay – LOS	
Palos Verdes Dr East/Miraleste Dr	N/A – 169.3 – F	N/A – 250.5 – F	0.78 – N/A – C	0.87 – N/A – D	No

**Note:** V/C = volume to capacity ratio; N/A = not applicable since delay is shown at unsignalized intersections and V/C ratio is shown at signalized intersections; delay shown in seconds per vehicle.

As shown in Table 20, no significant impacts are forecast to occur at City of Rancho Palos Verdes study intersections assuming full implementation of the recommended mitigation measure (1) for the existing plus Marymount project weekday conditions mid-day and afternoon peak hour.

**Mitigated Existing Plus Marymount Project Saturday Conditions Intersection Level of Service**

Table 21 summarizes existing plus Marymount project Saturday conditions mid-day peak hour LOS of the Palos Verdes Drive East/Miraleste Drive intersection assuming full implementation of the recommended mitigation measures; detailed LOS analysis sheets are contained in Appendix B.

**Table 21  
City of Rancho Palos Verdes  
Mitigated Existing Plus Marymount Project Saturday  
Conditions Mid-day Peak Hour Intersection LOS**

Study Intersection	Existing Saturday Conditions	Mitigated Existing Plus Marymount Project Saturday Conditions	Significant Impact?
	Mid-day Peak Hour (11 AM to 1 PM)	Mid-day Peak Hour (11 AM to 1 PM)	
	V/C – Delay – LOS	V/C – Delay – LOS	
Palos Verdes Dr East/Miraleste Dr	N/A – 25.9 – D	0.63 – N/A – B	No

**Note:** V/C = volume to capacity ratio; N/A = not applicable since delay is shown at unsignalized intersections and V/C ratio is shown at signalized intersections; delay shown in seconds per vehicle.

As shown in Table 21, no significant impacts are forecast to occur at City of Rancho Palos Verdes study intersections assuming full implementation of the recommended mitigation measure (1) for the existing plus Marymount project Saturday conditions mid-day peak hour.

**FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS**

This section evaluates study area traffic conditions in forecast year 2012 without the proposed project utilizing traffic volumes contained in the *Marymount College Facilities Expansion Project Traffic & Parking Impact Analysis (September 28, 2007)*.

Forecast year 2012 without project conditions does not assume the recommended mitigation measures identified for existing plus Marymount project conditions. Forecast year 2012 without project conditions includes the following City-planned modification at the Palos Verdes Drive East/Crest Road-College Entrance intersection associated with narrowing of Palos Verdes Drive East in the project vicinity from four lanes to two lanes:

- Narrowing the eastbound and westbound Palos Verdes Drive East approaches at the Palos Verdes Drive East/Crest Road-College Entrance intersection from one left-turn lane, two through lanes, and one right-turn lane to consist of one left-turn lane, one through lane, and one right-turn lane.

Additionally, forecast year 2012 conditions include the following recent modification at the Western Avenue (SR-213)/Crestwood Street intersection:

- Recent re-stripe of the eastbound Crestwood Street approach from one shared left-turn/through/right-turn lane to consist of one left-turn lane and one shared through/right-turn lane. Additionally, the east-west signal phasing has been modified from permitted left-turn phasing to split phasing.

**Forecast Year 2012 Without Project Weekday AM & PM Peak Hour Conditions  
Intersection Level of Service**

Table 22 summarizes forecast year 2012 without project weekday conditions a.m. and p.m. peak hour LOS of the City of Rancho Palos Verdes study intersections; detailed LOS analysis sheets are contained in Appendix B.

**Table 22  
City of Rancho Palos Verdes  
Forecast Year 2012 Without Project Weekday Conditions  
AM & PM Peak Hour Intersection LOS**

Study Intersection	Weekday AM Peak Hour (7 AM to 10 AM)			Weekday PM Peak Hour (4 PM to 6 PM)		
	V/C	Delay	LOS	V/C	Delay	LOS
Palos Verdes Dr East/Miraleste Dr	N/A	311.7	F	N/A	469.2	F
Palos Verdes Dr East/Crest Dr-College Entrance	0.50	N/A	A	0.39	N/A	A
Palos Verdes Dr East/Palos Verdes Dr South	N/A	28.2	D	N/A	31.9	D
Miraleste Dr/Via Colinita	N/A	23.5	C	N/A	19.7	C
Miraleste Dr/1 <sup>st</sup> St	N/A	14.8	B	N/A	14.9	B
Western Ave (SR-213)/Toscanini Dr	0.90	N/A	D	0.79	N/A	C
Western Ave (SR-213)/Trudie Dr-Capitol Dr	1.06	N/A	F	1.03	N/A	F
Western Ave (SR-213)/Crestwood St	0.86	N/A	D	0.90	N/A	D

**Note:** V/C = volume to capacity ratio; N/A = not applicable since delay is shown at unsignalized intersections and V/C ratio is shown at signalized intersections; delay shown in seconds per vehicle.

Table 23 summarizes forecast year 2012 without project weekday conditions a.m. and p.m. peak hour LOS of the City of Los Angeles study intersections; detailed LOS analysis sheets are contained in Appendix B.

**Table 23**  
**City of Los Angeles**  
**Forecast Year 2012 Without Project Weekday Conditions**  
**AM & PM Peak Hour Intersection LOS**

Study Intersection	Weekday AM Peak Hour (7 AM to 10 AM)		Weekday PM Peak Hour (4 PM to 6 PM)	
	V/C	LOS	V/C	LOS
Western Ave (SR-213)/Trudie Dr-Capitol Dr	1.074	F	1.048	F
Western Ave (SR-213)/Crestwood St	0.858	D	0.893	D
Western Ave (SR-213)/1 <sup>st</sup> St	1.516	F	1.438	F
Western Ave (SR-213)/9 <sup>th</sup> St	0.659	B	0.868	D
Western Ave (SR-213)/25 <sup>th</sup> St	0.814	D	0.805	D

**Note:** V/C = volume to capacity ratio.

**Forecast Year 2012 Without Project Weekday Mid-day & Afternoon Peak Hour Conditions Intersection Level of Service**

Table 24 summarizes forecast year 2012 without project weekday conditions mid-day peak hour and afternoon peak hour LOS of the City of Rancho Palos Verdes study intersections; detailed LOS analysis sheets are contained in Appendix B.

**Table 24**  
**City of Rancho Palos Verdes**  
**Forecast Year 2012 Without Project Weekday Conditions**  
**Mid-Day Peak Hour & Afternoon Peak Hour Intersection LOS**

Study Intersection	Weekday Mid-day-Peak Hour (11 AM to 1 PM)			Weekday Afternoon Peak Hour (2 PM to 4 PM)		
	V/C	Delay	LOS	V/C	Delay	LOS
Palos Verdes Dr East/Miraleste Dr	N/A	205.6	F	N/A	313.3	F
Palos Verdes Dr East/Crest Dr-College Entrance	0.36	N/A	A	0.51	N/A	A
Palos Verdes Dr East/Palos Verdes Dr South	N/A	19.7	C	N/A	46.4	E
Miraleste Dr/Via Colinita	N/A	17.6	C	N/A	18.3	C

**Note:** V/C = volume to capacity ratio; N/A = not applicable since delay is shown at unsignalized intersections and V/C ratio is shown at signalized intersections; delay shown in seconds per vehicle.

**Forecast Year 2012 Without Project Saturday Mid-day Conditions Intersection Level of Service**

Table 25 summarizes forecast year 2012 without project Saturday conditions mid-day peak hour LOS of the City of Rancho Palos Verdes study intersections; detailed LOS analysis sheets are contained in Appendix B.

**Table 25**  
**City of Rancho Palos Verdes**  
**Forecast Year 2012 Without Project**  
**Saturday Conditions Mid-day Peak Hour Intersection LOS**

Study Intersection	Saturday Mid-day Peak Hour (11 AM to 1 PM)		
	V/C	Delay	LOS
Palos Verdes Dr East/Miraleste Dr	N/A	36.5	E
Palos Verdes Dr East/Crest Dr-College Entrance	0.26	N/A	A
Palos Verdes Dr East/Palos Verdes Dr South	N/A	31.8	D
Miraleste Dr/Via Colinita	N/A	17.3	C

**Note:** V/C = volume to capacity ratio; N/A = not applicable since delay is shown at unsignalized intersections and V/C ratio is shown at signalized intersections; delay shown in seconds per vehicle.

### **FORECAST YEAR 2012 WITH PROJECT CONDITIONS**

Consistent with the *Marymount College Facilities Expansion Project Traffic & Parking Impact Analysis (September 28, 2007)*, this section analyzes the impact of adding trips forecast to be generated by the proposed project to forecast year 2012 without project traffic conditions.

#### **Forecast Year 2012 With Project Conditions Peak Hour Traffic Volumes**

Forecast year 2012 with project traffic volumes were derived by adding project-generated trips to forecast year 2012 without project traffic volumes.

Exhibit A-7 shows forecast year 2012 with project conditions weekday a.m. and p.m. peak hour volumes. Exhibit A-8 shows forecast year 2012 with project conditions weekday mid-day peak hour and afternoon peak hour volumes. Exhibit A-9 shows forecast year 2012 with project conditions Saturday mid-day peak hour volumes.

#### **Forecast Year 2012 With Project Weekday AM & PM Peak Hour Conditions Intersection Level of Service**

Table 26 summarizes forecast year 2012 with project weekday conditions a.m. and p.m. peak hour LOS of the City of Rancho Palos Verdes study intersections; detailed LOS analysis sheets are contained in Appendix B.

**Table 26**  
**City of Rancho Palos Verdes**  
**Forecast Year 2012 With Project Weekday**  
**Conditions AM & PM Peak Hour Intersection LOS**

Study Intersection	Forecast Year 2012 Without Project Conditions		Forecast Year 2012 With Project Conditions		Significant Impact?
	AM Peak Hour (7 AM to 10 AM)	PM Peak Hour (4 PM to 6 PM)	AM Peak Hour (7 AM to 10 AM)	PM Peak Hour (4 PM to 6 PM)	
	V/C – Delay – LOS	V/C – Delay – LOS	V/C – Delay – LOS	V/C – Delay – LOS	
Palos Verdes Dr East/Miraleste Dr	N/A – 311.7 – F	N/A – 469.2 – F	N/A – 766.9 – F	N/A – 715.3 – F	Yes
Palos Verdes Dr East/Crest Dr-College Entrance	0.50 – N/A – A	0.39 – N/A – A	0.61 – N/A – B	0.45 – N/A – A	No
Palos Verdes Dr East/Palos Verdes Dr South	N/A – 28.2 – D	N/A – 31.9 – D	N/A – 36.1 – E	N/A – 47.5 – E	Yes
Miraleste Dr/Via Colinita	N/A – 23.5 – C	N/A – 19.7 – C	N/A – 19.1 – C	N/A – 18.1 – C	No
Miraleste Dr/1 <sup>st</sup> St	N/A – 14.8 – B	N/A – 14.9 – B	N/A – 15.0 – B	N/A – 15.3 – C	No
Western Ave (SR-213)/Toscanini Dr	0.90 – N/A – D	0.79 – N/A – C	0.91 – N/A – E	0.80 – N/A – C	No
Western Ave (SR-213)/Trudie Dr-Capitol Dr	1.06 – N/A – F	1.03 – N/A – F	1.08 – N/A – F	1.06 – N/A – F	Yes
Western Ave (SR-213)/Crestwood St	0.86 – N/A – D	0.90 – N/A – D	0.87 – N/A – D	0.90 – N/A – D	No

**Note:** V/C = volume to capacity ratio; N/A = not applicable since delay is shown at unsignalized intersections and V/C ratio is shown at signalized intersections; delay shown in seconds per vehicle.

As shown in Table 26, based on City of Rancho Palos Verdes established thresholds of significance, the addition of project-generated trips is forecast to result in a significant impact at the following study intersections for forecast year 2012 with project weekday conditions:

- Palos Verdes Drive East/Miraleste Drive (a.m. and p.m. peak hours);
- Palos Verdes Drive East/Palos Verdes Drive South (a.m. and p.m. peak hours); and
- Western Avenue (SR-213)/Trudie Drive-Capitol Drive (a.m. and p.m. peak hours).

Table 27 summarizes forecast year 2012 with project weekday conditions a.m. and p.m. peak hour LOS of the City of Los Angeles study intersections; detailed LOS analysis sheets are contained in Appendix B.

**Table 27**  
**City of Los Angeles**  
**Forecast Year 2012 With Project Weekday**  
**Conditions AM & PM Peak Hour Intersection LOS**

Study Intersection	Forecast Year 2012 Without Project Conditions		Forecast Year 2012 With Project Conditions		Significant Impact?
	AM Peak Hour (7 AM to 10 AM)	PM Peak Hour (4 PM to 6 PM)	AM Peak Hour (7 AM to 10 AM)	PM Peak Hour (4 PM to 6 PM)	
	V/C – LOS	V/C – LOS	V/C – LOS	V/C – LOS	
Western Ave (SR-213)/Trudie Dr-Capitol Dr	1.074 – F	1.048 – F	1.097 – F	1.082 – F	Yes
Western Ave (SR-213)/Crestwood St	0.858 – D	0.893 – D	0.861 – D	0.898 – D	No
Western Ave (SR-213)/1 <sup>st</sup> St	1.516 – F	1.438 – F	1.465 – F	1.442 – F	No
Western Ave (SR-213)/9 <sup>th</sup> St	0.659 – B	0.868 – D	0.661 – B	0.868 – D	No
Western Ave (SR-213)/25 <sup>th</sup> St	0.814 – D	0.805 – D	0.833 – D	0.818 – D	No

**Note:** V/C = volume to capacity ratio.

As shown in Table 27, based on City of Los Angeles established thresholds of significance, the addition of project-generated trips is forecast to result in a significant impact at the following study intersection for forecast year 2012 with project weekday conditions:

- Western Avenue (SR-213)/Trudie Drive-Capitol Drive (a.m. and p.m. peak hours).

**Forecast Year 2012 With Project Weekday Mid-day & Afternoon Peak Hour Conditions Intersection Level of Service**

Table 28 summarizes forecast year 2012 with project weekday conditions mid-day peak hour and afternoon peak hour LOS of the City of Rancho Palos Verdes study intersections; detailed LOS analysis sheets are contained in Appendix B.

**Table 28**  
**City of Rancho Palos Verdes**  
**Forecast Year 2012 With Project Weekday Conditions**  
**Mid-day Peak Hour & Afternoon Peak Hour Intersection LOS**

Study Intersection	Forecast Year 2012 Without Project Conditions		Forecast Year 2012 With Project Conditions		Significant Impact?
	Mid-day Peak Hour (11 AM to 1 PM)	Afternoon Peak Hour (2 PM to 4 PM)	Mid-day Peak Hour (11 AM to 1 PM)	Afternoon Peak Hour (2 PM to 4 PM)	
	V/C – Delay – LOS	V/C – Delay – LOS	V/C – Delay – LOS	V/C – Delay – LOS	
Palos Verdes Dr East/Miraleste Dr	N/A – 205.6 – F	N/A – 313.3 – F	N/A – 398.2 – F	N/A – 525.6 – F	Yes
Palos Verdes Dr East/Crest Dr- College Entrance	0.36 – N/A – A	0.51 – N/A – A	0.48 – N/A – A	0.63 – N/A – B	No
Palos Verdes Dr East/Palos Verdes Dr South	N/A – 19.7 – C	N/A – 46.4 – E	N/A – 23.4 – C	N/A – 90.9 – F	Yes
Miraleste Dr/Via Colinita	N/A – 17.6 – C	N/A – 18.3 – C	N/A – 29.3 – D	N/A – 22.2 – C	No

**Note:** V/C = volume to capacity ratio; N/A = not applicable since delay is shown at unsignalized intersections and V/C ratio is shown at signalized intersections; delay shown in seconds per vehicle.

As shown in Table 28, based on City of Rancho Palos Verdes established thresholds of significance, the addition of project-generated trips is forecast to result in a significant impact at the following study intersections for forecast year 2012 with project weekday conditions:

- Palos Verdes Drive East/Miraleste Drive (mid-day and afternoon peak hours); and
- Palos Verdes Drive East/Palos Verdes Drive South (afternoon peak hour only).

**Forecast Year 2012 With Project Saturday Mid-day Peak Hour Conditions Intersection Level of Service**

Table 29 summarizes forecast year 2012 with project Saturday conditions mid-day peak hour LOS of the City of Rancho Palos Verdes study intersections; detailed LOS analysis sheets are contained in Appendix B.

**Table 29**  
**City of Rancho Palos Verdes**  
**Forecast Year 2012 With Project Saturday**  
**Conditions Mid-day Peak Hour Intersection LOS**

Study Intersection	Forecast Year 2012 Without Project Conditions	Forecast Year 2012 With Project Conditions	Significant Impact?
	Mid-day Peak Hour (11 AM to 1 PM)	Mid-day Peak Hour (11 AM to 1 PM)	
	V/C – Delay – LOS	V/C – Delay – LOS	
Palos Verdes Dr East/Miraleste Dr	N/A – 36.5 – E	N/A – 75.1 – F	Yes
Palos Verdes Dr East/Crest Dr-College Entrance	0.26 – N/A – A	0.33 – N/A – A	No
Palos Verdes Dr East/Palos Verdes Dr South	N/A – 31.8 – D	N/A – 38.9 – E	Yes
Miraleste Dr/Via Colinita	N/A – 17.3 – C	N/A – 18.5 – C	No

**Note:** V/C = volume to capacity ratio; N/A = not applicable since delay is shown at unsignalized intersections and V/C ratio is shown at signalized intersections; delay shown in seconds per vehicle.

As shown in Table 29, based on City of Rancho Palos Verdes established thresholds of significance, the addition of project-generated trips is forecast to result in a significant impact at the following study intersections for forecast year 2012 with project Saturday conditions:

- Palos Verdes Drive East/Miraleste Drive (mid-day peak hour); and
- Palos Verdes Drive East/Palos Verdes Drive South (mid-day peak hour).

**Forecast Year 2012 With Project Conditions Recommended Mitigation Measures**

The following feasible mitigation measures are recommended to eliminate significant traffic impacts for forecast year 2012 with project conditions:

**Mitigation Measure No. 4:**

**Palos Verdes Drive East/Miraleste Drive** – Prior to issuance of any Certificate of Occupancy, the applicant shall signalize the intersection. Additionally, the applicant shall design the Palos Verdes Drive East/Miraleste Drive intersection traffic signal to include a westbound right-turn overlap, which will preclude u-turn movement from southbound to northbound Palos Verdes Drive East. The applicant may be eligible for reimbursement from future projects that result in impacts at this intersection (*Same recommendation as mitigation measure 1 identified for existing plus Marymount project conditions*).

**Mitigation Measure No. 5:**

**Palos Verdes Drive East/Palos Verdes Drive South** – Prior to issuance of any Certificate of Occupancy, the applicant shall make a proportionate share contribution to implement the modification of the intersection to provide a two-stage gap acceptance design for southbound left-turning vehicles. The applicant shall make a proportionate share contribution to implement the construction of a raised median refuge area for vehicles to turn left from Palos Verdes Drive East to cross westbound Palos Verdes Drive South while

waiting for a gap in eastbound traffic to complete the turn to eastbound Palos Verdes Drive South. Additionally, the applicant shall make a proportionate share contribution to implement the narrowing of the existing raised median to provide an acceleration lane along Palos Verdes Drive South to accommodate vehicles accelerating to join eastbound Palos Verdes Drive South traffic flow. Modifications to the Palos Verdes Drive East/Palos Verdes Drive South intersection shall be designed taking into account truck turning radius requirements and shall be to the satisfaction of the Public Works Director. Since the Palos Verdes Drive East/Palos Verdes Drive South intersection is impacted by the proposed project for cumulative with proposed project conditions, a proportionate share contribution by the project applicant is applicable.

**Mitigation Measure No. 6:**

**Western Avenue (SR-213)/Trudie Drive-Capitol Drive** – Prior to issuance of any Certificate of Occupancy, the applicant shall implement the re-striping of the eastbound Trudie Drive approach from one shared left-turn/through lane and one de-facto right-turn lane to consist of one left-turn lane and one shared through/right-turn lane. Implementation of this mitigation measure likely requires coordination with City of Los Angeles and Caltrans staff. The applicant may be eligible for reimbursement from future projects that result in impacts at this intersection (*Same recommendation as mitigation measure 2 identified for existing plus Marymount project conditions*).

**Mitigation Measure No. 7:**

The traffic impacts and corresponding mitigation measures assume the Marymount College student enrollment as a maximum of 793 weekday students (based on the formula allowing 750 full-time students, 20 part-time students, and a marginal difference of 3.0 percent) and 150 weekend students. Therefore, prior to issuance of any Certificate of Occupancy, student enrollment shall be limited to a maximum of 793 weekday students and 150 weekend students, including full- and part-time students (*Same recommendation as mitigation measure 3 identified for existing plus Marymount project conditions*).

Assuming modifications to the project description as described, Mitigation Measures 4 through 7 are the same as those identified in the *Marymount College Facilities Expansion Project Traffic & Parking Impact Analysis (September 28, 2007)*, and the *Revised Marymount College Project Traffic & Parking Analysis (May 15, 2009)*.

**Mitigated Forecast Year 2012 With Project Weekday Conditions Intersection Level of Service**

Table 30 summarizes forecast year 2012 with project weekday conditions a.m. and p.m. peak hour LOS of the City of Rancho Palos Verdes study intersections assuming full implementation of the recommended mitigation measures; detailed LOS analysis sheets are contained in Appendix B. Assuming full implementation of the recommended Mitigation Measure 8, LOS calculations at the Palos Verdes Drive East/Palos Verdes Drive South intersection are determined using the *Highway Capacity Software (HCS)* to take into account the two-stage gap acceptance.

**Table 30**  
**City of Rancho Palos Verdes**  
**Mitigated Forecast Year 2012 With Project Weekday**  
**Conditions AM & PM Peak Hour Intersection LOS**

Study Intersection	Forecast Year 2012 Without Project Conditions		Mitigated Forecast Year 2012 With Project Conditions		Significant Impact?
	AM Peak Hour (7 AM to 10 AM)	PM Peak Hour (4 PM to 6 PM)	AM Peak Hour (7 AM to 10 AM)	PM Peak Hour (4 PM to 6 PM)	
	V/C – Delay – LOS	V/C – Delay – LOS	V/C – Delay – LOS	V/C – Delay – LOS	
Palos Verdes Dr East/Miraleste Dr	N/A – 311.7 – F	N/A – 469.2 – F	0.96 – N/A – E	0.90 – N/A – D	No
Palos Verdes Dr East/Palos Verdes Dr South	N/A – 28.2 – C	N/A – 31.9 – D	N/A – 19.1 – C	N/A – 20.4 – C	No
Western Ave (SR-213)/Trudie Dr-Capitol Dr	1.06 – N/A – F	1.03 – N/A – F	1.00 – N/A – E	1.01 – N/A – F	No

**Note:** V/C = volume to capacity ratio; N/A = not applicable since delay is shown at unsignalized intersections and V/C ratio is shown at signalized intersections; delay shown in seconds per vehicle.

As shown in Table 30, no significant impacts are forecast to occur at City of Rancho Palos Verdes mitigated study intersections assuming full implementation of the recommended mitigation measures (4 through 6) for the forecast year 2012 with project weekday conditions a.m. and p.m. peak hour.

Table 31 summarizes forecast year 2012 with project weekday conditions a.m. and p.m. peak hour LOS of the Western Avenue (SR-213)/Trudie-Capitol Drive intersection assuming full implementation of the recommended mitigation measure; detailed LOS analysis sheets are contained in Appendix B.

**Table 31**  
**City of Los Angeles**  
**Mitigated Forecast Year 2012 With Project Weekday**  
**Conditions AM & PM Peak Hour Intersection LOS**

Study Intersection	Forecast Year 2012 Without Project Conditions		Mitigated Forecast Year 2012 With Project Conditions		Significant Impact?
	AM Peak Hour (7 AM to 10 AM)	PM Peak Hour (4 PM to 6 PM)	AM Peak Hour (7 AM to 10 AM)	PM Peak Hour (4 PM to 6 PM)	
	V/C – LOS	V/C – LOS	V/C – LOS	V/C – LOS	
Western Ave (SR-213)/Trudie Dr-Capitol Dr	1.074 – F	1.048 – F	1.013 – F	1.023 – F	No

**Note:** V/C = volume to capacity ratio.

As shown in Table 31, no significant impacts are forecast to occur at City of Los Angeles mitigated study intersections assuming full implementation of the recommended mitigation measure (6) for the forecast year 2012 with project weekday conditions a.m. and p.m. peak hour.

Table 32 summarizes forecast year 2012 with project weekday conditions mid-day and afternoon peak hour LOS of the City of Rancho Palos Verdes study intersections assuming full

implementation of the recommended mitigation measures; detailed LOS analysis sheets are contained in Appendix B.

**Table 32**  
**City of Rancho Palos Verdes**  
**Mitigated Forecast Year 2012 With Project Weekday**  
**Conditions Mid-day Peak Hour & Afternoon Peak Hour Intersection LOS**

Study Intersection	Forecast Year 2012 Without Project Conditions		Mitigated Forecast Year 2012 With Project Conditions		Significant Impact?
	Mid-day Peak Hour (11 AM to 1 PM)	Afternoon Peak Hour (2 PM to 4 PM)	Mid-day Peak Hour (11 AM to 1 PM)	Afternoon Peak Hour (2 PM to 4 PM)	
	V/C – Delay – LOS	V/C – Delay – LOS	V/C – Delay – LOS	V/C – Delay – LOS	
Palos Verdes Dr East/Miraleste Dr	N/A – 205.6 – F	N/A – 313.3 – F	0.80 – N/A – C	0.90 – N/A – D	No
Palos Verdes Dr East/Palos Verdes Dr South	N/A – 19.7 – C	N/A – 46.4 – E	N/A – 15.8 – C	N/A – 25.3 – D	No

**Note:** V/C = volume to capacity ratio; N/A = not applicable since delay is shown at unsignalized intersections and V/C ratio is shown at signalized intersections; delay shown in seconds per vehicle.

As shown in Table 32, no significant impacts are forecast to occur at City of Rancho Palos Verdes mitigated study intersections assuming full implementation of the recommended mitigation measures (4 and 5) for the forecast year 2012 with project weekday conditions mid-day and afternoon peak hour.

Full implementation of Mitigation Measures 4, 5, and 6 would reduce the significant impacts to a level considered less than significant at the following intersections for forecast year 2012 with project weekday conditions:

- Palos Verdes Drive East/Miraleste Drive;
- Palos Verdes Drive East/Palos Verdes Drive South; and
- Western Avenue (SR-213)/Trudie Drive-Capitol Drive.

However, since proportionate share contribution to Mitigation Measure 5 would not fully implement the measure, the significant impact at the Palos Verdes Drive East/Palos Verdes Drive South intersection would not be reduced to a level considered less than significant. Therefore, a significant and unavoidable traffic impact would remain at the Palos Verdes Drive East/Palos Verdes Drive South intersection for forecast year 2012 with project weekday conditions.

**Mitigated Forecast Year 2012 With Project Saturday Conditions Intersection Level of Service**

Table 33 summarizes forecast year 2012 with project Saturday conditions mid-day peak hour LOS of the Palos Verdes Drive East/Miraleste Drive intersection assuming full implementation of the recommended mitigation measures; detailed LOS analysis sheets are contained in Appendix B.

**Table 33**  
**City of Rancho Palos Verdes**  
**Mitigated Forecast Year 2012 With Project Saturday**  
**Conditions Mid-day Peak Hour Intersection LOS**

Study Intersection	Forecast Year 2012 Without Project Saturday Conditions	Mitigated Forecast Year 2012 With Project Saturday Conditions	Significant Impact?
	Mid-day Peak Hour (11 AM to 1 PM)	Mid-day Peak Hour (11 AM to 1 PM)	
	V/C – Delay – LOS	V/C – Delay – LOS	
Palos Verdes Dr East/Miraleste Dr	N/A – 36.5 – E	0.67 – N/A – B	No
Palos Verdes Dr East/Palos Verdes Dr South	N/A – 31.8 – D	N/A – 20.1 – C	No

**Note:** V/C = volume to capacity ratio; N/A = not applicable since delay is shown at unsignalized intersections and V/C ratio is shown at signalized intersections; delay shown in seconds per vehicle.

As shown in Table 33, no significant impacts are forecast to occur at City of Rancho Palos Verdes mitigated study intersections assuming full implementation of the recommended mitigation measures (7 and 8) for the forecast year 2012 with project Saturday conditions mid-day peak hour.

Full implementation of Mitigation Measures 4 and 5 would reduce the significant impacts to a level considered less than significant at the following intersections for forecast year 2012 with project Saturday conditions:

- Palos Verdes Drive East/Miraleste Drive; and
- Palos Verdes Drive East/Palos Verdes Drive South.

However, since proportionate share contribution to Mitigation Measure 5 would not fully implement the measure, the significant impact at the Palos Verdes Drive East/Palos Verdes Drive South intersection would not be reduced to a level considered less than significant. Therefore, a significant and unavoidable traffic impact would remain at the Palos Verdes Drive East/Palos Verdes Drive South intersection for forecast year 2012 with project Saturday conditions.

### **LOS ANGELES CONGESTION MANAGEMENT PROGRAM ANALYSIS**

The purpose of the Los Angeles Congestion Management Program (CMP) is to develop a coordinated approach to managing and decreasing traffic congestion by linking the various transportation, land use and air quality planning programs throughout the County. The program is consistent with that of the Southern California Association of Governments (SCAG). The CMP program requires review of substantial individual projects, which might on their own impact the CMP transportation system.

Utilizing CMP (*Los Angeles County Metropolitan Transportation Authority, July 2004*) guidelines, the following intersections are included in the CMP study area:

- Western Avenue (SR-213)/Toscanini Drive; and
- Western Avenue (SR-213)/9<sup>th</sup> Street.

## CMP Thresholds of Significance

To determine whether the addition of project-generated trips results in a significant impact at the CMP study facility, and thus requires mitigation, the Los Angeles County CMP utilizes the following threshold of significance based on V/C ratios calculated to “2” decimals:

- A significant project impact occurs when a proposed project increases traffic demand at a CMP study facility by two-percent of capacity ( $V/C \geq 0.02$ ), causing or worsening LOS F ( $V/C > 1.00$ ).

## Existing Plus Marymount Project Weekday AM & PM Peak Hour Conditions CMP Intersection Level of Service

Table 34 summarizes the existing plus Marymount project weekday a.m. peak hour and p.m. peak hour LOS of the CMP study intersections; detailed LOS analysis sheets are contained in Appendix B.

**Table 34**  
**Existing Plus Marymount Project Weekday**  
**Conditions AM & PM Peak Hour CMP Intersection LOS**

CMP Study Intersection	Existing Weekday Conditions		Existing Plus Marymount Project Weekday Conditions		Significant Impact?
	AM Peak Hour (7 AM to 10 AM)	PM Peak Hour (4 PM to 6 PM)	AM Peak Hour (7 AM to 10 AM)	PM Peak Hour (4 PM to 6 PM)	
	V/C - LOS	V/C - LOS	V/C - LOS	V/C - LOS	
Western Ave (SR-213)/Toscanini Dr	0.81 – D	0.70 – B	0.82 – D	0.71 – B	No
Western Ave (SR-213)/9 <sup>th</sup> Street	0.64 – B	0.82 – D	0.64 – B	0.82 – D	No

**Note:** V/C = Volume to capacity ratio.

As shown in Table 34, the addition of project-generated trips at the CMP study intersections is forecast to result in no significant impacts for existing plus Marymount project conditions.

## Forecast Year 2012 With Project Weekday AM & PM Hour Conditions CMP Intersection Peak Level of Service

Table 35 summarizes the forecast year 2012 with project weekday a.m. peak hour and p.m. peak hour LOS of the CMP study intersections; detailed LOS analysis sheets are contained in Appendix B.

**Table 35**  
**Forecast Year 2012 With Project Weekday**  
**Conditions AM & PM Peak Hour CMP Intersection LOS**

CMP Study Intersection	Forecast Year 2012 Without Project Weekday Conditions		Forecast Year 2012 With Project Weekday Conditions		Significant Impact?
	AM Peak Hour (7 AM to 10 AM)	PM Peak Hour (4 PM to 6 PM)	AM Peak Hour (7 AM to 10 AM)	PM Peak Hour (4 PM to 6 PM)	
	V/C - LOS	V/C - LOS	V/C - LOS	V/C - LOS	
Western Ave (SR-213)/Toscanini Dr	0.90 – D	0.79 – C	0.91 – E	0.80 – C	No
Western Ave (SR-213)/9 <sup>th</sup> Street	0.69 – B	0.87 – D	0.69 – B	0.87 – D	No

**Note:** V/C = Volume to capacity ratio.

As shown in Table 35, the addition of project-generated trips at the CMP study intersections is forecast to result in no significant impacts for forecast year 2012 with project conditions.

### CMP Transit Impact Analysis

The following transit services are available in the vicinity of the proposed project site:

- Palos Verdes Peninsula Transit Authority (PVPTA) Gold, Orange, and Green Lines; and
- Metro Bus Lines 205, 447 and 550.

The proposed project site is located approximately 1.4 miles (directly) from the Metro Bus Lines 205, 447 and 550, generally serving the San Pedro area east of the proposed project site, with bus stops along Western Avenue and 7<sup>th</sup> Street. The PVPTA Gold and Orange lines pass adjacent Marymount College via Palos Verdes Drive East. The PVPTA Green line passes through the Palos Verdes Drive East/Miraleste Drive intersection approximately one mile (directly) from the project site.

Additionally, the College provides a shuttle bus service operating on a set schedule to transport students and faculty to and from the two housing facilities (Palos Verdes North and Pacific View) and the campus, a distance of approximately six miles. The shuttle operates between 7:00 a.m. and 10:00 p.m. Monday through Friday, and operates on a limited schedule on the weekends during the last week of each semester, finals week, and for special occasions or events. Based on shuttle ridership information provided by the College, approximately 136 students/faculty utilize the shuttle on a typical weekday to arrive on campus from the Palos Verdes North housing facility and 76 students/faculty utilize the shuttle on a typical weekday to arrive on campus from the Pacific View housing facility. Shuttle ridership leaving the campus shows lower usage, likely due to carpooling with students driving in their own vehicles.

The proposed project is forecast to generate approximately 1,931 weekday daily trips, which includes approximately 200 weekday a.m. peak hour trips and approximately 175 weekday p.m. peak hour trips. As per CMP guidelines, person trips can be estimated using a 1.4 factor to convert total vehicle trips to person trips, which results in a total of 280 a.m. peak hour person trips, 245 p.m. peak hour person trips and 2,703 daily person trips generated by the project.

Based on the CMP guidelines for determining trips assigned to transit the following factor applicable to the proposed project is utilized:

- 3.5 percent of Total Person Trips Generated.

Table 36 shows the calculation of project-generated transit trips utilizing CMP guidelines.

**Table 36  
CMP Transit Trip Generation of Proposed Project**

	<b>AM Peak Hour (7 AM to 10 AM)</b>	<b>PM Peak Hour (4 PM to 6 PM)</b>	<b>Daily Trips</b>
Trip Generation of Proposed Project (Vehicles)	200	175	1,931
Person Trips Conversion Factor	1.4	1.4	1.4
Person Trips of Proposed Project	280	245	2,703
3.5% Transit Trips Conversion Factor	3.5%	3.5%	3.5%
<b>Total Transit Trips of Proposed Project</b>	<b>10</b>	<b>9</b>	<b>95</b>

As shown in Table 36, based on the CMP guidelines, and the proximity of the various project land uses in relation to available transit in the project vicinity, the proposed project is forecast to generate approximately 10 a.m. peak hour transit trips, approximately 9 p.m. peak hour transit trips, and approximately 95 daily transit trips.

## **STATE HIGHWAY ANALYSIS**

The purpose of the Caltrans *Guide for the Preparation of Traffic Impact Studies (State of California Department of Transportation, December 2002)* is to provide a safe and efficient State transportation system, provide consistency and uniformity in the identification of traffic impacts generated by local land use proposals, and consistency and equity in the identification of measures to mitigate the traffic impacts generated by land use proposals. The Caltrans traffic studies guide identifies review of substantial individual projects, which might on their own impact the CMP State Highway transportation system.

### **State Highway Intersection Analysis Methodology**

Caltrans advocates use of Highway Capacity Manual (HCM) intersection analysis methodology to analyze the operation of study intersections. The HCM analysis methodology describes the operation of an intersection using a range of LOS from LOS A (free-flow conditions) to LOS F (severely congested conditions), based on the corresponding stopped delay experienced per vehicle as shown in Table 37.

**Table 37**  
**State Highway LOS & Delay Ranges for Signalized Intersections**

LOS	Delay (in seconds)
	Signalized Intersections
A	≤ 10.0
B	> 10.0 to ≤ 20.0
C	> 20.0 to ≤ 35.0
D	> 35.0 to ≤ 55.0
E	> 55.0 to ≤ 80.0
F	> 80.0

**Source:** Transportation Research Board, *Highway Capacity Manual, HCM 2000 Edition* (Washington D.C., 2000).

Level of service at signalized intersections is based on the average stopped delay per vehicle for all movements. The Caltrans goal for peak hour intersection operation is LOS C or better.

**State Highway Intersection Threshold of Significance**

While Caltrans has not established traffic thresholds of significance, this traffic analysis utilizes the following traffic threshold of significance:

- A significant project impact occurs at a State Highway study intersection when the addition of project-generated trips causes the peak hour level of service of the study intersection to change from acceptable operation (LOS A, B, or C) to deficient operation (LOS D, E or F).

**Existing Weekday AM & PM Peak Hour Conditions Intersection Level of Service**

Table 38 summarizes existing weekday conditions a.m. and p.m. peak hour LOS of the State Highway study intersections; detailed LOS analysis sheets are contained in Appendix B.

**Table 38**  
**State Highway Existing Weekday Conditions**  
**AM & PM Peak Hour Intersection LOS**

State Highway Study Intersection	Weekday AM Peak Hour (7 AM to 10 AM)	Weekday PM Peak Hour (4 PM to 6 PM)
	Delay - LOS	Delay - LOS
Western Ave (SR-213)/Toscanini Dr	17.5 – B	9.8 – A
Western Ave (SR-213)/Trudie Dr-Capitol Dr	23.0 – C	22.4 – C
Western Ave (SR-213)/Crestwood St	17.4 – B	13.6 – B
Western Ave (SR-213)/1 <sup>st</sup> St	<b>68.9 – E</b>	<b>86.6 – F</b>
Western Ave (SR-213)/9 <sup>th</sup> St	21.8 – C	23.4 – C
Western Ave (SR-213)/25 <sup>th</sup> St	25.6 – C	24.9 – C

**Note:** Delay is shown in seconds.

As shown in Table 38, the Western Avenue (SR-213)/1<sup>st</sup> Street intersection is currently operating at a deficient LOS (LOS D or worse) according to Caltrans performance criteria for weekday conditions.

**Existing Plus Marymount Project Weekday AM & PM Peak Hour Conditions Intersection Level of Service**

Table 39 summarizes existing plus Marymount project weekday conditions a.m. and p.m. peak hour LOS of the State Highway study intersections; detailed LOS analysis sheets are contained in Appendix B.

**Table 39  
State Highway  
Existing Plus Marymount Project Weekday  
Conditions AM & PM Peak Hour Intersection LOS**

State Highway Study Intersection	Existing Weekday Conditions		Existing Plus Marymount Project Weekday Conditions		Significant Impact?
	AM Peak Hour (7 AM to 10 AM)	PM Peak Hour (4 PM to 6 PM)	AM Peak Hour (7 AM to 10 AM)	PM Peak Hour (4 PM to 6 PM)	
	Delay - LOS	Delay - LOS	Delay - LOS	Delay - LOS	
Western Ave (SR-213)/Toscanini Dr	17.5 – B	9.8 – A	17.4 – B	9.8 – A	No
Western Ave (SR-213)/Trudie Dr-Capitol Dr	23.0 – C	22.4 – C	24.0 – C	23.7 – C	No
Western Ave (SR-213)/Crestwood St	17.4 – B	13.6 – B	17.5 – B	13.6 – B	No
Western Ave (SR-213)/1 <sup>st</sup> St	<b>68.9 – E</b>	<b>86.6 – F</b>	<b>67.9 – E</b>	<b>88.8 – F</b>	No
Western Ave (SR-213)/9 <sup>th</sup> St	21.8 – C	23.4 – C	21.8 – C	23.5 – C	No
Western Ave (SR-213)/25 <sup>th</sup> St	25.6 – C	24.9 – C	25.8 – C	25.0 – C	No

**Note:** Delay is shown in seconds.

As shown in Table 39, with the addition of project-generated trips, the Western Avenue (SR-213)/1<sup>st</sup> Street intersection is forecast to continue to operate at a deficient LOS (LOS D or worse) according to Caltrans performance criteria for existing plus Marymount project weekday conditions.

As also shown in Table 39, the addition of project-generated trips is forecast to result in no significant impacts at the State Highway study intersections for existing plus Marymount project weekday conditions.

**Forecast Year 2012 Without Project Weekday AM & PM Peak Hour Conditions Intersection Level of Service**

Table 40 summarizes forecast year 2012 without project weekday conditions a.m. and p.m. peak hour LOS of the State Highway study intersections; detailed LOS analysis sheets are contained in Appendix B.

**Table 40**  
**State Highway Forecast Year 2012 Without Project**  
**Weekday Conditions AM & PM Peak Hour Intersection LOS**

State Highway Study Intersection	Weekday AM Peak Hour (7 AM to 10 AM)	Weekday PM Peak Hour (4 PM to 6 PM)
	Delay - LOS	Delay - LOS
Western Ave (SR-213)/Toscanini Dr	18.3 – B	10.3 – B
Western Ave (SR-213)/Trudie Dr-Capitol Dr	34.9 – C	<b>35.9 – D</b>
Western Ave (SR-213)/Crestwood St	17.2 – B	21.9 – C
Western Ave (SR-213)/1 <sup>st</sup> St	<b>86.4 – F</b>	<b>117.6 – F</b>
Western Ave (SR-213)/9 <sup>th</sup> St	22.2 – C	24.5 – C
Western Ave (SR-213)/25 <sup>th</sup> St	27.3 – C	27.0 – C

**Note:** Delay is shown in seconds.

As shown in Table 40, the Western Avenue (SR-213)/1<sup>st</sup> Street intersection is forecast to operate at a deficient LOS (LOS D or worse) according to Caltrans performance criteria for forecast year 2012 without project weekday conditions.

**Forecast Year 2012 With Project Weekday AM & PM Peak Hour Conditions Intersection Level of Service**

Table 41 summarizes forecast year 2012 with project weekday conditions a.m. and p.m. peak hour LOS of the State Highway study intersections; detailed LOS analysis sheets are contained in Appendix B.

**Table 41**  
**State Highway**  
**Forecast Year 2012 With Project Weekday**  
**Conditions AM & PM Peak Hour Intersection LOS**

State Highway Study Intersection	Forecast Year 2012 Without Project Weekday Conditions		Forecast Year 2012 With Project Weekday Conditions		Significant Impact?
	AM Peak Hour (7 AM to 10 AM)	PM Peak Hour (4 PM to 6 PM)	AM Peak Hour (7 AM to 10 AM)	PM Peak Hour (4 PM to 6 PM)	
	Delay - LOS	Delay - LOS	Delay - LOS	Delay - LOS	
Western Ave (SR-213)/Toscanini Dr	18.3 – B	10.3 – B	18.3 – B	10.3 – B	No
Western Ave (SR-213)/Trudie Dr-Capitol Dr	34.9 – C	<b>35.9 – D</b>	<b>38.2 – D</b>	<b>40.2 – D</b>	Yes
Western Ave (SR-213)/Crestwood St	17.2 – B	21.9 – C	17.3 – B	22.1 – C	No
Western Ave (SR-213)/1 <sup>st</sup> St	<b>86.4 – F</b>	<b>117.6 – F</b>	<b>84.8 – F</b>	<b>120.1 – F</b>	No
Western Ave (SR-213)/9 <sup>th</sup> St	22.2 – C	24.5 – C	22.3 – C	24.6 – C	No
Western Ave (SR-213)/25 <sup>th</sup> St	27.3 – C	27.0 – C	27.7 – C	27.2 – C	No

**Note:** Delay is shown in seconds.

As shown in Table 41, with the addition of project-generated trips, the following study intersections are forecast to operate at a deficient LOS (LOS D or worse) according to Caltrans performance criteria for forecast year 2012 with project weekday conditions:

- Western Avenue (SR-213)/Trudie Drive-Capitol Drive (both a.m. and p.m. peak hour); and
- Western Avenue (SR-213)/1<sup>st</sup> Street (both a.m. and p.m. peak hour).

As also shown in Table 41, the addition of project-generated trips is forecast to result in a significant impact at the Western Avenue (SR-213)/Trudie Drive-Capitol Drive intersection for forecast year 2012 with project weekday conditions.

### **Forecast Year 2012 With Project Conditions Recommended Mitigation Measures**

The following feasible mitigation measure is recommended to eliminate significant State Highway traffic impacts for forecast year 2012 with project conditions:

#### **Mitigation Measure No. 8:**

**Western Avenue (SR-213)/Trudie Drive-Capitol Drive** – Prior to issuance of any Certificate of Occupancy, the applicant shall implement the re-striping of the eastbound Trudie Drive approach from one shared left-turn/through lane and one de-facto right-turn lane to consist of one left-turn lane and one shared through/right-turn lane. Implementation of this mitigation measure likely requires coordination with City of Los Angeles and Caltrans staff (*Same recommendation as mitigation measure 2 identified for existing plus Marymount project conditions*).

Assuming modifications to the project description as described, Mitigation Measure 8 is the same as that identified in the *Marymount College Facilities Expansion Project Traffic & Parking Impact Analysis (September 28, 2007)* , and the *Revised Marymount College Project Traffic & Parking Analysis (May 15, 2009)*.

### **Mitigated Forecast Year 2012 With Project Weekday AM & PM Conditions Peak Hour Intersection Level of Service**

Table 42 summarizes forecast year 2012 with project weekday conditions a.m. and p.m. peak hour LOS of the Western Avenue (SR-213)/Trudie Drive-Capitol Drive intersection assuming full implementation of the recommended mitigation measure; detailed LOS analysis sheets are contained in Appendix B.

**Table 42**  
**State Highway**  
**Mitigated Forecast Year 2012 With Project Weekday**  
**Conditions AM & PM Peak Hour Intersection LOS**

State Highway Study Intersection	Forecast Year 2012 Without Project Weekday Conditions		Mitigated Forecast Year 2012 With Project Weekday Conditions		Significant Impact?
	AM Peak Hour (7 AM to 10 AM)	PM Peak Hour (4 PM to 6 PM)	AM Peak Hour (7 AM to 10 AM)	PM Peak Hour (4 PM to 6 PM)	
	Delay - LOS	Delay - LOS	Delay - LOS	Delay - LOS	
Western Ave (SR-213)/Trudie Dr-Capitol Dr	34.9 – C	<b>35.9 – D</b>	28.0 – C	32.9 – C	No

**Note:** Delay is shown in seconds.

As shown in Table 42, no significant impacts are forecast to occur at the State Highway Western Avenue (SR-213)/Trudie Drive-Capitol Drive intersection assuming full implementation of the recommended mitigation measure (8) for the forecast year 2012 with project conditions a.m. and p.m. peak hour.

## 2. PARKING ANALYSIS

Parking operations associated with the project modifications are analyzed to determine if impacts are greater, the same, or less than those identified in the *Marymount College Facilities Expansion Project Traffic & Parking Impact Analysis (September 28, 2007)*, and the *Revised Marymount College Project Traffic & Parking Analysis (May 15, 2009)*. The analysis includes review of parking capacity required based on City of Palos Verdes Parking Code, as well as based on observed college-related parking counts, including forecast demand associated with the two proposed project components (i.e., campus facilities and BA Program).

It should be noted, City Municipal Code for college/university does not differentiate between AA and BA college programs.

### Existing On-Site Parking Required According to City Code

Table 43 summarizes the parking capacity required according to City of Rancho Palos Verdes Parking Code (RPVMC Section 17.50.020) to accommodate current on-site land uses based on the following existing conditions:

- Maximum student enrollment of 793 students;
- 215 employees and faculty members; and
- 648 student seats provided on campus.

**Table 43  
Forecast Existing Parking Spaces Required Per City Code**

City Parking Code Requirement	Existing Marymount College Conditions	
	Quantity	Parking Spaces Required
College and University		
- 1 Space per 2 Regularly Enrolled Students	793 Regularly Enrolled Students	397
- 1 Space per 2 Employees/Faculty	215 Employees/Faculty	108
- 1 space per 5 Student Seats	648 Student Seats	130
<b>Total Parking spaces Required for Existing Marymount College Conditions</b>		<b>635</b>

**Note:** City Municipal Code does not differentiate between AA and BA college programs

As shown in Table 43, according to City of Rancho Palos Verdes Parking Code, 635 parking spaces are currently required to accommodate the existing Marymount College parking demand.

Table 44 summarizes the current parking capacity required according to City code versus parking spaces provided at the Marymount College.

**Table 44  
Review of Existing Parking Spaces Adequacy Based on City Code**

	Existing Marymount College Conditions
Forecast Parking Spaces Required Per City Code	635 spaces <sup>1</sup>
Parking Spaces Provided	343 existing spaces
Forecast Surplus/Deficient Parking Spaces Provided	-292 spaces
Sufficient Parking Spaces Provided?	No
Observed Overflow Parking Demand on Adjacent Streets	49 vehicles <sup>2</sup>

1 = Based on 793 regularly enrolled students.

2 = Based on Fall 2005 parking demand counts at 2:00 p.m.

As shown in Table 44, since the Marymount College currently provides 343 parking spaces, a 292 parking space deficiency currently exists based on City of Rancho Palos Verdes Parking Code. It should be noted, while parking spaces required by City code indicated a potential deficiency of 292 parking spaces, only 49 college-related vehicles were observed to park on the street during the weekday peak parking demand between 2:00 p.m. and 3:00 p.m. when 54 parking spaces were unoccupied on-campus

**EXISTING PLUS MARYMOUNT PROJECT CONDITIONS**

This section reviews parking capacity required for the proposed project based on City of Rancho Palos Verdes Parking Code as well as based on observed College-related parking counts.

### Existing Plus Marymount Project On-Site Parking Required According to City Code

Table 45 summarizes the parking capacity required according to City of Rancho Palos Verdes Parking Code to accommodate existing College uses as well as proposed project. As previously noted, City Municipal Code does not differentiate between AA and BA college programs; therefore, the tables in this section apply to both project conditions.

**Table 45  
Forecast Parking Spaces Required Per City Code**

City Parking Code Requirement	Existing Marymount College Conditions		Proposed Expansion Project		Existing With Expansion Project Marymount College	
	Quantity	Parking Spaces Required	Quantity	Parking Spaces Required	Quantity	Parking Spaces Required
College						
- 1 Space per 2 Students	793 Stu	397	0 New Stu	0	793 Stu	397
- 1 Space per 2 E/F	215 E/F	108	7 New E/F	4	222 E/F	112
- 1 Space per 5 Stu Seats	648 Stu Seats	130	5 Net Stu Seats	1	653 Stu Seats	131
<b>Parking Spaces Required</b>		<b>635</b>		<b>5</b>		<b>640</b>

**Note:** Stu = Students; E/F = Employees/Faculty.

As shown in Table 45, according to the City of Rancho Palos Verdes Parking Code, 5 additional parking spaces would be required to accommodate the proposed project parking demand. As also shown in Table 45, according to the City of Rancho Palos Verdes Parking Code, 640 parking spaces would be required to accommodate the entire Marymount College parking demand assuming completion of the proposed project.

Table 46 summarizes the parking capacity required based on City code versus the number of parking spaces planned to be provided at the Marymount College assuming completion of the proposed project.

**Table 46**  
**Review of parking Spaces Adequacy Based on City Code**

	<b>Existing Marymount College Conditions</b>	<b>Proposed Expansion Project</b>	<b>Existing With Expansion Project Marymount College</b>
Forecast Parking Spaces Required	635 spaces <sup>1</sup>	5 additional spaces	640 total spaces <sup>1</sup>
Parking Spaces Provided	343 existing spaces	120 additional spaces	463 total spaces
Forecast Surplus/Deficient Parking Spaces Provided	-292 spaces	+115 spaces	-177 spaces
Sufficient Parking Spaces Provided?	No	Yes	No
Observed Overflow Parking Demand on Adjacent Streets	49 vehicles <sup>2</sup>	N/A	N/A

**Note:** N/A = Not Applicable.

1 = Based on 793 regularly enrolled students.

2 = Based on Fall 2005 parking demand counts at 2:00 p.m.

As shown in Table 46, since the proposed project is planned to add 120 parking spaces, a surplus of 115 parking spaces is forecast to occur based on City of Rancho Palos Verdes Parking Code. As also shown in Table 46, since the entire Marymount College assuming completion of the proposed project is planned to provide 463 parking spaces, a 177 parking space deficiency is forecast to occur based on City of Rancho Palos Verdes Parking Code.

Since parking deficiencies are forecast to occur for existing and future conditions based on calculations using City code rather than observed parking counts, an alternate parking analysis has been prepared to more accurately portray future parking conditions assuming completion of the proposed project. It is worth noting, parking required based on City code indicates 640 parking spaces may be required and is based on the strict interpretation of code.

Table 47 summarizes the Marymount College-related weekday and Saturday peak hour parking ratio for existing conditions, based on observed parking demand during the Fall 2005 semester when weekday student enrollment was 658 students and weekend student enrollment was 80 students.

**Table 47**  
**Existing Marymount College Peak Hour Parking Ratio**

<b>Parking Component</b>	<b>Weekday</b>	<b>Saturday</b>
Observed Peak Hour Parking Demand	372 vehicles <sup>1</sup> (11 AM)	87 vehicles <sup>1</sup> (12 PM)
Student Enrollment (Fall 2005)	658 Students	738 Students <sup>2</sup>
<b>Peak Hour Parking Ratio (demand/student)</b>	0.57 parked vehicles/student	0.12 parked vehicles/student

1 = Based on Fall 2005 parking demand counts.

2 = 738 students accounts for 658 weekday enrolled students and 80 weekend enrolled students.

As shown in Table 47, the existing peak parking ratio at Marymount College is 0.57 parked vehicles/student during weekday conditions and 0.12 parked vehicles/student during Saturday conditions. It is noted the parking ratio identified above assumes all on-street parking associated with Marymount College is included, and therefore, forecast demand using these ratios assume all Marymount College-related on-street parking activity is relocated on-campus.

The parking ratios presented above reflect parking activity for a junior college (AA Program) and require an adjustment when accounting for University (BA Program) students parking behavior. *ITE* provides weekday parking rates for University and Junior College land uses, with a higher rate for University students. To account for higher forecast parking demand by University (BA Program) students, a multiplier is derived that will be utilized in the parking demand calculations.

The derivation of the multiplier to account for higher weekday parking demand by students in the BA Program is shown in Table 48..

**Table 48**  
**University to Junior College Weekday Peak Hour Parking Rate Multiplier**

Parking Component	Weekday
<i>ITE</i> University Parking Rate (BA Program)	0.30 vehicles/student population
<i>ITE</i> Junior College Parking Rate (AA Program)	0.21 vehicles/student population
<b><i>ITE</i> University to Junior College Peak Hour Parking Rate Multiplier</b>	1.43 veh/student population

**Source:** *ITE Parking Generation, (3<sup>d</sup> Edition, 2004).*

As shown Table 48, the multiplier to account for higher weekday parking demand by students in the BA Program is derived as 1.43. Since *ITE* parking rate data is not available for weekend conditions, this analysis conservatively assumes use of the weekday peak hour parking multiplier when calculating weekend student parking demand.

**Forecast Existing Plus Marymount Project Parking Demand**

Forecast parking demand for forecast existing plus Marymount project weekday and Saturday conditions has been prepared utilizing the following assumptions:

- Maximum weekday student enrollment is 793 students (based on the formula allowing 750 full-time students, 20 part-time students, and a marginal difference of 3.0 percent);
- Up to 250 students are enrolled in the BA Program, remaining balance of student enrollment would be in AA Program;
- Maximum weekend student enrollment is 150 students (based on modified project description) with no weekend students enrolled in the BA Program;
- Students would park at the campus based on the observed vehicle to student peak parking ratios with application of multiplier to account for BA Program student parking activity where applicable;

- Calculations of parking spaces required assumes no Marymount College-related parking on adjacent streets, i.e., all on-street parking demand is relocated to on-campus parking areas;
- Parking spaces required for new student seats is based on City of Rancho Palos Verdes Parking Code for colleges and universities;
- Addition of 7 new security, custodial, and maintenance staff; and
- The cumulative projects identified within the traffic analysis section are not forecast to increase parking demand at the parking study area.

Table 49 summarizes the forecast parking capacity required for existing plus Marymount project weekday peak hour conditions assuming a maximum weekday enrollment of 793 students, based on the observed weekday parking ratio and City of Rancho Palos Verdes Parking Code.

**Table 49  
Forecast Weekday Parking Demand Based  
on Observed Parking Ratio and City Code**

Parking Component	Peak Hour Parking Space Demand
7 New Employees/Faculty <sup>1</sup>	4
793 Students	
- 543 AA Program students * 0.57 parked vehicles/student	310
- 250 BA Program students * 0.57 parked vehicles/student * 1.43 multiplier	204
5 Net New Student Seats (City Code: 1 parking space per 5 student seats) <sup>2</sup>	1
Forecast Parking Spaces Required	519
Parking Spaces Provided (343 existing + 120 added by proposed project) <sup>3</sup>	463
<b>Project Parking Surplus/Deficiency</b>	<b>-56</b>

1 = Based on City of Rancho Palos Verdes Parking Code for Colleges and Universities for employee/faculty category.

2 = Based on City of Rancho Palos Verdes Parking Code for Colleges and Universities.

3 = Based on site plan (Rasmussen and Associates, November 2005).

As shown in Table 49, since the proposed project is planned to add 120 parking spaces to the existing 343 parking spaces, a 56 parking space deficiency is forecast to occur during the weekday peak hour based on the observed weekday parking ratio and City of Rancho Palos Verdes Parking Code.

Table 50 summarizes the forecast parking capacity required for existing plus Marymount project Saturday peak hour conditions assuming a maximum weekend enrollment of 150 students, based on the observed Saturday parking ratio and City of Rancho Palos Verdes Parking Code. Since weekday enrolled students utilize the campus during weekend conditions, the 793 maximum enrolled weekday students are included in the Table 50 Saturday parking demand forecast.

**Table 50  
Forecast Saturday Parking Demand Based  
on Observed Parking Ratio and City Code**

Parking Component	Peak Hour Parking Space Demand
7 New Employees/Faculty <sup>1</sup>	4
943 Students <sup>2</sup>	
- 693 AA Program students * 0.12 parked vehicles/student	83
- 250 BA Program students * 0.12 parked vehicles/student * 1.43 multiplier	43
5 Net New Student Seats (City Code: 1 parking space per 5 student seats) <sup>3</sup>	<u>1</u>
Forecast Parking Spaces Required	131
Parking Spaces Provided (343 existing + 120 added by proposed project) <sup>4</sup>	<u>463</u>
<b>Proposed Project Parking Surplus/Deficiency</b>	<b>+332</b>

1 = Based on City of Rancho Palos Verdes Parking Code for Colleges and Universities for employee/faculty category.

2 = 943 students not living on-campus accounts for 793 weekday enrolled students and 150 weekend enrolled students.

3 = Based on City of Rancho Palos Verdes Parking Code for Colleges and Universities.

4 = Based on site plan (Rasmussen and Associates, November 2005).

As shown in Table 50, since the proposed project is planned to add 120 parking spaces to the existing 343 parking spaces, a 332 parking space surplus is forecast to occur during the Saturday peak hour based on the observed Saturday parking ratio and City of Rancho Palos Verdes Parking Code.

### **Parking Mitigation Measures**

The following mitigation measures have been identified to reduce weekday parking demand associated with Marymount College assuming implementation of the proposed project:

#### **Mitigation Measure 9:**

Prior to issuance of any Certificate of Occupancy, the applicant shall institute, to the satisfaction of the Director of Planning, Building, and Code Enforcement and the Public Works Director, parking management strategies to reduce weekday College-related parking demand by the following values:

- 11 percent or greater for student enrollment between 744 and 793;
- 6 percent or greater for student enrollment between 694 and 743;
- 0 percent or greater for student enrollment of 693 or less.

Potential parking management strategies may include, but are not limited to, the following:

- Provision of “carpool only” parking spaces;
- Implementation of parking pricing for campus parking permits;
- Utilization of remote parking;
- Provision of increased shuttle services;

- Offering financial incentives; and
- Implementation of restrictions on parking allowed by residents of the Palos Verdes North Facility.

Marymount College shall provide, on an annual basis, a Parking Management Strategy Program documenting prior-year achieved parking demand reductions and strategies for use in upcoming academic school year. Marymount College shall annually submit the Parking Management Strategy Program to the Department of Planning, Building, and Code Enforcement by July 1<sup>st</sup> for review by the Director of Planning, Building, and Code Enforcement. The Parking Management Strategy Program shall be modified on an as needed basis, as deemed necessary by the Director of Planning, Building, and Code Enforcement.

**Mitigation Measure 10:**

The parking impacts and corresponding mitigation measures assume the Marymount College student enrollment as a maximum of 793 weekday students (based on the formula allowing 750 full-time students, 20 part-time students, and a marginal difference of 3.0 percent) and 150 weekend students. Additionally, it is assumed, Marymount College student enrollment as a maximum of 250 weekday students enrolled in the BA Program and a maximum of 793 weekday students minus current BA Program weekday students enrolled in the AA Program. Therefore, prior to issuance of any Certificate of Occupancy, student enrollment shall be limited to a maximum of 793 weekday students and 150 weekend students, including full- and part-time students, and maximum of 250 weekday students enrolled in the BA Program and a maximum of 793 weekday students minus current BA Program weekday students enrolled in the AA Program.

**MITIGATED EXISTING PLUS MARYMOUNT PROJECT PARKING CONDITIONS**

This analysis conservatively assumes no reduction of forecast trip generation of the proposed project associated with reduced traffic and parking related to parking management strategies.

Table 51 summarizes the mitigated forecast parking capacity required for existing plus Marymount project weekday conditions based on the observed parking ratio as well as proposed project improvements.

**Table 51  
Mitigated Forecast Parking Demand Based  
on Observed Parking Ratio and City Code**

Parking Component	Peak Hour Parking Space Demand
7 New Employees/Faculty <sup>1</sup>	4
793 Students - 543 AA Program students * 0.57 parked vehicles/student - 250 BA Program students * 0.57 parked vehicles/student * 1.43 multiplier	310 204
5 Net New Student Seats (City Code: 1 parking space per 5 student seats) <sup>2</sup>	1
Subtotal Forecast Parking Spaces Required	519
Mitigation Measure: Parking Management Strategy (11% Reduction applied to 519 demand)	<u>- 57</u>
Total Forecast Parking Spaces Required	462
Parking Spaces Provided (343 existing + 120 added by proposed project) <sup>3</sup>	<u>463</u>
<b>Parking Surplus/Deficiency</b>	<b>+1</b>

1 = Based on City of Rancho Palos Verdes Parking Code for Colleges and Universities for employee/faculty category.

2 = Based on City of Rancho Palos Verdes Parking Code for Colleges and Universities.

3 = Based on site plan (Rasmussen and Associates, November 2005).

As shown in Table 51, assuming implementation of the recommended mitigation measures (9 and 10), a parking surplus of one (1) space is forecast to occur during the weekday peak hour based on the observed weekday parking ratio and City of Rancho Palos Verdes Parking Code for project conditions.

### 3. MITIGATION MEASURES

To reduce project traffic impacts to a level considered less than significant for existing plus Marymount project conditions, the following mitigation measures are recommended:

#### Mitigation Measure No. 1:

**Palos Verdes Drive East/Miraleste Drive** – Prior to issuance of any Certificate of Occupancy, the applicant shall signalize the intersection. Additionally, the applicant shall design the Palos Verdes Drive East/Miraleste Drive intersection traffic signal to include a westbound right-turn overlap, which will preclude u-turn movement from southbound to northbound Palos Verdes Drive East. The applicant may be eligible for reimbursement from future projects that result in impacts at this intersection.

#### Mitigation Measure No. 2:

**Western Avenue (SR-213)/Trudie Drive-Capitol Drive** – Prior to issuance of any Certificate of Occupancy, the applicant shall implement the re-stripping of the eastbound Trudie Drive approach from one shared left-turn/through

lane and one de-facto right-turn lane to consist of one left-turn lane and one shared through/right-turn lane. Implementation of this mitigation measure likely requires coordination with City of Los Angeles and Caltrans staff. The applicant may be eligible for reimbursement from future projects that result in impacts at this intersection.

**Mitigation Measure No. 3:**

The traffic impacts and corresponding mitigation measures assume the Marymount College student enrollment as a maximum of 793 weekday students (based on the formula allowing 750 full-time students, 20 part-time students, and a marginal difference of 3.0 percent) and 150 weekend students. Additionally, it is assumed, Marymount College student enrollment as a maximum of 250 weekday students enrolled in the BA Program and a maximum of 793 weekday students minus current BA Program weekday students enrolled in the AA Program. Therefore, prior to issuance of any Certificate of Occupancy, student enrollment shall be limited to a maximum of 793 weekday students and 150 weekend students, including full- and part-time students, and maximum of 250 weekday students enrolled in the BA Program and a maximum of 793 weekday students minus current BA Program weekday students enrolled in the AA Program.

Assuming modifications to the project description as described, Mitigation Measures 1 through 3 are the same as those identified in the *Marymount College Facilities Expansion Project Traffic & Parking Impact Analysis (September 28, 2007)*.

To reduce project traffic impacts to a level considered less than significant for forecast year 2012 with project conditions, the following mitigation measures are recommended:

**Mitigation Measure No. 4:**

**Palos Verdes Drive East/Miraleste Drive** – Prior to issuance of any Certificate of Occupancy, the applicant shall signalize the intersection. Additionally, the applicant shall design the Palos Verdes Drive East/Miraleste Drive intersection traffic signal to include a westbound right-turn overlap, which will preclude u-turn movement from southbound to northbound Palos Verdes Drive East. The applicant may be eligible for reimbursement from future projects that result in impacts at this intersection (*Same recommendation as mitigation measure 1 identified for existing plus Marymount project conditions*).

**Mitigation Measure No. 5:**

**Palos Verdes Drive East/Palos Verdes Drive South** – Prior to issuance of any Certificate of Occupancy, the applicant shall make a proportionate share contribution to implement the modification of the intersection to provide a two-stage gap acceptance design for southbound left-turning vehicles. The applicant shall make a proportionate share contribution to implement the construction of a raised median refuge area for vehicles to turn left from Palos Verdes Drive East to cross westbound Palos Verdes Drive South while waiting for a gap in eastbound traffic to complete the turn to eastbound Palos Verdes Drive South. Additionally, the applicant shall make a proportionate share contribution to implement the narrowing of the existing raised median to provide an acceleration lane along Palos Verdes Drive South to

accommodate vehicles accelerating to join eastbound Palos Verdes Drive South traffic flow. Modifications to the Palos Verdes Drive East/Palos Verdes Drive South intersection shall be designed taking into account truck turning radius requirements and shall be to the satisfaction of the Public Works Director. Since the Palos Verdes Drive East/Palos Verdes Drive South intersection is impacted by the proposed project for cumulative with proposed project conditions, a proportionate share contribution by the project applicant is applicable.

**Mitigation Measure No. 6:**

**Western Avenue (SR-213)/Trudie Drive-Capitol Drive** – Prior to issuance of any Certificate of Occupancy, the applicant shall implement the re-striping of the eastbound Trudie Drive approach from one shared left-turn/through lane and one de-facto right-turn lane to consist of one left-turn lane and one shared through/right-turn lane. Implementation of this mitigation measure likely requires coordination with City of Los Angeles and Caltrans staff. The applicant may be eligible for reimbursement from future projects that result in impacts at this intersection (*Same recommendation as mitigation measure 2 identified for existing plus Marymount project conditions*).

**Mitigation Measure No. 7:**

The traffic impacts and corresponding mitigation measures assume the Marymount College student enrollment as a maximum of 793 weekday students (based on the formula allowing 750 full-time students, 20 part-time students, and a marginal difference of 3.0 percent) and 150 weekend students. Therefore, prior to issuance of any Certificate of Occupancy, student enrollment shall be limited to a maximum of 793 weekday students and 150 weekend students, including full- and part-time students (*Same recommendation as mitigation measure 3 identified for existing plus Marymount project conditions*).

Assuming modifications to the project description as described, Mitigation Measures 7 through 10 are the same as those identified in the *Marymount College Facilities Expansion Project Traffic & Parking Impact Analysis (September 28, 2007)*.

To reduce project traffic impacts to a level considered less than significant at the State Highway study intersection for forecast year 2012 with project conditions, the following mitigation measure is recommended:

**Mitigation Measure No. 8:**

**Western Avenue (SR-213)/Trudie Drive-Capitol Drive** – Prior to issuance of any Certificate of Occupancy, the applicant shall implement the re-striping of the eastbound Trudie Drive approach from one shared left-turn/through lane and one de-facto right-turn lane to consist of one left-turn lane and one shared through/right-turn lane. Implementation of this mitigation measure likely requires coordination with City of Los Angeles and Caltrans staff (*Same recommendation as mitigation measure 2 identified for existing plus Marymount project conditions*).

Assuming modifications to the project description as described, Mitigation Measure 16 is the same as that identified in the *Marymount College Facilities Expansion Project Traffic & Parking Impact Analysis (September 28, 2007)*.

To reduce project parking impacts to a level considered less than significant for existing plus Marymount project conditions, the following mitigation measures are recommended:

**Mitigation Measure No. 9:** Prior to issuance of any Certificate of Occupancy the applicant shall institute, to the satisfaction of the Director of Planning, Building, and Code Enforcement and the Public Works Director, parking management strategies to reduce weekday College-related parking demand by the following values:

- 11 percent or greater for student enrollment between 744 and 793;
- 6 percent or greater for student enrollment between 694 and 743;
- 0 percent or greater for student enrollment of 693 or less.

Potential parking management strategies may include, but are not limited to, the following:

- Provision of “carpool only” parking spaces;
- Implementation of parking pricing for campus parking permits;
- Utilization of remote parking;
- Provision of increased shuttle services;
- Offering financial incentives; and
- Implementation of restrictions on parking allowed by residents of the Palos Verdes North Facility.

Marymount College shall provide, on an annual basis, a Parking Management Strategy Program documenting prior-year achieved parking demand reductions and strategies for use in upcoming academic school year. Marymount College shall annually submit the Parking Management Strategy Program to the Department of Planning, Building, and Code Enforcement by July 1<sup>st</sup> for review by the Director of Planning, Building, and Code Enforcement. The Parking Management Strategy Program shall be modified on an as needed basis, as deemed necessary by the Director of Planning, Building, and Code Enforcement.

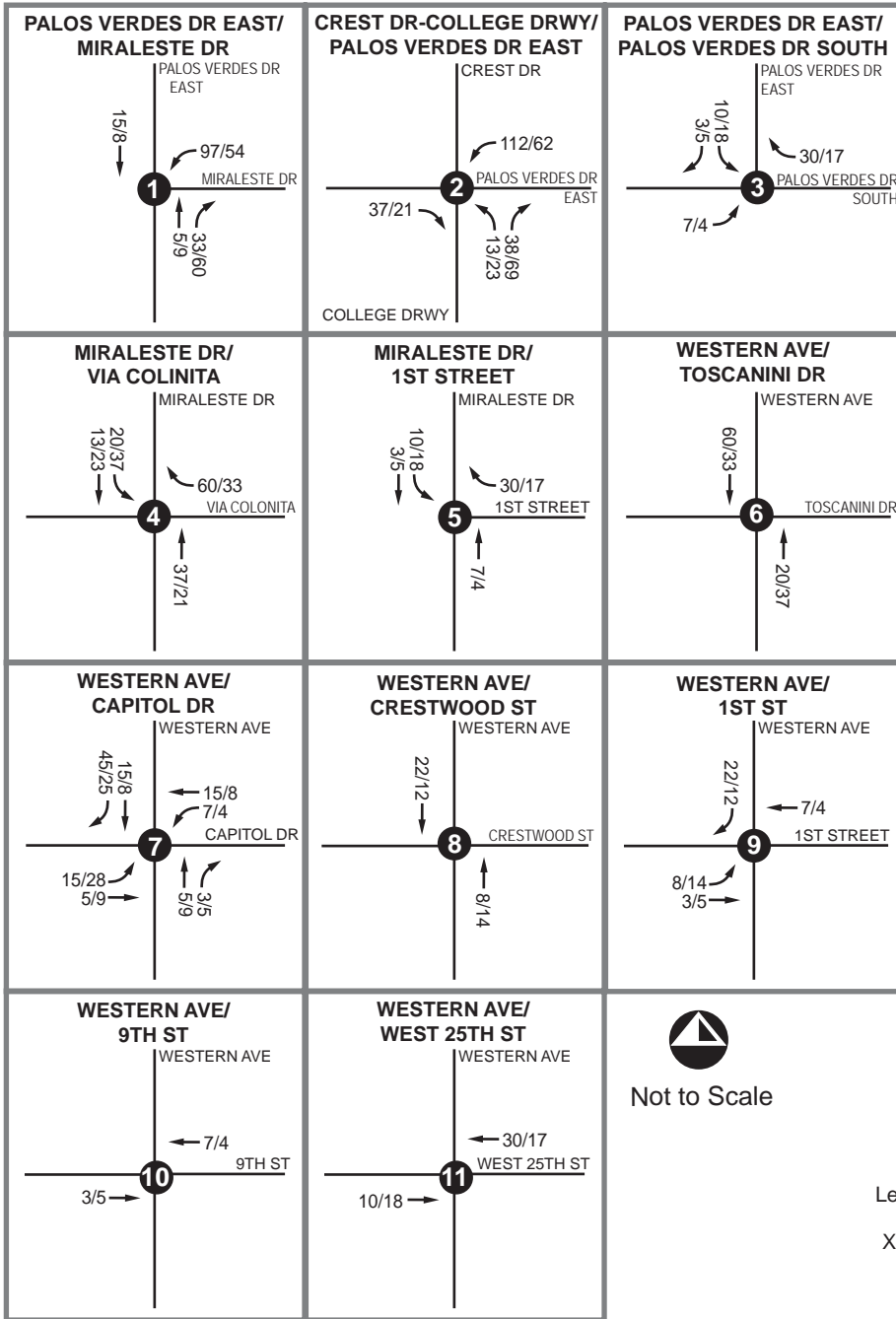
**Mitigation Measure No. 10:** The parking impacts and corresponding mitigation measures assume the Marymount College student enrollment as a maximum of 793 weekday students (based on the formula allowing 750 full-time students, 20 part-time students, and a marginal difference of 3.0 percent) and 150 weekend students. Additionally, it is assumed, Marymount College student enrollment as a maximum of 250 weekday students enrolled in the BA Program and a

maximum of 793 weekday students minus current BA Program weekday students enrolled in the AA Program. Therefore, prior to issuance of any Certificate of Occupancy, student enrollment shall be limited to a maximum of 793 weekday students and 150 weekend students, including full- and part-time students, and maximum of 250 weekday students enrolled in the BA Program and a maximum of 793 weekday students minus current BA Program weekday students enrolled in the AA Program.

#### **4. CONCLUSIONS**

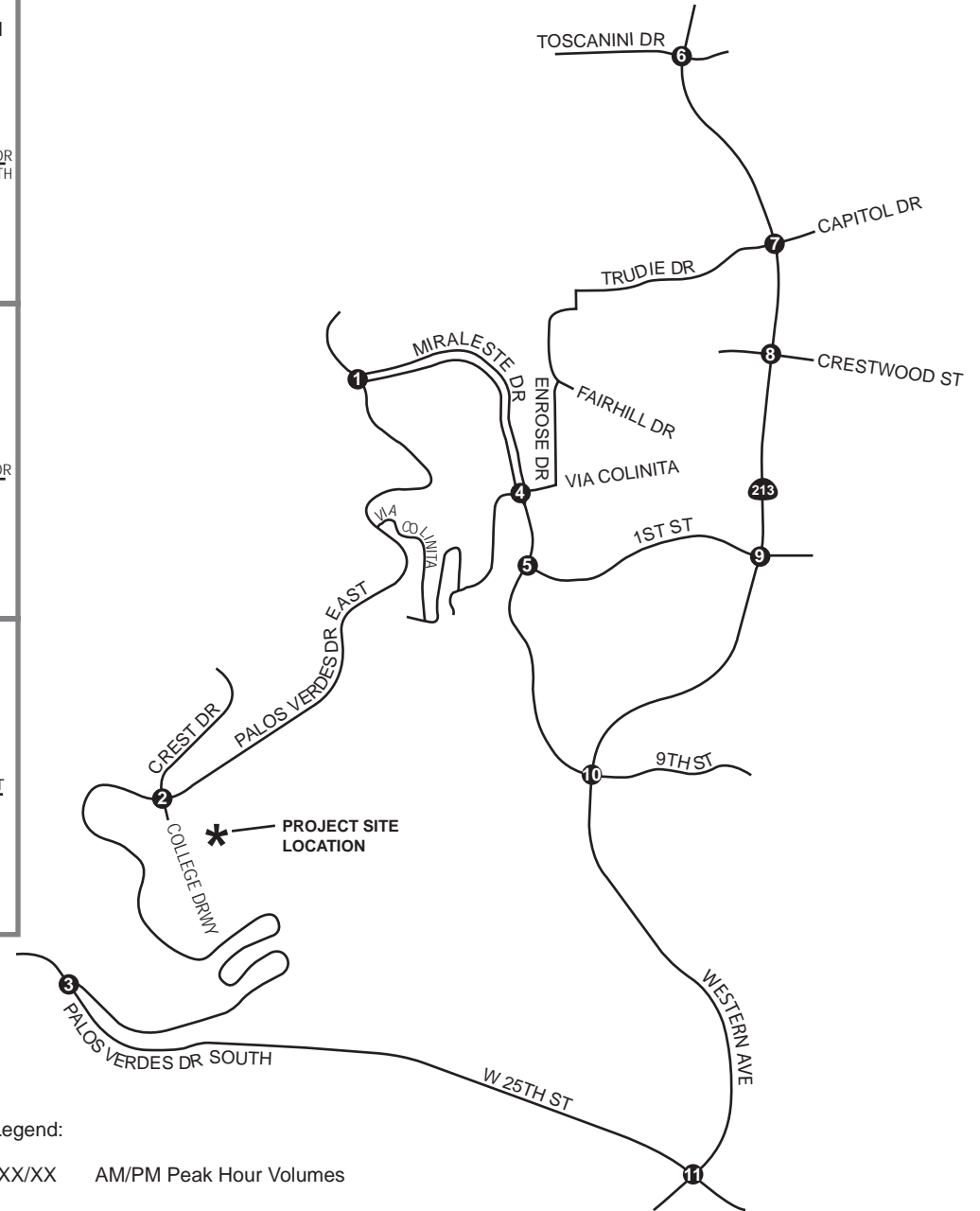
We have analyzed traffic and parking operations associated with modifications to the proposed project to determine if impacts are greater, the same, or less than those identified in the *Marymount College Facilities Expansion Project Traffic & Parking Impact Analysis (September 28, 2007)*. While forecast traffic generation increased for both weekday and Saturday conditions, the traffic impacts and associated mitigation measures remained the same as identified in the *Marymount College Facilities Expansion Project Traffic & Parking Impact Analysis (September 28, 2007)*, and the *Revised Marymount College Project Traffic & Parking Analysis (May 15, 2009)*. Since forecast parking demand increased with the project modifications, a weekday parking deficiency was identified, and therefore, two parking mitigation measures were identified, which are similar to mitigation identified in the *Marymount College Facilities Expansion Project Traffic & Parking Impact Analysis (September 28, 2007)*.

**APPENDIX A**  
**Volume Exhibits**

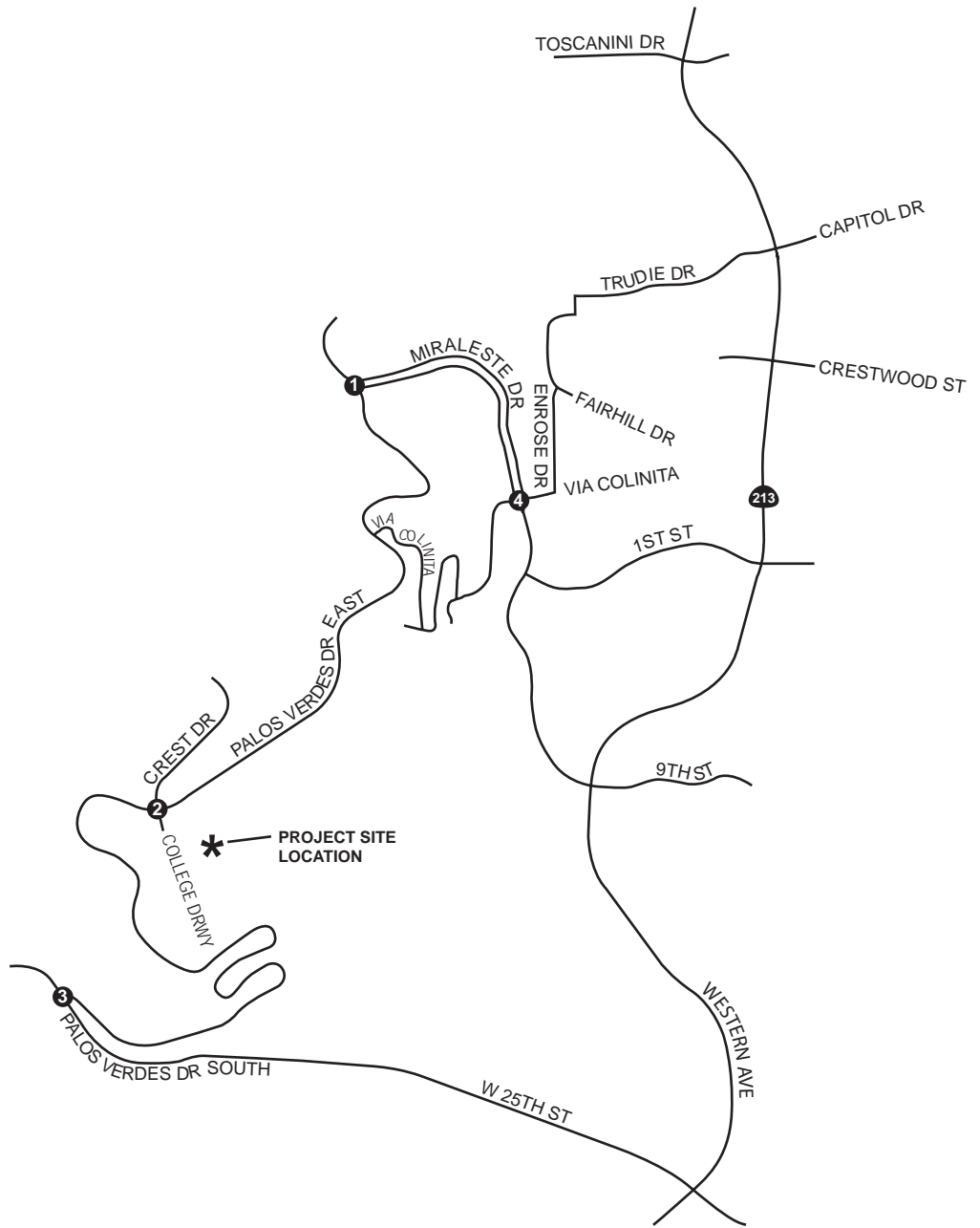
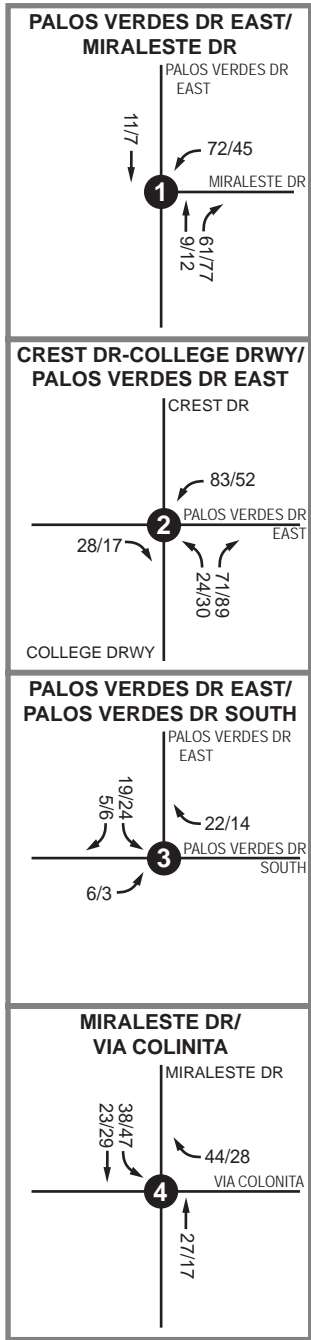


  
 Not to Scale

Legend:  
 XX/XX AM/PM Peak Hour Volumes



## Forecast Proposed Project Conditions Weekday AM & PM Peak Hour Trip Assignment

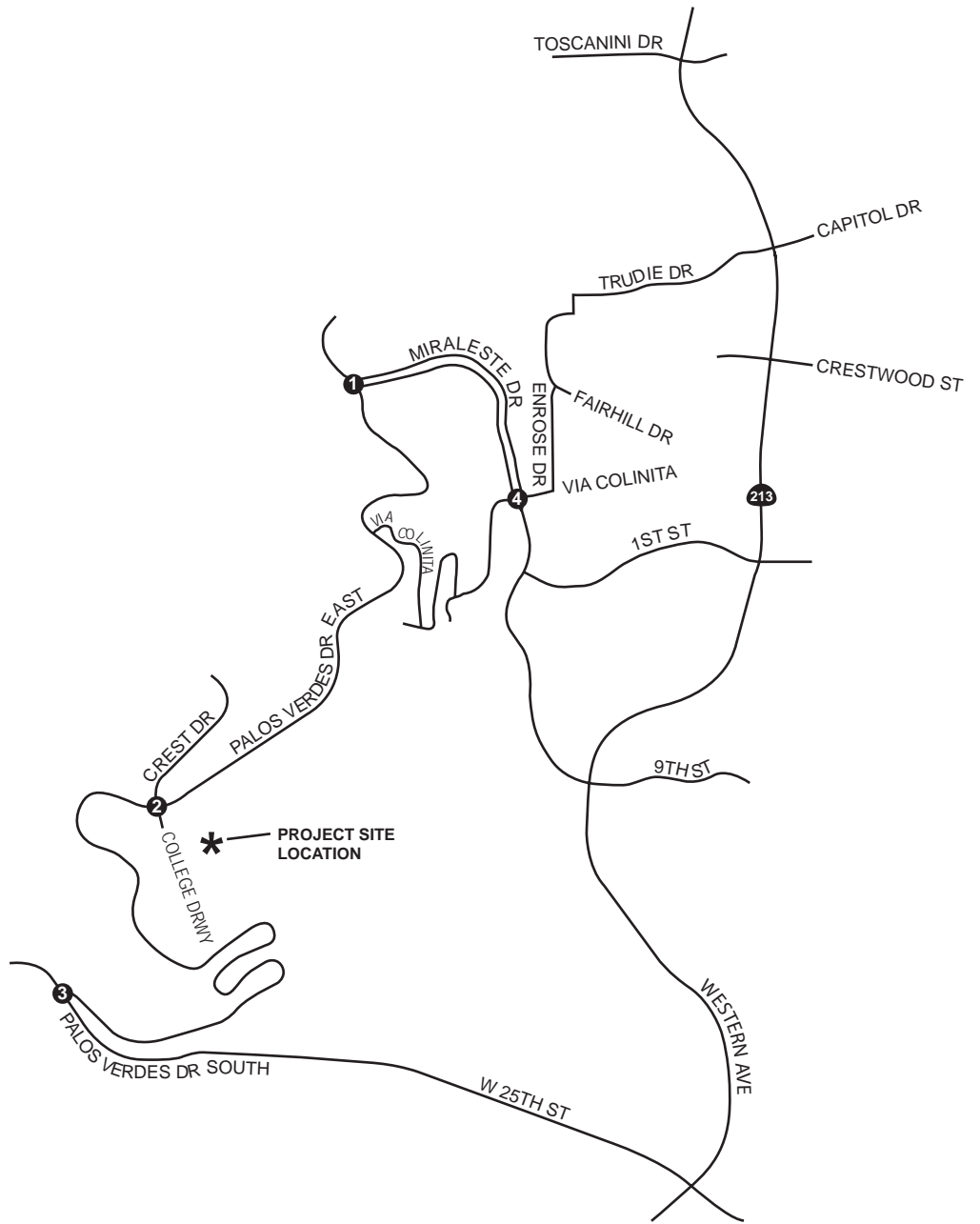
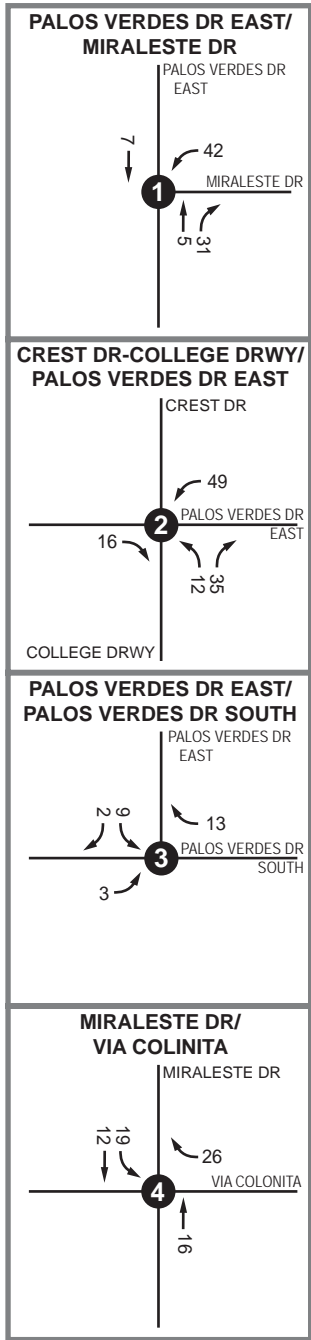


Legend:

XX/XX Weekday Mid-Day 11:00 AM-1:00 PM/Afternoon 2:00-4:00 PM Peak Hour Volumes



# Forecast Proposed Project Conditions Weekday Mid-day 11:00 AM-1:00 PM & Afternoon 2:00-4:00 PM Peak Hour Trip Assignment

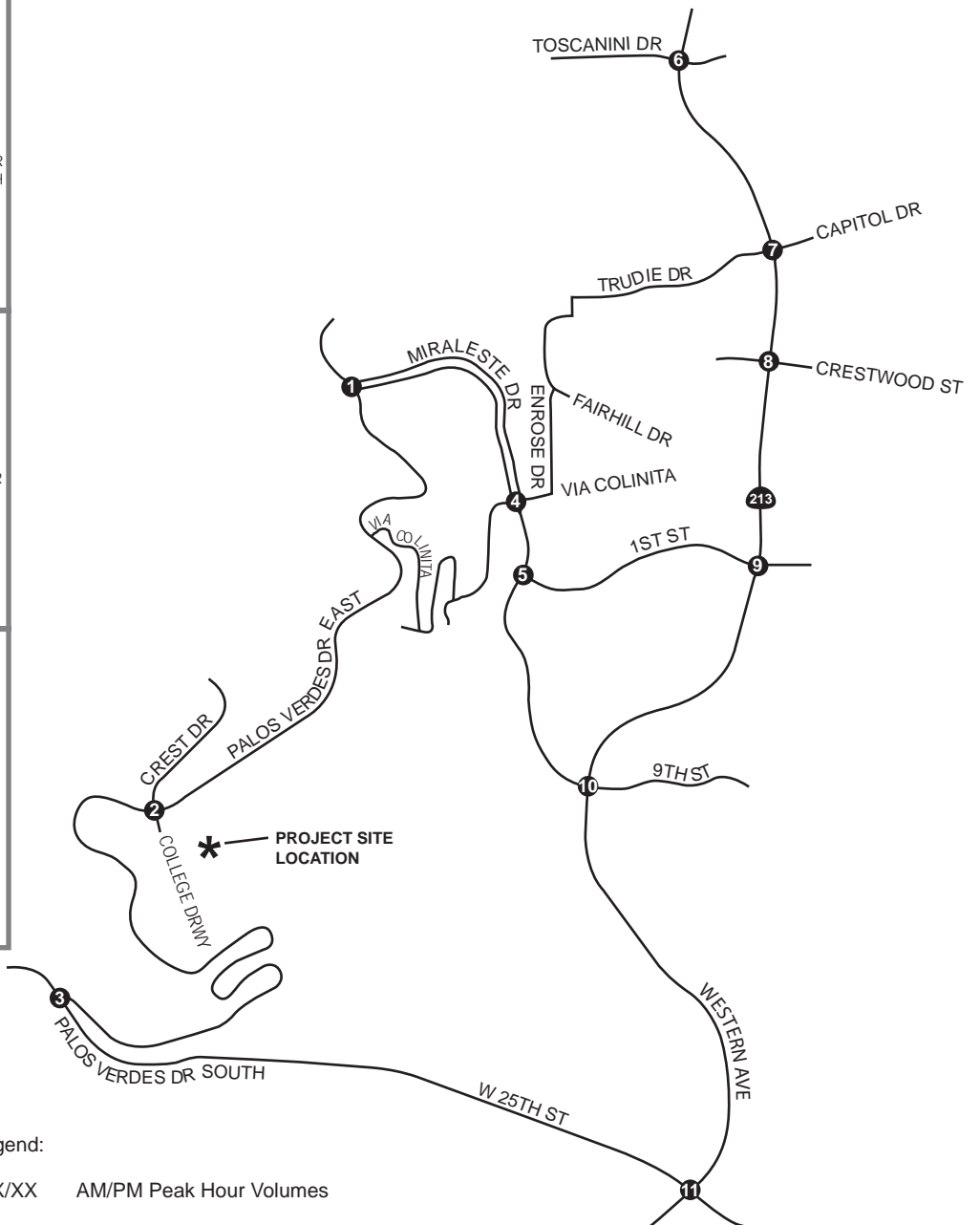
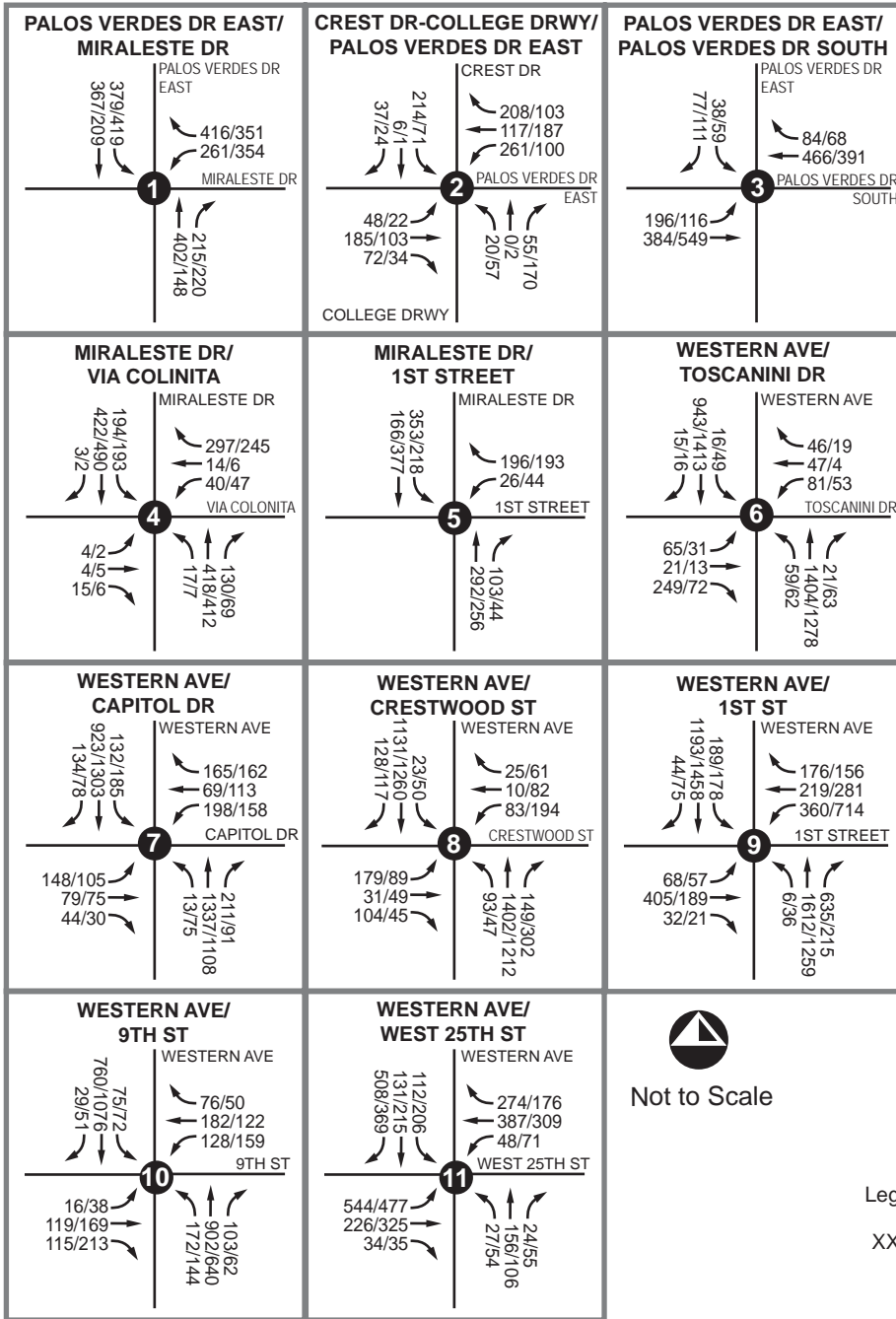


Legend:

XX/XX Mid-Day Weekend 11:00-1:00 PM Peak Hour Volumes



# Forecast Proposed Project Conditions Saturday 11:00 AM-1:00 PM Peak Hour Trip Assignment

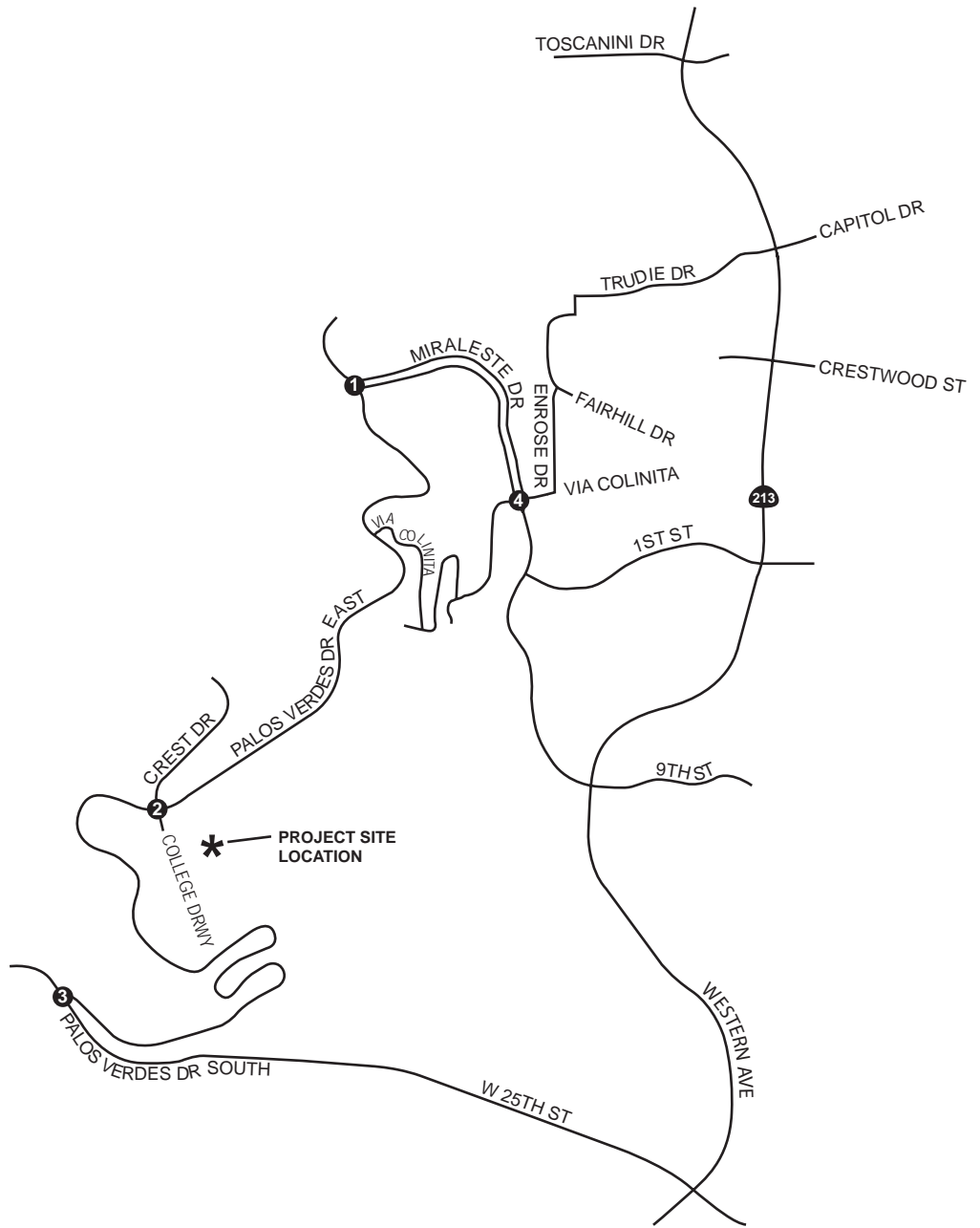
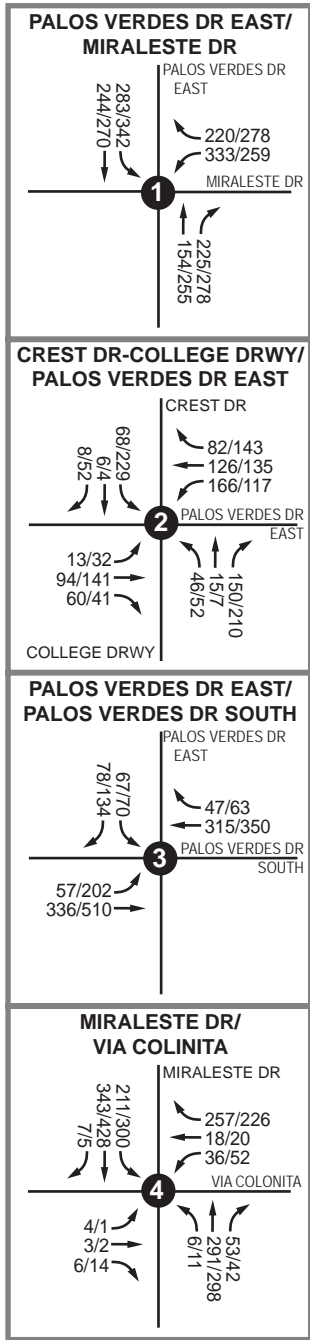


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Legend:  
XX/XX AM/PM Peak Hour Volumes

## Existing Plus Marymount Project Conditions Weekday AM & PM Peak Hour Intersection Volumes





Legend:

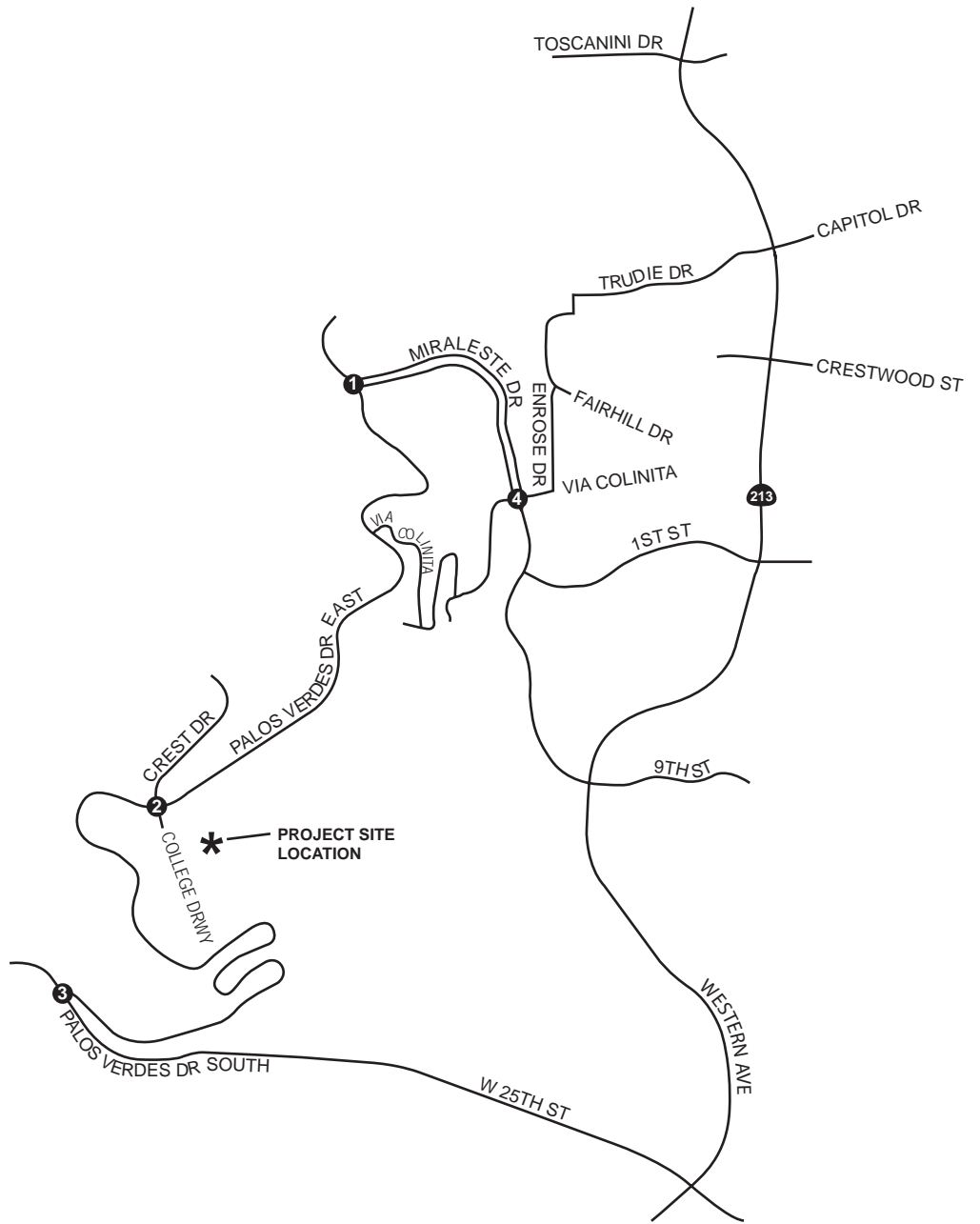
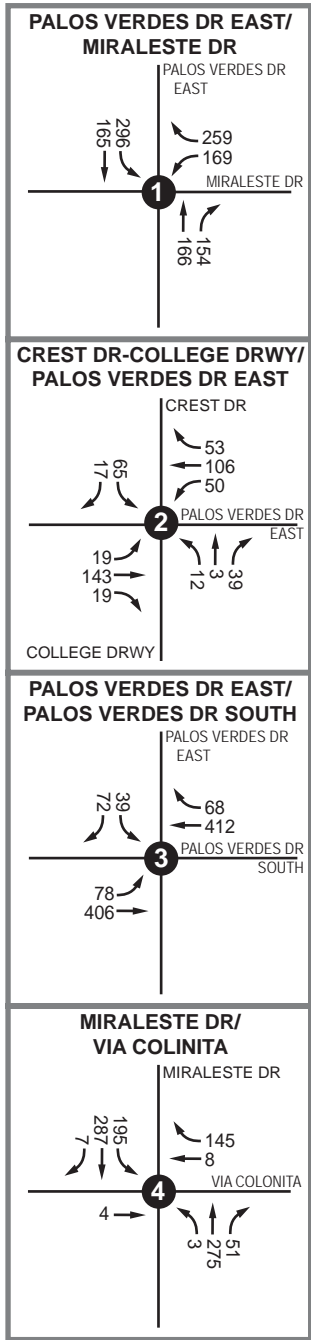
XX/XX Weekday Mid-Day 11:00 AM-1:00 PM/Afternoon 2:00-4:00 PM Peak Hour Volumes



Not to Scale



# Existing Plus Marymount Project Conditions Weekday Mid-day 11:00 AM-1:00 PM & Afternoon 2:00-4:00 PM Peak Hour Intersection Volumes

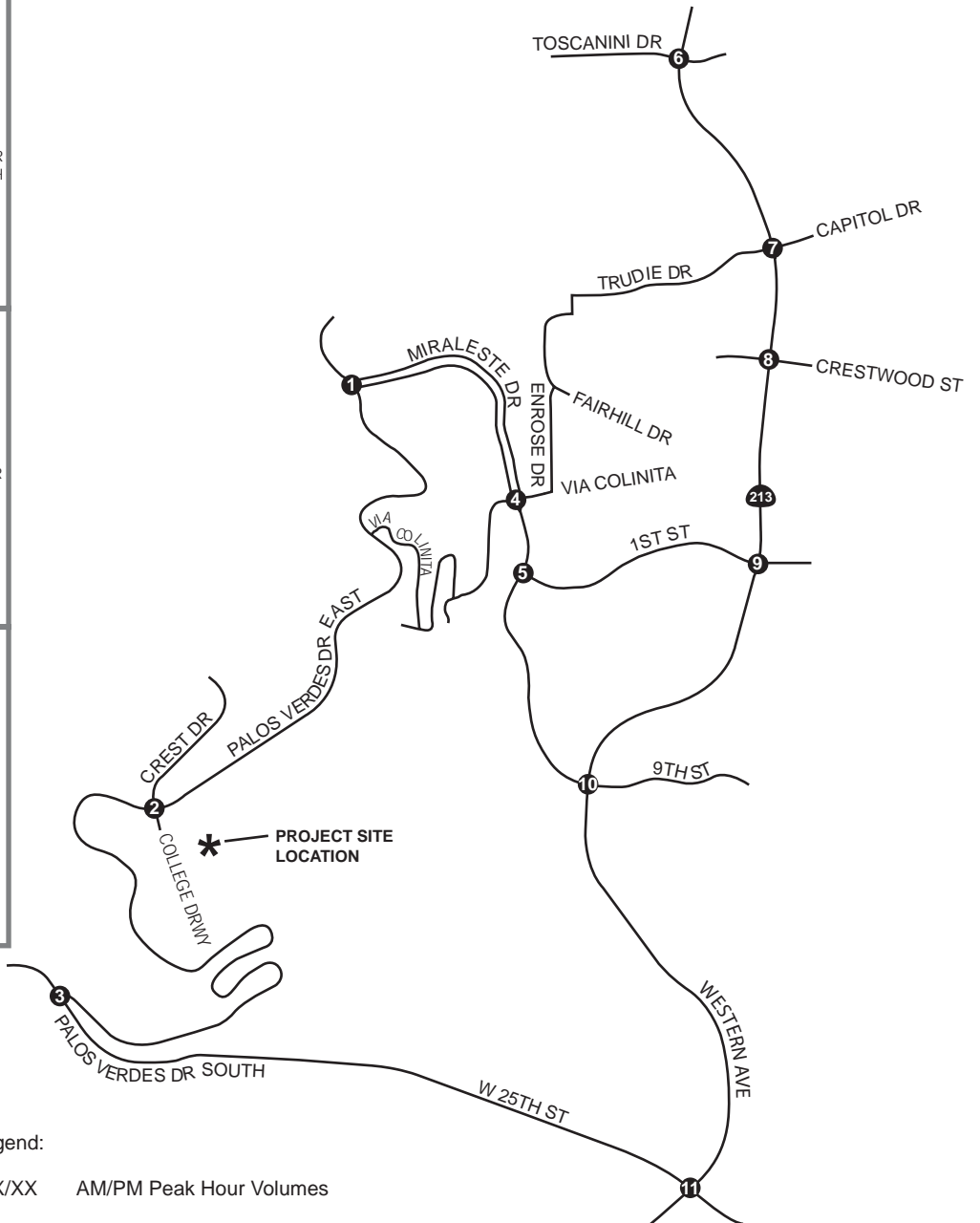
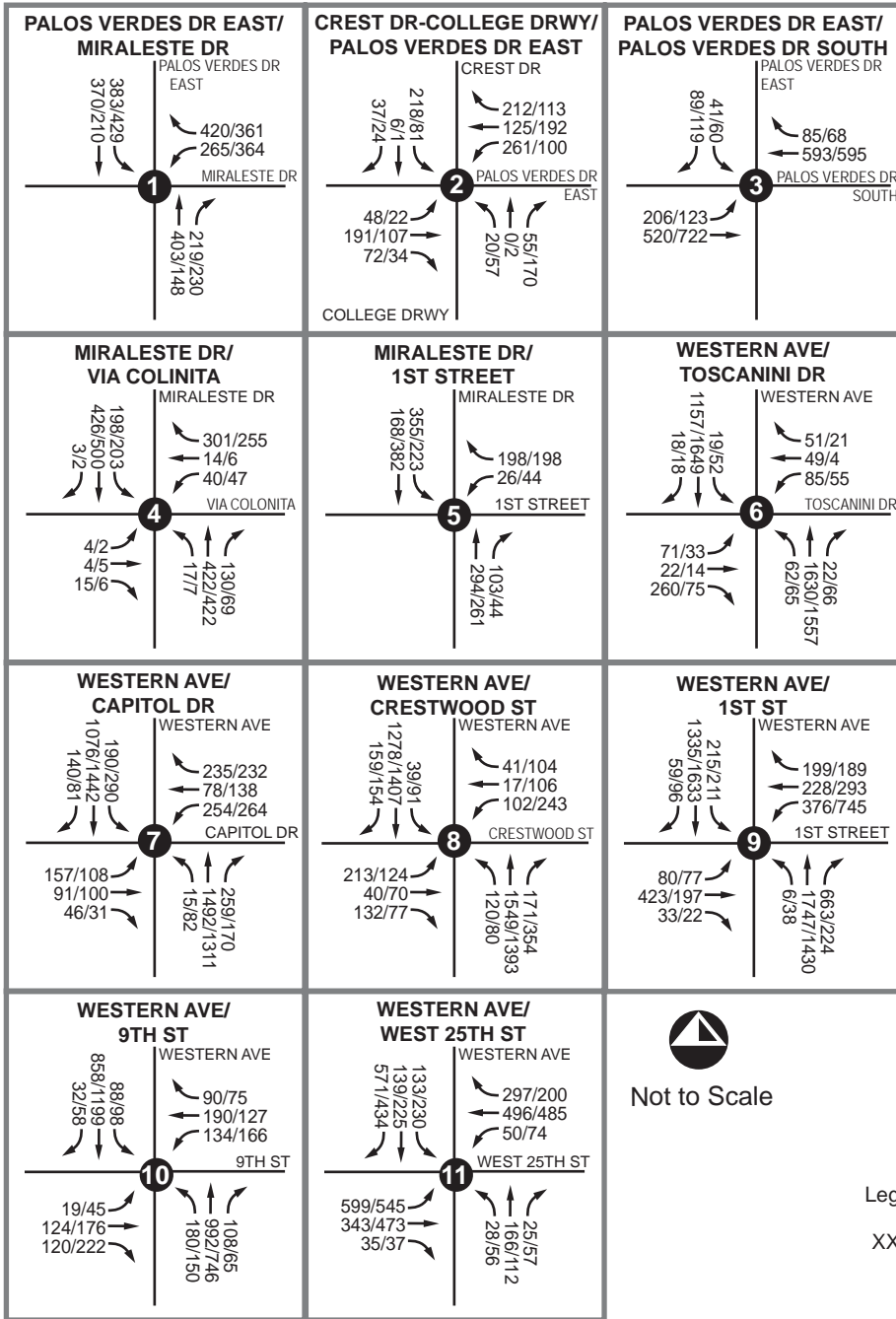


Legend:

XX Saturday Mid-Day Peak Hour Volumes



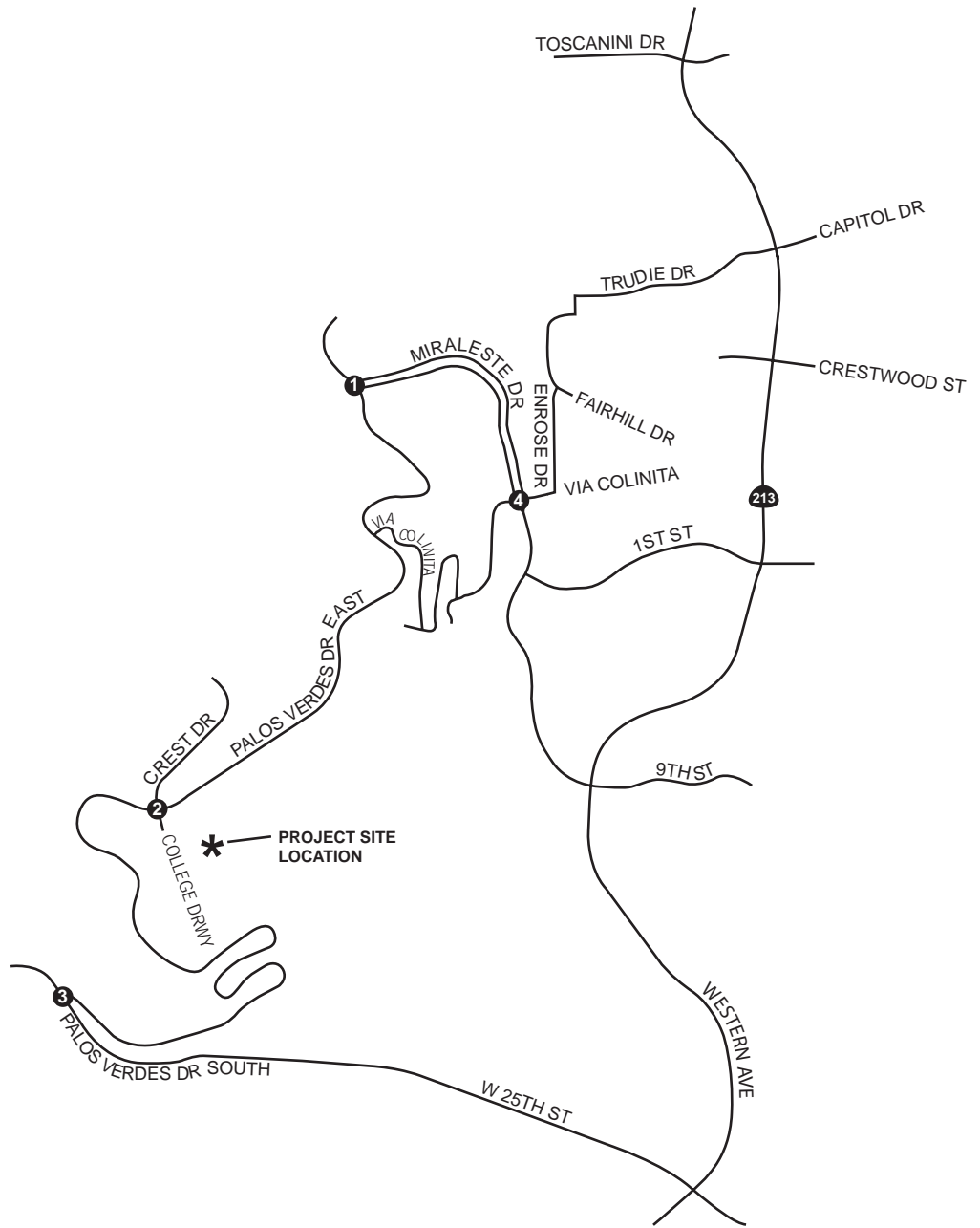
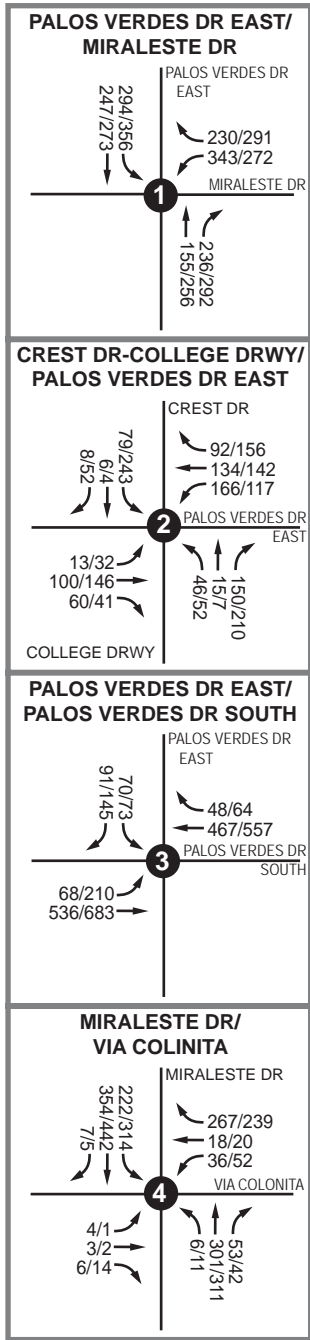
# Existing Plus Marymount Project Conditions Saturday Mid-Day Peak Hour Intersection Volumes



  
 Not to Scale

Legend:  
 XX/XX AM/PM Peak Hour Volumes

## Forecast Year 2012 With Project Conditions Weekday AM & PM Peak Hour Intersection Volumes

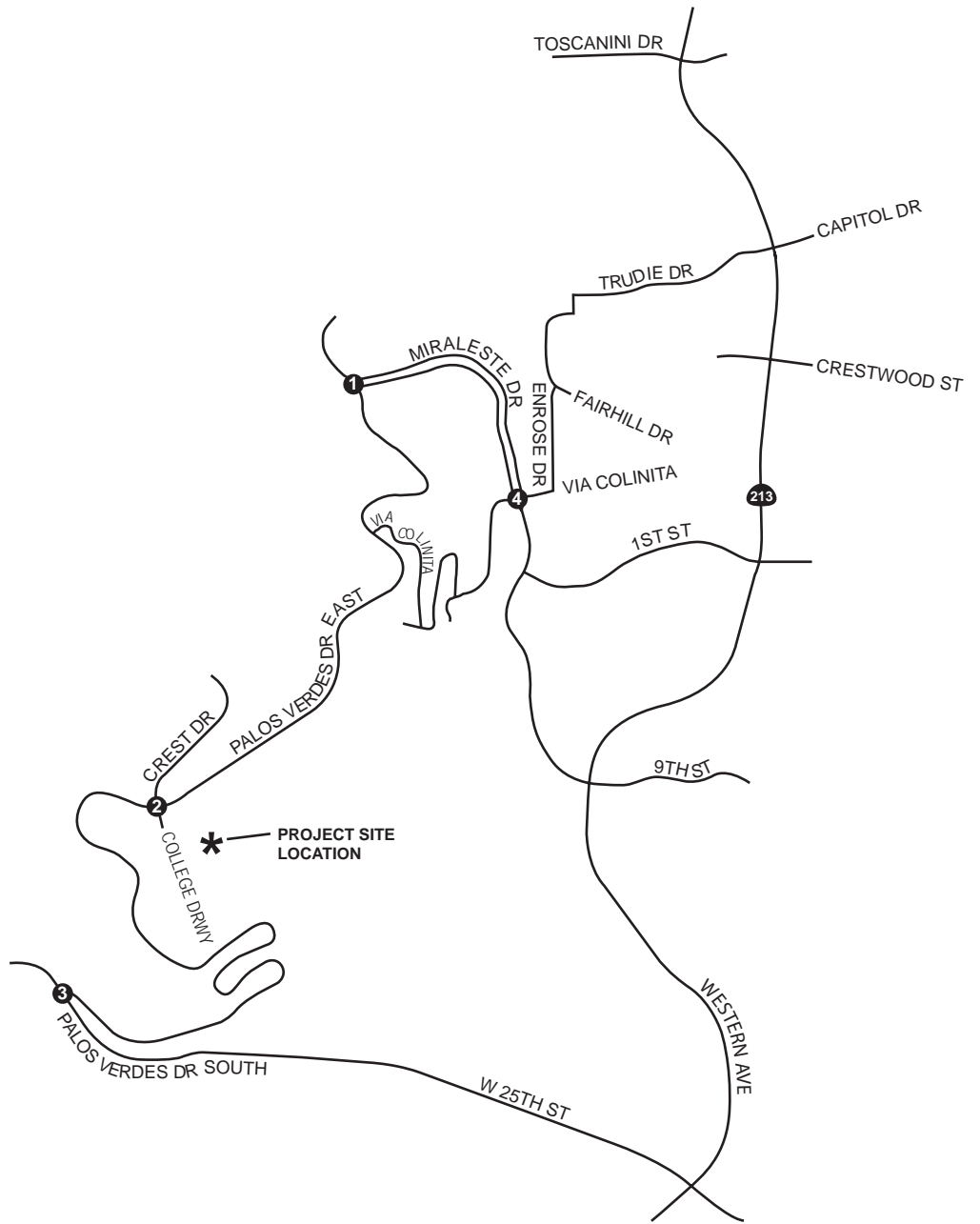
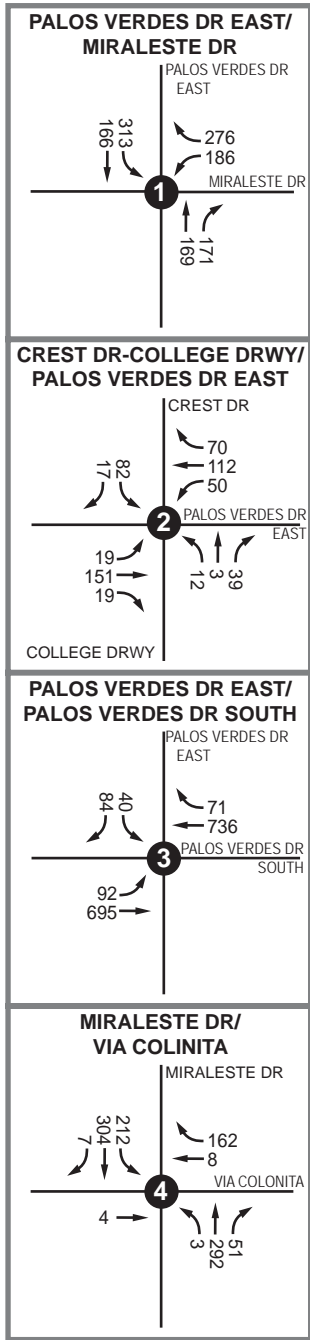


Legend:

XX/XX Weekday Mid-Day 11:00 AM-1:00 PM/Afternoon 2:00-4:00 PM Peak Hour Volumes



## Forecast Year 2012 With Project Conditions Weekday Mid-Day 11:00 AM-1:00 PM & Afternoon 2:00-4:00 PM Peak Hour Intersection Volumes



Legend:

XX Saturday Mid-Day Peak Hour Volumes



Not to Scale



# Forecast Year 2012 With Project Conditions Saturday Mid-Day Peak Hour Intersection Volumes

**APPENDIX B**  
**LOS Analysis Sheets**

## **Existing Conditions**

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS
AM PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #1 PALOS VERDES DRIVE EAST/MIRALESTE DRIVE

Average Delay (sec/veh): 90.7 Worst Case Level Of Service: F[287.9]

Table with 5 columns: Approach, North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module:

Table with 12 columns representing traffic volumes for different movements and approaches.

Critical Gap Module:

Table with 3 columns: Critical Gap, FollowUpTim, and values.

Capacity Module:

Table with 3 columns: Cnflict Vol, Potent Cap., Move Cap., Volume/Cap.

Level Of Service Module:

Table with 3 columns: 2Way95thQ, Control Del, LOS by Move.

Table with 4 columns: Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS
AM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #2 CREST DRIVE - COLLEGE ENTRANCE/PALOS VERDES DRIVE EAST

Cycle (sec): 100 Critical Vol./Cap.(X): 0.436

Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 30 Level Of Service: A

Table with 5 columns: Approach, North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, Lanes.

Volume Module:

Table with 12 columns representing traffic volumes for different movements and approaches.

Saturation Flow Module:

Table with 12 columns representing saturation flow values.

Capacity Analysis Module:

Table with 12 columns representing capacity analysis values.

Crit Moves: \*\*\*\* \*\*

\*\*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS
AM PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #3 PALOS VERDES DRIVE EAST/PALOS VERDES DRIVE SOUTH

Average Delay (sec/veh): 3.1 Worst Case Level Of Service: C[ 19.0]

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled
Rights: Include Include Include Include
Lanes: 0 0 0 0 0 1 0 0 0 1 1 0 1 0 0 0 0 1 0 1

Volume Module:

Table with 12 columns for traffic metrics: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume. Rows include data for North, South, East, and West bound movements.

Critical Gap Module:

Table with 12 columns for critical gap metrics: Critical Gap, FollowUpTim. Rows include data for North, South, East, and West bound movements.

Capacity Module:

Table with 12 columns for capacity metrics: Cnflct Vol, Potent Cap., Move Cap., Volume/Cap. Rows include data for North, South, East, and West bound movements.

Level Of Service Module:

Table with 12 columns for level of service metrics: 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS. Rows include data for North, South, East, and West bound movements.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS
AM PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #5 MIRALESTE DRIVE/1ST STREET

Average Delay (sec/veh): 5.6 Worst Case Level Of Service: B[ 14.7]

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 0 0 1 0 1 1 0 1 0 0 0 0 0 0 0 1 0 0 0 1

Volume Module:

Table with 12 columns for traffic metrics: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume. Rows include data for North, South, East, and West bound movements.

Critical Gap Module:

Table with 12 columns for critical gap metrics: Critical Gap, FollowUpTim. Rows include data for North, South, East, and West bound movements.

Capacity Module:

Table with 12 columns for capacity metrics: Cnflct Vol, Potent Cap., Move Cap., Volume/Cap. Rows include data for North, South, East, and West bound movements.

Level Of Service Module:

Table with 12 columns for level of service metrics: 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS. Rows include data for North, South, East, and West bound movements.

Note: Queue reported is the number of cars per lane.

HCS+: Unsignalized Intersections Release 5.2

TWO-WAY STOP CONTROL SUMMARY

Analyst: DK  
 Agency/Co.: RBF  
 Date Performed: 7/23/2007  
 Analysis Time Period: AM Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: Existing  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: Miraleste Dr  
 Intersection Orientation: NS  
 Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	Northbound			Southbound		
	1 L	2 T	3 R	4 L	5 T	6 R
Volume	17	381	130	173	410	3
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	18	414	141	188	445	3
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L	TR	
Upstream Signal?	No			No		

Minor Street: Approach Movement	Westbound			Eastbound		
	7 L	8 T	9 R	10 L	11 T	12 R
Volume	40	14	237	4	4	15
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	43	15	257	4	4	16
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage	/			No /		
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Delay, Queue Length, and Level of Service

Approach Movement Lane Config	NB	SB	Westbound		Eastbound		
	1	4	7	8	9	10	11 12
	LT	L	LT		R		LTR
v (vph)	18	188	58		257		24
C(m) (vph)	1123	1026	251		643		239
v/c	0.02	0.18	0.23		0.40		0.10
95% queue length	0.05	0.67	0.87		1.92		0.33
Control Delay	8.3	9.3	23.6		14.3		21.7
LOS	A	A	C		B		C
Approach Delay				16.0			21.7
Approach LOS				C			C

HCS+: Unsignalized Intersections Release 5.2

Phone:  
 E-Mail:  
 Fax:

TWO-WAY STOP CONTROL(TWSC) ANALYSIS

Analyst: DK  
 Agency/Co.: RBF  
 Date Performed: 7/23/2007  
 Analysis Time Period: AM Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: Existing  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: Miraleste Dr  
 Intersection Orientation: NS  
 Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street Movements	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	17	381	130	173	410	3
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	5	104	35	47	111	1
Hourly Flow Rate, HFR	18	414	141	188	445	3
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L	TR	
Upstream Signal?	No			No		

Minor Street Movements	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	40	14	237	4	4	15
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	11	4	64	1	1	4
Hourly Flow Rate, HFR	43	15	257	4	4	16
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage	/			No /		
RT Channelized?	No					
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Pedestrian Volumes and Adjustments

Movements	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)	12.0	12.0	12.0	12.0
Walking Speed (ft/sec)	4.0	4.0	4.0	4.0
Percent Blockage	0	0	0	0

Upstream Signal Data							
	Prog. Flow vph	Sat Flow vph	Arrival Type	Green Time sec	Cycle Length sec	Prog. Speed mph	Distance to Signal feet
S2	Left-Turn						
	Through						
S5	Left-Turn						
	Through						

Worksheet 3-Data for Computing Effect of Delay to Major Street Vehicles

	Movement 2	Movement 5
Shared ln volume, major th vehicles:	414	
Shared ln volume, major rt vehicles:	0	
Sat flow rate, major th vehicles:	1700	
Sat flow rate, major rt vehicles:	1700	
Number of major street through lanes:	1	

Worksheet 4-Critical Gap and Follow-up Time Calculation

Critical Gap Calculation									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
t(c,base)	4.1	4.1	7.1	6.5	6.2	7.1	6.5	6.2	
t(c,hv)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
P(hv)	0	0	0	0	0	0	0	0	
t(c,g)			0.20	0.20	0.10	0.20	0.20	0.10	
Grade/100			0.00	0.00	0.00	0.00	0.00	0.00	
t(3,lt)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
t(c,T): 1-stage	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
2-stage	0.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	
t(c) 1-stage	4.1	4.1	7.1	6.5	6.2	7.1	6.5	6.2	
2-stage	4.1	4.1	6.1	5.5	6.2	6.1	5.5	6.2	

Follow-Up Time Calculations									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
t(f,base)	2.20	2.20	3.50	4.00	3.30	3.50	4.00	3.30	
t(f,HV)	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	
P(HV)	0	0	0	0	0	0	0	0	
t(f)	2.2	2.2	3.5	4.0	3.3	3.5	4.0	3.3	

Worksheet 5-Effect of Upstream Signals

Computation 1-Queue Clearance Time at Upstream Signal				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
V prog				
Total Saturation Flow Rate, s (vph)				
Arrival Type				
Effective Green, g (sec)				
Cycle Length, C (sec)				
Rp (from Exhibit 16-11)				
Proportion vehicles arriving on green P				
g(q1)				
g(q2)				
g(q)				

Computation 2-Proportion of TWSC Intersection Time blocked				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
alpha				
beta				
Travel time, t(a) (sec)				
Smoothing Factor, F				
Proportion of conflicting flow, f				
Max platooned flow, V(c,max)				
Min platooned flow, V(c,min)				
Duration of blocked period, t(p)				
Proportion time blocked, p		0.000		0.000

Computation 3-Platoon Event Periods	Result
p(2)	0.000
p(5)	0.000
p(dom)	
p(subo)	
Constrained or unconstrained?	

Proportion unblocked for minor movements, p(x)	(1)	(2)	(3)
	Single-stage Process	Two-Stage Stage I	Process Stage II
p(1)			
p(4)			
p(7)			
p(8)			
p(9)			
p(10)			
p(11)			
p(12)			

Computation 4 and 5 Single-Stage Process									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
V c,x	448	555	1282	1274	414	1478	1413	446	
s									
Px									
V c,u,x									
C r,x									
C plat,x									

	Two-Stage Process							
	7		8		10		11	
	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2
V(c,x)	450	832	450	824	822	656	822	591
s		1500		1500		1500		1500
P(x)								
V(c,u,x)								
C(r,x)								
C(plat,x)								

Worksheet 6-Impedance and Capacity Equations

Step 1: RT from Minor St.	9	12
Conflicting Flows	414	446
Potential Capacity	643	617
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	643	617
Probability of Queue free St.	0.60	0.97
Step 2: LT from Major St.	4	1
Conflicting Flows	555	448
Potential Capacity	1026	1123
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	1026	1123
Probability of Queue free St.	0.82	0.98
Maj L-Shared Prob Q free St.		0.98
Step 3: TH from Minor St.	8	11
Conflicting Flows	1274	1413
Potential Capacity	169	139
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.80	0.80
Movement Capacity	135	111
Probability of Queue free St.	0.94	0.98
Step 4: LT from Minor St.	7	10
Conflicting Flows	1282	1478
Potential Capacity	144	105
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.79	0.76
Maj. L, Min T Adj. Imp Factor.	0.83	0.81
Cap. Adj. factor due to Impeding mvmnt	0.81	0.49
Movement Capacity	117	51

Worksheet 7-Computation of the Effect of Two-stage Gap Acceptance

Step 3: TH from Minor St.	8	11
Part 1 - First Stage		
Conflicting Flows	450	822
Potential Capacity	575	391
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.98	0.82
Movement Capacity	563	319
Probability of Queue free St.	0.97	0.99
Part 2 - Second Stage		
Conflicting Flows	824	591
Potential Capacity	390	498
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.82	0.98
Movement Capacity	319	487
Part 3 - Single Stage		
Conflicting Flows	1274	1413
Potential Capacity	169	139
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.80	0.80
Movement Capacity	135	111
Result for 2 stage process:		
a	0.95	0.95

Y	2.58	1.11
C t	270	230
Probability of Queue free St.	0.94	0.98
Step 4: LT from Minor St.	7	10
Part 1 - First Stage		
Conflicting Flows	450	822
Potential Capacity	592	371
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.98	0.82
Movement Capacity	579	303
Part 2 - Second Stage		
Conflicting Flows	832	656
Potential Capacity	366	458
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.79	0.57
Movement Capacity	288	262
Part 3 - Single Stage		
Conflicting Flows	1282	1478
Potential Capacity	144	105
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.79	0.76
Maj. L, Min T Adj. Imp Factor.	0.83	0.81
Cap. Adj. factor due to Impeding mvmnt	0.81	0.49
Movement Capacity	117	51
Results for Two-stage process:		
a	0.95	0.95
Y	3.02	10.96
C t	245	70

Worksheet 8-Shared Lane Calculations

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (vph)	43	15	257	4	4	16
Movement Capacity (vph)	245	270	643	70	230	617
Shared Lane Capacity (vph)	251				239	

Worksheet 9-Computation of Effect of Flared Minor Street Approaches

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
C sep	245	270	643	70	230	617
Volume	43	15	257	4	4	16
Delay						
Q sep						
Q sep +1						
round (Qsep +1)						
n max						
C sh	251				239	
SUM C sep						
n						
C act						

Worksheet 10-Delay, Queue Length, and Level of Service

Movement	1	4	7	8	9	10	11	12
Lane Config	LT	L	LT		R		LTR	
v (vph)	18	188	58		257		24	
C(m) (vph)	1123	1026	251		643		239	
v/c	0.02	0.18	0.23		0.40		0.10	
95% queue length	0.05	0.67	0.87		1.92		0.33	
Control Delay	8.3	9.3	23.6		14.3		21.7	
LOS	A	A	C		B		C	
Approach Delay				16.0			21.7	
Approach LOS				C			C	

Worksheet 11-Shared Major LT Impedance and Delay

	Movement 2	Movement 5
p(oj)	0.98	0.82
v(i1), Volume for stream 2 or 5	414	
v(i2), Volume for stream 3 or 6	0	
s(i1), Saturation flow rate for stream 2 or 5	1700	
s(i2), Saturation flow rate for stream 3 or 6	1700	
P*(oj)	0.98	
d(M,LT), Delay for stream 1 or 4	8.3	9.3
N, Number of major street through lanes	1	
d(rank,1) Delay for stream 2 or 5	0.2	

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS
AM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #6 WESTERN AVENUE/TOSCANINI DRIVE
\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.812
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 69 Level Of Service: D

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module: Table with 12 columns (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume) and 12 rows.

Saturation Flow Module: Table with 12 columns (Sat/Lane, Adjustment, Lanes, Final Sat) and 4 rows.

Capacity Analysis Module: Table with 12 columns (Vol/Sat, Crit Moves) and 2 rows.

\*\*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS
AM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #7 WESTERN AVENUE/CAPITOL DRIVE
\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.913
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 107 Level Of Service: E

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module: Table with 12 columns (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume) and 12 rows.

Saturation Flow Module: Table with 12 columns (Sat/Lane, Adjustment, Lanes, Final Sat) and 4 rows.

Capacity Analysis Module: Table with 12 columns (Vol/Sat, Crit Moves) and 2 rows.

\*\*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) EXISTING CONDITIONS AM PEAK HOUR

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)
Intersection #8 WESTERN AVENUE/CRESTWOOD STREET
Cycle (sec): 100 Critical Vol./Cap.(X): 0.859
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 83 Level Of Service: D
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Permitted Permitted Permitted Permitted
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Lanes: 1 0 2 0 1 1 0 2 0 1 0 0 1 0 0 1 0
Volume Module:
Base Vol: 93 1394 149 23 1107 128 179 31 104 83 10 25
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 93 1394 149 23 1107 128 179 31 104 83 10 25
Added Vol: 0 0 0 0 2 0 0 0 0 0 0 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 93 1394 149 23 1109 128 179 31 104 83 10 25
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 101 1515 162 25 1205 139 195 34 113 90 11 27
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 101 1515 162 25 1205 139 195 34 113 90 11 27
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 101 1515 162 25 1205 139 195 34 113 90 11 27
Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 1.00 2.00 1.00 1.00 2.00 1.00 0.57 0.10 0.33 1.00 0.29 0.71
Final Sat.: 1600 3200 1600 1600 3200 1600 912 158 530 1600 457 1143
Capacity Analysis Module:
Vol/Sat: 0.06 0.47 0.10 0.02 0.38 0.09 0.12 0.21 0.21 0.06 0.02 0.02
Crit Moves: \*\*\*\* \*\*\*\* \*\*\*\* \*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) EXISTING CONDITIONS AM PEAK HOUR

Level Of Service Computation Report
Circular 212 Planning Method (Future Volume Alternative)
Intersection #9 WESTERN AVENUE/1ST STREET
Cycle (sec): 100 Critical Vol./Cap.(X): 1.414
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Permitted Prot+Permit Permitted Prot+Permit
Rights: Ovl Include Include Ovl
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Lanes: 1 0 2 0 1 1 0 1 1 0 1 0 0 1 0 1
Volume Module:
Base Vol: 6 1612 635 189 1193 20 60 402 32 360 211 176
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 6 1612 635 189 1193 20 60 402 32 360 211 176
Added Vol: 0 0 0 0 0 2 0 0 0 0 0 1 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 6 1612 635 189 1193 22 60 402 32 360 212 176
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 7 1752 690 205 1297 24 65 437 35 391 230 191
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 7 1752 690 205 1297 24 65 437 35 391 230 191
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 7 1752 690 205 1297 24 65 437 35 391 230 191
Saturation Flow Module:
Sat/Lane: 1375 1375 1375 1375 1375 1375 1375 1375 1375 1375 1375 1375
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 1.00 2.00 1.00 1.00 1.96 0.04 1.00 0.93 0.07 1.00 1.00 1.00
Final Sat.: 1375 2750 1375 1375 2700 50 1375 1274 101 1375 1375 1375
Capacity Analysis Module:
Vol/Sat: 0.00 0.64 0.50 0.15 0.48 0.48 0.05 0.34 0.34 0.28 0.17 0.14
Crit Volume: 876 205 472 391
Crit Moves: \*\*\*\* \*\*\*\* \*\*\*\* \*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) EXISTING CONDITIONS AM PEAK HOUR

Level of Service Computation Report
Circular 212 Planning Method (Future Volume Alternative)
Intersection #10 WESTERN AVENUE/9TH STREET
Cycle (sec): 100 Critical Vol./Cap.(X): 0.607
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 51 Level Of Service: B
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Protected Protected Permitted Permitted
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Lanes: 1 0 2 0 1 1 0 2 0 1 1 0 1 0 1
Volume Module:
Base Vol: 172 902 103 75 760 29 16 116 115 128 174 76
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 172 902 103 75 760 29 16 116 115 128 174 76
Added Vol: 0 0 0 0 0 0 0 0 0 0 1 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 172 902 103 75 760 29 16 116 115 128 175 76
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 187 980 112 82 826 32 17 126 125 139 190 83
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 187 980 112 82 826 32 17 126 125 139 190 83
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 187 980 112 82 826 32 17 126 125 139 190 83
Saturation Flow Module:
Sat/Lane: 1425 1425 1425 1425 1425 1425 1425 1425 1425 1425 1425 1425
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 1.00 2.00 1.00 1.00 2.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Final Sat.: 1425 2850 1425 1425 2850 1425 1425 1425 1425 1425 1425 1425
Capacity Analysis Module:
Vol/Sat: 0.13 0.34 0.08 0.06 0.29 0.02 0.01 0.09 0.09 0.10 0.13 0.06
Crit Volume: 187 413 126 139
Crit Moves: \*\*\*\* \*\*\*\* \*\*\*\* \*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) EXISTING CONDITIONS AM PEAK HOUR

Level of Service Computation Report
Circular 212 Planning Method (Future Volume Alternative)
Intersection #11 WESTERN AVENUE/WEST 25TH STREET
Cycle (sec): 100 Critical Vol./Cap.(X): 0.681
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 63 Level Of Service: B
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Permitted Permitted Protected Protected
Rights: Include Ovl Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Lanes: 1 0 2 0 1 1 0 2 0 1 2 0 2 0 1 1 0 1 0 1
Volume Module:
Base Vol: 27 156 24 112 131 508 544 215 34 48 355 274
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 27 156 24 112 131 508 544 215 34 48 355 274
Added Vol: 0 0 0 0 0 0 0 1 0 0 3 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 27 156 24 112 131 508 544 216 34 48 358 274
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 29 170 26 122 142 552 591 235 37 52 389 298
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 29 170 26 122 142 552 591 235 37 52 389 298
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.10 1.00 1.00 1.00 1.00 1.00
FinalVolume: 29 170 26 122 142 552 650 235 37 52 389 298
Saturation Flow Module:
Sat/Lane: 1425 1425 1425 1425 1425 1425 1425 1425 1425 1425 1425 1425
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 1.00 2.00 1.00 1.00 2.00 1.00 2.00 2.00 1.00 1.00 1.00 1.00
Final Sat.: 1425 2850 1425 1425 2850 1425 2850 2850 1425 1425 1425 1425
Capacity Analysis Module:
Vol/Sat: 0.02 0.06 0.02 0.09 0.05 0.39 0.23 0.08 0.03 0.04 0.27 0.21
Crit Volume: 29 552 0 389
Crit Moves: \*\*\*\* \*\*\*\* \*\*\*\* \*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS
PM PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #1 PALOS VERDES DRIVE EAST/MIRALESTE DRIVE

Average Delay (sec/veh): 174.6 Worst Case Level Of Service: F[414.9]

Table with 5 columns: Approach, North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module:

Table with 13 columns for traffic volume metrics: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume.

Critical Gap Module:

Table with 4 columns: Critical Gap, FollowUpTim, Capacity Module.

Capacity Module:

Table with 4 columns: Cnflct Vol, Potent Cap., Move Cap., Volume/Cap.

Level Of Service Module:

Table with 4 columns: 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS
PM PEAK HOUR

Level Of Service Computation Report

ICU l(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #2 CREST DRIVE - COLLEGE ENTRANCE/PALOS VERDES DRIVE EAST

Cycle (sec): 100 Critical Vol./Cap.(X): 0.326

Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 26 Level Of Service: A

Table with 5 columns: Approach, North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, Lanes.

Volume Module:

Table with 13 columns for traffic volume metrics: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume.

Saturation Flow Module:

Table with 13 columns for saturation flow metrics: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with 13 columns for capacity analysis metrics: Vol/Sat, Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS
PM PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #3 PALOS VERDES DRIVE EAST/PALOS VERDES DRIVE SOUTH

Average Delay (sec/veh): 2.9 Worst Case Level Of Service: C[ 17.8]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module:

Table with 12 columns representing traffic volumes for different movements and directions.

Critical Gap Module:

Table with 12 columns for critical gap values and follow-up times.

Capacity Module:

Table with 12 columns for capacity metrics like conflict volume, potent capacity, and volume/capacity.

Level Of Service Module:

Table with 12 columns for level of service metrics including delay, LOS by movement, and approach delay.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS
PM PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #5 MIRALESTE DRIVE/1ST STREET

Average Delay (sec/veh): 4.5 Worst Case Level Of Service: B[ 14.6]

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module:

Table with 12 columns representing traffic volumes for different movements and directions.

Critical Gap Module:

Table with 12 columns for critical gap values and follow-up times.

Capacity Module:

Table with 12 columns for capacity metrics like conflict volume, potent capacity, and volume/capacity.

Level Of Service Module:

Table with 12 columns for level of service metrics including delay, LOS by movement, and approach delay.

Note: Queue reported is the number of cars per lane.

HCS+: Unsignalized Intersections Release 5.2

TWO-WAY STOP CONTROL SUMMARY

Analyst: DK  
 Agency/Co.: RBF  
 Date Performed: 7/23/2007  
 Analysis Time Period: PM PEAK HOUR  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: Existing  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: Miraleste Dr  
 Intersection Orientation: NS

Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	Approach Movement	Northbound			Southbound		
		1 L	2 T	3 R	4 L	5 T	6 R
Volume		7	392	69	156	467	2
Peak-Hour Factor, PHF		0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR		7	426	74	169	507	2
Percent Heavy Vehicles		0	--	--	0	--	--
Median Type/Storage		Raised curb			/ 2		
RT Channelized?		No					
Lanes		0	1	1	1	1	0
Configuration		LT R			L TR		
Upstream Signal?		No			No		

Minor Street:	Approach Movement	Westbound			Eastbound		
		7 L	8 T	9 R	10 L	11 T	12 R
Volume		47	6	211	2	5	6
Peak Hour Factor, PHF		0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR		51	6	229	2	5	6
Percent Heavy Vehicles		0	0	0	0	0	0
Percent Grade (%)		0			0		
Flared Approach: Exists?/Storage					/ No /		
Lanes		0	1	1	0	1	0
Configuration		LT R			LTR		

Delay, Queue Length, and Level of Service

Approach Movement	NB	SB	Westbound			Eastbound		
			1 LT	4 L	7 LT	8 R	9 R	10 L
v (vph)	7	169	57			229		13
C(m) (vph)	1066	1075	260			633		284
v/c	0.01	0.16	0.22			0.36		0.05
95% queue length	0.02	0.56	0.82			1.65		0.14
Control Delay	8.4	9.0	22.7			13.9		18.3
LOS	A	A	C			B		C
Approach Delay						15.6		18.3
Approach LOS						C		C

HCS+: Unsignalized Intersections Release 5.2

Phone:  
 E-Mail:  
 Fax:

TWO-WAY STOP CONTROL(TWSC) ANALYSIS

Analyst: DK  
 Agency/Co.: RBF  
 Date Performed: 7/23/2007  
 Analysis Time Period: PM PEAK HOUR  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: Existing  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: Miraleste Dr  
 Intersection Orientation: NS

Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street Movements	1 L	2 T	3 R	4		6 R
				L	T	
Volume	7	392	69	156	467	2
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	2	107	19	42	127	1
Hourly Flow Rate, HFR	7	426	74	169	507	2
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L TR		
Upstream Signal?	No			No		

Minor Street Movements	7 L	8 T	9 R	10		12 R
				L	T	
Volume	47	6	211	2	5	6
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	13	2	57	1	1	2
Hourly Flow Rate, HFR	51	6	229	2	5	6
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage				/ No /		
RT Channelized?	No					
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Pedestrian Volumes and Adjustments

Movements	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)	12.0	12.0	12.0	12.0
Walking Speed (ft/sec)	4.0	4.0	4.0	4.0
Percent Blockage	0	0	0	0

Upstream Signal Data							
	Prog. Flow vph	Sat Flow vph	Arrival Type	Green Time sec	Cycle Length sec	Prog. Speed mph	Distance to Signal feet
S2	Left-Turn						
	Through						
S5	Left-Turn						
	Through						

Worksheet 3-Data for Computing Effect of Delay to Major Street Vehicles

	Movement 2	Movement 5
Shared ln volume, major th vehicles:	426	
Shared ln volume, major rt vehicles:	0	
Sat flow rate, major th vehicles:	1700	
Sat flow rate, major rt vehicles:	1700	
Number of major street through lanes:	1	

Worksheet 4-Critical Gap and Follow-up Time Calculation

Critical Gap Calculation									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
t(c,base)	4.1	4.1	7.1	6.5	6.2	7.1	6.5	6.2	
t(c,hv)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
P(hv)	0	0	0	0	0	0	0	0	
t(c,g)			0.20	0.20	0.10	0.20	0.20	0.10	
Grade/100			0.00	0.00	0.00	0.00	0.00	0.00	
t(3,lt)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
t(c,T):	1-stage	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
	2-stage	0.00	0.00	1.00	1.00	0.00	1.00	1.00	
t(c)	1-stage	4.1	4.1	7.1	6.5	7.1	6.5	6.2	
	2-stage	4.1	4.1	6.1	5.5	6.2	6.1	5.5	

Follow-Up Time Calculations									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
t(f,base)	2.20	2.20	3.50	4.00	3.30	3.50	4.00	3.30	
t(f,HV)	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	
P(HV)	0	0	0	0	0	0	0	0	
t(f)	2.2	2.2	3.5	4.0	3.3	3.5	4.0	3.3	

Worksheet 5-Effect of Upstream Signals

Computation 1-Queue Clearance Time at Upstream Signal				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
V prog				
Total Saturation Flow Rate, s (vph)				
Arrival Type				
Effective Green, g (sec)				
Cycle Length, C (sec)				
Rp (from Exhibit 16-11)				
Proportion vehicles arriving on green P				
g(q1)				
g(q2)				
g(q)				

Computation 2-Proportion of TWSC Intersection Time blocked				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
alpha				
beta				
Travel time, t(a) (sec)				
Smoothing Factor, F				
Proportion of conflicting flow, f				
Max platooned flow, V(c,max)				
Min platooned flow, V(c,min)				
Duration of blocked period, t(p)				
Proportion time blocked, p		0.000		0.000

Computation 3-Platoon Event Periods	Result
p(2)	0.000
p(5)	0.000
p(dom)	
p(subo)	
Constrained or unconstrained?	

Proportion unblocked for minor movements, p(x)	(1)	(2)	(3)
	Single-stage Process	Two-Stage Stage I	Process Stage II
p(1)			
p(4)			
p(7)			
p(8)			
p(9)			
p(10)			
p(11)			
p(12)			

Computation 4 and 5 Single-Stage Process									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
V c,x	509	500	1292	1287	426	1440	1360	508	
s									
Px									
V c,u,x									
C r,x									
C plat,x									

	7		8		10		11	
	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2
V(c,x)	440	852	440	847	846	594	846	514
s		1500		1500		1500		1500
P(x)								
V(c,u,x)								
C(r,x)								
C(plat,x)								

Worksheet 6-Impedance and Capacity Equations

Step 1: RT from Minor St.	9	12
Conflicting Flows	426	508
Potential Capacity	633	569
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	633	569
Probability of Queue free St.	0.64	0.99
Step 2: LT from Major St.	4	1
Conflicting Flows	500	509
Potential Capacity	1075	1066
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	1075	1066
Probability of Queue free St.	0.84	0.99
Maj L-Shared Prob Q free St.		0.99
Step 3: TH from Minor St.	8	11
Conflicting Flows	1287	1360
Potential Capacity	166	150
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.84	0.84
Movement Capacity	139	125
Probability of Queue free St.	0.98	0.98
Step 4: LT from Minor St.	7	10
Conflicting Flows	1292	1440
Potential Capacity	141	112
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.82	0.82
Maj. L, Min T Adj. Imp Factor.	0.86	0.86
Cap. Adj. factor due to Impeding mvmnt	0.85	0.55
Movement Capacity	120	61

Worksheet 7-Computation of the Effect of Two-stage Gap Acceptance

Step 3: TH from Minor St.	8	11
Part 1 - First Stage		
Conflicting Flows	440	846
Potential Capacity	581	381
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.99	0.84
Movement Capacity	576	321
Probability of Queue free St.	0.99	0.98
Part 2 - Second Stage		
Conflicting Flows	847	514
Potential Capacity	381	539
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.84	0.99
Movement Capacity	321	534
Part 3 - Single Stage		
Conflicting Flows	1287	1360
Potential Capacity	166	150
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.84	0.84
Movement Capacity	139	125
Result for 2 stage process:		
a	0.95	0.95

Y	2.50	0.82
C t	281	255
Probability of Queue free St.	0.98	0.98
Step 4: LT from Minor St.	7	10
Part 1 - First Stage		
Conflicting Flows	440	846
Potential Capacity	600	360
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.99	0.84
Movement Capacity	595	303
Part 2 - Second Stage		
Conflicting Flows	852	594
Potential Capacity	357	495
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.82	0.63
Movement Capacity	293	310
Part 3 - Single Stage		
Conflicting Flows	1292	1440
Potential Capacity	141	112
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.82	0.82
Maj. L, Min T Adj. Imp Factor.	0.86	0.86
Cap. Adj. factor due to Impeding mvmnt	0.85	0.55
Movement Capacity	120	61
Results for Two-stage process:		
a	0.95	0.95
Y	2.86	3.03
C t	258	128

Worksheet 8-Shared Lane Calculations

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (vph)	51	6	229	2	5	6
Movement Capacity (vph)	258	281	633	128	255	569
Shared Lane Capacity (vph)	260				284	

Worksheet 9-Computation of Effect of Flared Minor Street Approaches

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
C sep	258	281	633	128	255	569
Volume	51	6	229	2	5	6
Delay						
Q sep						
Q sep +1						
round (Qsep +1)						
n max						
C sh	260				284	
SUM C sep						
n						
C act						

Worksheet 10-Delay, Queue Length, and Level of Service

Movement	1	4	7	8	9	10	11	12
Lane Config	LT	L	LT		R		LTR	
v (vph)	7	169	57		229		13	
C(m) (vph)	1066	1075	260		633		284	
v/c	0.01	0.16	0.22		0.36		0.05	
95% queue length	0.02	0.56	0.82		1.65		0.14	
Control Delay	8.4	9.0	22.7		13.9		18.3	
LOS	A	A	C		B		C	
Approach Delay				15.6			18.3	
Approach LOS				C			C	

Worksheet 11-Shared Major LT Impedance and Delay

	Movement 2	Movement 5
p(oj)	0.99	0.84
v(i1), Volume for stream 2 or 5	426	
v(i2), Volume for stream 3 or 6	0	
s(i1), Saturation flow rate for stream 2 or 5	1700	
s(i2), Saturation flow rate for stream 3 or 6	1700	
P*(oj)	0.99	
d(M,LT), Delay for stream 1 or 4	8.4	9.0
N, Number of major street through lanes	1	
d(rank,1) Delay for stream 2 or 5	0.1	

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS
PM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #6 WESTERN AVENUE/TOSCANINI DRIVE
\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.701
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 50 Level Of Service: C

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module:

Table with 10 columns and 13 rows of traffic volume data including Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume.

Saturation Flow Module:

Table with 10 columns and 4 rows of saturation flow data including Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with 10 columns and 3 rows of capacity analysis data including Vol/Sat, Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS
PM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #7 WESTERN AVENUE/CAPITOL DRIVE
\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.801
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 67 Level Of Service: D

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module:

Table with 10 columns and 13 rows of traffic volume data including Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume.

Saturation Flow Module:

Table with 10 columns and 4 rows of saturation flow data including Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with 10 columns and 3 rows of capacity analysis data including Vol/Sat, Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS
PM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #8 WESTERN AVENUE/CRESTWOOD STREET
\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.812
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 69 Level Of Service: D

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume.

Saturation Flow Module: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module: Vol/Sat, Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS
PM PEAK HOUR

Level Of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #9 WESTERN AVENUE/1ST STREET
\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 1.317
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume.

Saturation Flow Module: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module: Vol/Sat, Crit Volume, Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) EXISTING CONDITIONS PM PEAK HOUR

Level of Service Computation Report Circular 212 Planning Method (Future Volume Alternative) Intersection #10 WESTERN AVENUE/9TH STREET. Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Cycle, Loss Time, Optimal Cycle, Control, Rights, Min. Green, Lanes, Volume Module, Saturation Flow Module, and Capacity Analysis Module.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) EXISTING CONDITIONS PM PEAK HOUR

Level of Service Computation Report Circular 212 Planning Method (Future Volume Alternative) Intersection #11 WESTERN AVENUE/WEST 25TH STREET. Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Cycle, Loss Time, Optimal Cycle, Control, Rights, Min. Green, Lanes, Volume Module, Saturation Flow Module, and Capacity Analysis Module.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS
WEEKDAY (11:00-1:00) PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #1 PALOS VERDES DRIVE EAST/MIRALESTE DRIVE
\*\*\*\*\*

Average Delay (sec/veh): 64.3 Worst Case Level Of Service: F[169.3]

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 0 0 0 1 0 1 0 1 0 0 0 0 0 0 0 1

Volume Module:

Base Vol: 0 144 161 283 232 0 0 0 0 254 0 220
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 0 144 161 283 232 0 0 0 0 254 0 220
Added Vol: 0 1 3 0 1 0 0 0 0 7 0 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 0 145 164 283 233 0 0 0 0 261 0 220
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 0 158 178 308 253 0 0 0 0 284 0 239
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 0 158 178 308 253 0 0 0 0 284 0 239

Critical Gap Module:

Critical Gap:xxxxx xxxx xxxxx 4.1 xxxx xxxxx xxxxx xxxx xxxxxx 6.4 xxxx 6.2
FollowUpTim:xxxxx xxxx xxxxxx 2.2 xxxx xxxxxx xxxxxx xxxx xxxxxx 3.5 xxxx 3.3

Capacity Module:

Cnflct Vol: xxxx xxxx xxxxxx 336 xxxx xxxxxx xxxxx xxxx xxxxxx 1115 xxxxx 247
Potent Cap.: xxxx xxxx xxxxxx 1235 xxxx xxxxxx xxxxx xxxx xxxxxx 232 xxxxx 797
Move Cap.: xxxx xxxx xxxxxx 1235 xxxx xxxxxx xxxxx xxxx xxxxxx 187 xxxxx 797
Volume/Cap: xxxx xxxx xxxxxx 0.25 xxxx xxxxxx xxxxx xxxx xxxxxx 1.51 xxxxx 0.30

Level Of Service Module:

2Way95thQ: xxxxx xxxxx xxxxxx 1.0 xxxxx xxxxxx xxxxx xxxx xxxxxx 18.0 xxxxx 1.3
Control Del:xxxxxx xxxxx xxxxxx 8.9 xxxxx xxxxxx xxxxxx xxxx xxxxxx 302.3 xxxxx 11.4

LOS by Move: \* \* \* A \* \* \* \* \* F \* \* B
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx
SharedQueue:xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx
Shrd ConDel:xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx

Shared LOS: \* \* \* \* \* \* \* \* \* \* \* \* \* \* \* \*
ApproachDel: xxxxxxx xxxxxxx xxxxxxx xxxxxxx 169.3
ApproachLOS: \* \* \* \* \* F

\*\*\*\*\*

Note: Queue reported is the number of cars per lane.

\*\*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS
WEEKDAY (11:00-1:00) PEAK HOUR

Level Of Service Computation Report

ICU l(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #2 CREST DRIVE - COLLEGE ENTRANCE/PALOS VERDES DRIVE EAST
\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.314

Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxxx

Optimal Cycle: 25 Level Of Service: A

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Permitted Permitted Permitted Permitted
Rights: Include Include Include Include
Lanes: 0 0 1 0 0 0 1 0 0 1 1 0 2 0 1 1 0 2 0 1

Volume Module:

Base Vol: 21 15 76 68 6 8 13 94 30 75 126 82
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 21 15 76 68 6 8 13 94 30 75 126 82
Added Vol: 1 0 4 0 0 0 0 0 3 8 0 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 22 15 80 68 6 8 13 94 33 83 126 82
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 24 16 87 74 7 9 14 102 36 90 137 89
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 24 16 87 74 7 9 14 102 36 90 137 89
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 24 16 87 74 7 9 14 102 36 90 137 89

Saturation Flow Module:

Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 0.19 0.13 0.68 0.92 0.08 1.00 1.00 2.00 1.00 1.00 2.00 1.00
Final Sat.: 301 205 1094 1470 130 1600 1600 3200 1600 1600 3200 1600

Capacity Analysis Module:

Vol/Sat: 0.01 0.08 0.08 0.05 0.05 0.01 0.01 0.03 0.02 0.06 0.04 0.06
Crit Moves: \*\*\*\* \*\*\*\* \*\*\*\* \*\*\*\*

\*\*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS
WEEKDAY (11:00-1:00) PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

\*\*\*\*\*

Intersection #3 PALOS VERDES DRIVE EAST/PALOS VERDES DRIVE SOUTH

\*\*\*\*\*

Average Delay (sec/veh): 2.4 Worst Case Level Of Service: B[ 13.5]

\*\*\*\*\*

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled

Rights: Include Include Include Include

Lanes: 0 0 0 0 0 1 0 0 0 1 1 0 1 0 0 0 0 1 0 1

-----|-----|-----|-----|

Volume Module:

Table with 16 columns and 12 rows of traffic volume data including Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and FinalVolume.

-----|-----|-----|-----|

Critical Gap Module:

Critical Gp:xxxxx xxxxx xxxxx 6.4 xxxxx 6.2 4.1 xxxxx xxxxxx xxxxx xxxxx xxxxxx

FollowUpTim:xxxxx xxxxx xxxxx 3.5 xxxxx 3.3 2.2 xxxxx xxxxxx xxxxx xxxxx xxxxxx

-----|-----|-----|-----|

Capacity Module:

Cnflct Vol: xxxxx xxxxx xxxxxx 821 xxxxx 342 370 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Potent Cap.: xxxxx xxxxx xxxxxx 347 xxxxx 705 1200 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Move Cap.: xxxxx xxxxx xxxxxx 335 xxxxx 705 1200 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Volume/Cap: xxxxx xxxxx xxxxx 0.16 xxxxx 0.11 0.05 xxxxx xxxxx xxxxx xxxxx xxxxxx

-----|-----|-----|-----|

Level Of Service Module:

2Way95thQ: xxxxx xxxxx xxxxxx 0.5 xxxxx 0.4 0.1 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Control Del:xxxxxx xxxxx xxxxxx 17.7 xxxxx 10.8 8.1 xxxxx xxxxxx xxxxxx xxxxx xxxxxx

LOS by Move: \* \* \* C \* B A \* \* \* \* \* \*

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx

SharedQueue:xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx

Shrd ConDel:xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx

Shared LOS: \* \* \* \* \* \* \* \* \* \* \* \* \* \* \* \*

ApproachDel: xxxxxxx 13.5 xxxxxxx xxxxxxx

ApproachLOS: \* B \* \*

\*\*\*\*\*

Note: Queue reported is the number of cars per lane.

\*\*\*\*\*

HCS+: Unsignalized Intersections Release 5.2

TWO-WAY STOP CONTROL SUMMARY

Analyst: DK  
 Agency/Co.: RBF  
 Date Performed: 7/23/2007  
 Analysis Time Period: Weekday Mid-day Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: Existing  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: Miraleste Dr  
 Intersection Orientation: NS Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	Northbound			Southbound		
	1 L	2 T	3 R	4 L	5 T	6 R
Volume	6	264	53	173	319	7
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	6	286	57	188	346	7
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L	TR	
Upstream Signal?	No			No		

Minor Street: Approach Movement	Westbound			Eastbound		
	7 L	8 T	9 R	10 L	11 T	12 R
Volume	36	18	213	4	3	6
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	39	19	231	4	3	6
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage	/			No /		
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Delay, Queue Length, and Level of Service

Approach Movement	NB	SB	Westbound			Eastbound		
	1	4	7	8	9	10	11	12
Lane Config	LT	L	LT		R		LTR	
v (vph)	6	188	58		231		13	
C(m) (vph)	1217	1227	319		758		327	
v/c	0.00	0.15	0.18		0.30		0.04	
95% queue length	0.01	0.54	0.65		1.29		0.12	
Control Delay	8.0	8.5	18.8		11.8		16.5	
LOS	A	A	C		B		C	
Approach Delay				13.2			16.5	
Approach LOS				B			C	

HCS+: Unsignalized Intersections Release 5.2

Phone:  
 E-Mail:

Fax:

TWO-WAY STOP CONTROL(TWSC) ANALYSIS

Analyst: DK  
 Agency/Co.: RBF  
 Date Performed: 7/23/2007  
 Analysis Time Period: Weekday Mid-day Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: Existing  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: Miraleste Dr  
 Intersection Orientation: NS Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street Movements	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	6	264	53	173	319	7
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	2	72	14	47	87	2
Hourly Flow Rate, HFR	6	286	57	188	346	7
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L	TR	
Upstream Signal?	No			No		

Minor Street Movements	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	36	18	213	4	3	6
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	10	5	58	1	1	2
Hourly Flow Rate, HFR	39	19	231	4	3	6
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage	/			No /		
RT Channelized?	No					
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Pedestrian Volumes and Adjustments

Movements	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)	12.0	12.0	12.0	12.0
Walking Speed (ft/sec)	4.0	4.0	4.0	4.0
Percent Blockage	0	0	0	0

Upstream Signal Data							
	Prog. Flow vph	Sat Flow vph	Arrival Type	Green Time sec	Cycle Length sec	Prog. Speed mph	Distance to Signal feet
S2	Left-Turn						
	Through						
S5	Left-Turn						
	Through						

Worksheet 3-Data for Computing Effect of Delay to Major Street Vehicles

	Movement 2	Movement 5
Shared ln volume, major th vehicles:	286	
Shared ln volume, major rt vehicles:	0	
Sat flow rate, major th vehicles:	1700	
Sat flow rate, major rt vehicles:	1700	
Number of major street through lanes:	1	

Worksheet 4-Critical Gap and Follow-up Time Calculation

Critical Gap Calculation									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
t(c,base)	4.1	4.1	7.1	6.5	6.2	7.1	6.5	6.2	
t(c,hv)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
P(hv)	0	0	0	0	0	0	0	0	
t(c,g)			0.20	0.20	0.10	0.20	0.20	0.10	
Grade/100			0.00	0.00	0.00	0.00	0.00	0.00	
t(3,lt)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
t(c,T):	1-stage	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
	2-stage	0.00	0.00	1.00	1.00	0.00	1.00	1.00	
t(c)	1-stage	4.1	4.1	7.1	6.5	7.1	6.5	6.2	
	2-stage	4.1	4.1	6.1	5.5	6.2	6.1	5.5	

Follow-Up Time Calculations									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
t(f,base)	2.20	2.20	3.50	4.00	3.30	3.50	4.00	3.30	
t(f,HV)	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	
P(HV)	0	0	0	0	0	0	0	0	
t(f)	2.2	2.2	3.5	4.0	3.3	3.5	4.0	3.3	

Worksheet 5-Effect of Upstream Signals

Computation 1-Queue Clearance Time at Upstream Signal				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
V prog				
Total Saturation Flow Rate, s (vph)				
Arrival Type				
Effective Green, g (sec)				
Cycle Length, C (sec)				
Rp (from Exhibit 16-11)				
Proportion vehicles arriving on green P				
g(q1)				
g(q2)				
g(q)				

Computation 2-Proportion of TWSC Intersection Time blocked				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
alpha				
beta				
Travel time, t(a) (sec)				
Smoothing Factor, F				
Proportion of conflicting flow, f				
Max platooned flow, V(c,max)				
Min platooned flow, V(c,min)				
Duration of blocked period, t(p)				
Proportion time blocked, p		0.000		0.000

Computation 3-Platoon Event Periods	Result
p(2)	0.000
p(5)	0.000
p(dom)	
p(subo)	
Constrained or unconstrained?	

Proportion unblocked for minor movements, p(x)	(1)	(2)	(3)
	Single-stage Process	Two-Stage Stage I	Process Stage II
p(1)			
p(4)			
p(7)			
p(8)			
p(9)			
p(10)			
p(11)			
p(12)			

Computation 4 and 5 Single-Stage Process									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
V c,x	353	343	1028	1027	286	1178	1081	350	
s									
Px									
V c,u,x									
C r,x									
C plat,x									

	7		8		10		11	
	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2
V(c,x)	298	730	298	729	726	452	726	355
s		1500		1500		1500		1500
P(x)								
V(c,u,x)								
C(r,x)								
C(plat,x)								

Worksheet 6-Impedance and Capacity Equations

Step 1: RT from Minor St.	9	12
Conflicting Flows	286	350
Potential Capacity	758	698
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	758	698
Probability of Queue free St.	0.70	0.99
Step 2: LT from Major St.	4	1
Conflicting Flows	343	353
Potential Capacity	1227	1217
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	1227	1217
Probability of Queue free St.	0.85	1.00
Maj L-Shared Prob Q free St.		0.99
Step 3: TH from Minor St.	8	11
Conflicting Flows	1027	1081
Potential Capacity	236	220
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.84	0.84
Movement Capacity	199	185
Probability of Queue free St.	0.94	0.99
Step 4: LT from Minor St.	7	10
Conflicting Flows	1028	1178
Potential Capacity	214	169
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.83	0.79
Maj. L, Min T Adj. Imp Factor.	0.87	0.84
Cap. Adj. factor due to Impeding mvmnt	0.86	0.58
Movement Capacity	185	99

Worksheet 7-Computation of the Effect of Two-stage Gap Acceptance

Step 3: TH from Minor St.	8	11
Part 1 - First Stage		
Conflicting Flows	298	726
Potential Capacity	671	433
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.99	0.85
Movement Capacity	667	367
Probability of Queue free St.	0.97	0.99
Part 2 - Second Stage		
Conflicting Flows	729	355
Potential Capacity	431	633
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.85	0.99
Movement Capacity	365	629
Part 3 - Single Stage		
Conflicting Flows	1027	1081
Potential Capacity	236	220
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.84	0.84
Movement Capacity	199	185
Result for 2 stage process:		
a	0.95	0.95

Y	2.92	0.71
C t	329	309
Probability of Queue free St.	0.94	0.99
Step 4: LT from Minor St.	7	10
Part 1 - First Stage		
Conflicting Flows	298	726
Potential Capacity	715	419
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.99	0.85
Movement Capacity	711	355
Part 2 - Second Stage		
Conflicting Flows	730	452
Potential Capacity	417	591
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.83	0.67
Movement Capacity	347	397
Part 3 - Single Stage		
Conflicting Flows	1028	1178
Potential Capacity	214	169
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.83	0.79
Maj. L, Min T Adj. Imp Factor.	0.87	0.84
Cap. Adj. factor due to Impeding mvmnt	0.86	0.58
Movement Capacity	185	99
Results for Two-stage process:		
a	0.95	0.95
Y	3.37	2.33
C t	314	186

Worksheet 8-Shared Lane Calculations

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (vph)	39	19	231	4	3	6
Movement Capacity (vph)	314	329	758	186	309	698
Shared Lane Capacity (vph)	319				327	

Worksheet 9-Computation of Effect of Flared Minor Street Approaches

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
C sep	314	329	758	186	309	698
Volume	39	19	231	4	3	6
Delay						
Q sep						
Q sep +1						
round (Qsep +1)						
n max						
C sh	319				327	
SUM C sep						
n						
C act						

Worksheet 10-Delay, Queue Length, and Level of Service

Movement	1	4	7	8	9	10	11	12
Lane Config	LT	L	LT		R		LTR	
v (vph)	6	188	58		231		13	
C(m) (vph)	1217	1227	319		758		327	
v/c	0.00	0.15	0.18		0.30		0.04	
95% queue length	0.01	0.54	0.65		1.29		0.12	
Control Delay	8.0	8.5	18.8		11.8		16.5	
LOS	A	A	C		B		C	
Approach Delay				13.2			16.5	
Approach LOS				B			C	

Worksheet 11-Shared Major LT Impedance and Delay

	Movement 2	Movement 5
p(oj)	1.00	0.85
v(i1), Volume for stream 2 or 5	286	
v(i2), Volume for stream 3 or 6	0	
s(i1), Saturation flow rate for stream 2 or 5	1700	
s(i2), Saturation flow rate for stream 3 or 6	1700	
P*(oj)	0.99	
d(M,LT), Delay for stream 1 or 4	8.0	8.5
N, Number of major street through lanes	1	
d(rank,1) Delay for stream 2 or 5	0.0	

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) EXISTING CONDITIONS WEEKDAY (2:00-4:00) PEAK HOUR

Level Of Service Computation Report
2000 HCM Unsignalized Method (Future Volume Alternative)
Intersection #1 PALOS VERDES DRIVE EAST/MIRALESTE DRIVE
Average Delay (sec/veh): 82.2 Worst Case Level Of Service: F[250.5]
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 0 0 0 1 0 1 0 1 0 0 0 0 0 0 0 1
Volume Module:
Base Vol: 0 242 197 342 262 0 0 0 0 0 208 0 278
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 0 242 197 342 262 0 0 0 0 0 208 0 278
Added Vol: 0 1 5 0 1 0 0 0 0 0 6 0 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 0 243 202 342 263 0 0 0 0 0 214 0 278
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 0 264 220 372 286 0 0 0 0 0 233 0 302
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 0 264 220 372 286 0 0 0 0 0 233 0 302
Critical Gap Module:
Critical Gp:xxxxx xxxx xxxxx 4.1 xxxx xxxxx xxxxx xxxx xxxxxx 6.4 xxxx 6.2
FollowUpTim:xxxxx xxxx xxxxxx 2.2 xxxx xxxxxx xxxxx xxxx xxxxxx 3.5 xxxx 3.3
Capacity Module:
Cnflict Vol: xxxx xxxx xxxxxx 484 xxxx xxxxxx xxxxx xxxx xxxxxx 1403 xxxxx 374
Potent Cap.: xxxx xxxx xxxxxx 1090 xxxx xxxxxx xxxxx xxxx xxxxxx 156 xxxxx 677
Move Cap.: xxxx xxxx xxxxxx 1090 xxxx xxxxxx xxxxx xxxx xxxxxx 114 xxxxx 677
Volume/Cap: xxxx xxxx xxxxx 0.34 xxxx xxxxx xxxxx xxxx xxxxxx 2.03 xxxxx 0.45
Level Of Service Module:
2Way95thQ: xxxx xxxx xxxxxx 1.5 xxxx xxxxxx xxxxx xxxx xxxxxx 19.3 xxxxx 2.3
Control Del:xxxxx xxxxx xxxxxx 10.0 xxxxx xxxxxx xxxxxx xxxx xxxxxx 557.1 xxxxx 14.5
LOS by Move: \* \* \* B \* \* \* \* \* F \* \* B
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx
SharedQueue:xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx
Shrd ConDel:xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx
Shared LOS: \*
ApproachDel: xxxxxx xxxxxx xxxxxx 250.5
ApproachLOS: \*
Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) EXISTING CONDITIONS WEEKDAY (2:00-4:00) PEAK HOUR

Level Of Service Computation Report
ICU l(Loss as Cycle Length %) Method (Future Volume Alternative)
Intersection #2 CREST DRIVE - COLLEGE ENTRANCE/PALOS VERDES DRIVE EAST
Cycle (sec): 100 Critical Vol./Cap.(X): 0.477
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxxx
Optimal Cycle: 32 Level Of Service: A
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Permitted Permitted Permitted Permitted
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0 0
Lanes: 0 0 1 0 0 0 1 0 0 1 1 0 2 0 1 1 0 2 0 1
Volume Module:
Base Vol: 21 7 116 229 4 52 32 141 21 58 135 143
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 21 7 116 229 4 52 32 141 21 58 135 143
Added Vol: 2 0 5 0 0 0 0 0 2 7 0 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 23 7 121 229 4 52 32 141 23 65 135 143
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 25 8 132 249 4 57 35 153 25 71 147 155
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 25 8 132 249 4 57 35 153 25 71 147 155
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 25 8 132 249 4 57 35 153 25 71 147 155
Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 0.15 0.05 0.80 0.98 0.02 1.00 1.00 2.00 1.00 1.00 2.00 1.00
Final Sat.: 244 74 1282 1573 27 1600 1600 3200 1600 1600 3200 1600
Capacity Analysis Module:
Vol/Sat: 0.02 0.10 0.10 0.16 0.16 0.04 0.02 0.05 0.02 0.04 0.05 0.10
Crit Moves: \*\*\*\* \*\*\*\* \*\*\*\* \*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS
WEEKDAY (2:00-4:00) PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

\*\*\*\*\*

Intersection #3 PALOS VERDES DRIVE EAST/PALOS VERDES DRIVE SOUTH

\*\*\*\*\*

Average Delay (sec/veh): 4.1 Worst Case Level Of Service: C[ 20.4]

\*\*\*\*\*

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled

Rights: Include Include Include Include

Lanes: 0 0 0 0 0 1 0 0 0 1 1 0 1 0 0 0 0 1 0 1

-----|-----|-----|-----|

Volume Module:

Table with 16 columns and 10 rows showing traffic volume data: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume.

-----|-----|-----|-----|

Critical Gap Module:

Critical Gp:xxxxx xxxxx xxxxx 6.4 xxxxx 6.2 4.1 xxxxx xxxxxx xxxxx xxxxx xxxxxx

FollowUpTim:xxxxx xxxxx xxxxx 3.5 xxxxx 3.3 2.2 xxxxx xxxxxx xxxxx xxxxx xxxxxx

-----|-----|-----|-----|

Capacity Module:

Cnflct Vol: xxxxx xxxxx xxxxxx 1365 xxxxx 380 434 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Potent Cap.: xxxxx xxxxx xxxxxx 164 xxxxx 671 1137 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Move Cap.: xxxxx xxxxx xxxxxx 140 xxxxx 671 1137 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Volume/Cap: xxxxx xxxxx xxxxx 0.36 xxxxx 0.21 0.19 xxxxx xxxxx xxxxx xxxxx xxxxxx

-----|-----|-----|-----|

Level Of Service Module:

2Way95thQ: xxxxx xxxxx xxxxxx 1.5 xxxxx 0.8 0.7 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Control Del:xxxxxx xxxxx xxxxxx 44.3 xxxxx 11.8 8.9 xxxxx xxxxxx xxxxxx xxxxx xxxxxx

LOS by Move: \* \* \* E \* B A \* \* \* \* \* \*

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx

SharedQueue:xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx

Shrd ConDel:xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx

Shared LOS: \* \* \* \* \* \* \* \* \* \* \* \* \* \* \* \*

ApproachDel: xxxxxxxx 20.4 xxxxxxxx xxxxxxxx

ApproachLOS: \* C \* \*

\*\*\*\*\*

Note: Queue reported is the number of cars per lane.

\*\*\*\*\*

HCS+: Unsignalized Intersections Release 5.2

TWO-WAY STOP CONTROL SUMMARY

Analyst: DK  
 Agency/Co.: RBF  
 Date Performed: 7/23/2007  
 Analysis Time Period: Weekday Afternoon Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: Existing  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: Miraleste Dr  
 Intersection Orientation: NS  
 Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	Northbound			Southbound		
	L	T	R	L	T	R
Volume	11	281	42	253	399	5
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	11	305	45	274	433	5
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L	TR	
Upstream Signal?	No			No		

Minor Street: Approach Movement	Westbound			Eastbound			
	L	T	R	L	T	R	
Volume	52	20	199	1	2	14	
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly Flow Rate, HFR	56	21	216	1	2	15	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)	0			0			
Flared Approach: Exists?/Storage				/	No		/
Lanes	0	1	1	0	1	0	
Configuration	LT R			LTR			

Delay, Queue Length, and Level of Service

Approach Movement Lane Config	NB	SB	Westbound		Eastbound		
	1	4	7	8	9	10	11 12
	LT	L	LT		R		LTR
v (vph)	11	274	77		216		18
C(m) (vph)	1133	1220	206		740		424
v/c	0.01	0.22	0.37		0.29		0.04
95% queue length	0.03	0.86	1.63		1.21		0.13
Control Delay	8.2	8.8	32.5		11.9		13.9
LOS	A	A	D		B		B
Approach Delay				17.3			
Approach LOS				C			

HCS+: Unsignalized Intersections Release 5.2

Phone:  
 E-Mail:

Fax:

TWO-WAY STOP CONTROL(TWSC) ANALYSIS

Analyst: DK  
 Agency/Co.: RBF  
 Date Performed: 7/23/2007  
 Analysis Time Period: Weekday Afternoon Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: Existing  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: Miraleste Dr  
 Intersection Orientation: NS  
 Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street Movements	Northbound			Southbound		
	L	T	R	L	T	R
Volume	11	281	42	253	399	5
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	3	76	11	69	108	1
Hourly Flow Rate, HFR	11	305	45	274	433	5
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L	TR	
Upstream Signal?	No			No		

Minor Street Movements	Westbound			Eastbound			
	L	T	R	L	T	R	
Volume	52	20	199	1	2	14	
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	
Peak-15 Minute Volume	14	5	54	0	1	4	
Hourly Flow Rate, HFR	56	21	216	1	2	15	
Percent Heavy Vehicles	0	0	0	0	0	0	
Percent Grade (%)	0			0			
Flared Approach: Exists?/Storage				/	No		/
RT Channelized?	No						
Lanes	0	1	1	0	1	0	
Configuration	LT R			LTR			

Pedestrian Volumes and Adjustments

Movements	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)	12.0	12.0	12.0	12.0
Walking Speed (ft/sec)	4.0	4.0	4.0	4.0
Percent Blockage	0	0	0	0

Upstream Signal Data							
	Prog. Flow vph	Sat Flow vph	Arrival Type	Green Time sec	Cycle Length sec	Prog. Speed mph	Distance to Signal feet
S2	Left-Turn						
	Through						
S5	Left-Turn						
	Through						

Worksheet 3-Data for Computing Effect of Delay to Major Street Vehicles

	Movement 2	Movement 5
Shared ln volume, major th vehicles:	305	
Shared ln volume, major rt vehicles:	0	
Sat flow rate, major th vehicles:	1700	
Sat flow rate, major rt vehicles:	1700	
Number of major street through lanes:	1	

Worksheet 4-Critical Gap and Follow-up Time Calculation

Critical Gap Calculation									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
t(c,base)	4.1	4.1	7.1	6.5	6.2	7.1	6.5	6.2	
t(c,hv)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
P(hv)	0	0	0	0	0	0	0	0	
t(c,g)			0.20	0.20	0.10	0.20	0.20	0.10	
Grade/100			0.00	0.00	0.00	0.00	0.00	0.00	
t(3,lt)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
t(c,T):	1-stage	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
	2-stage	0.00	0.00	1.00	1.00	0.00	1.00	1.00	
t(c)	1-stage	4.1	4.1	7.1	6.5	7.1	6.5	6.2	
	2-stage	4.1	4.1	6.1	5.5	6.2	6.1	5.5	

Follow-Up Time Calculations									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
t(f,base)	2.20	2.20	3.50	4.00	3.30	3.50	4.00	3.30	
t(f,HV)	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	
P(HV)	0	0	0	0	0	0	0	0	
t(f)	2.2	2.2	3.5	4.0	3.3	3.5	4.0	3.3	

Worksheet 5-Effect of Upstream Signals

Computation 1-Queue Clearance Time at Upstream Signal				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
V prog				
Total Saturation Flow Rate, s (vph)				
Arrival Type				
Effective Green, g (sec)				
Cycle Length, C (sec)				
Rp (from Exhibit 16-11)				
Proportion vehicles arriving on green P				
g(q1)				
g(q2)				
g(q)				

Computation 2-Proportion of TWSC Intersection Time blocked				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
alpha				
beta				
Travel time, t(a) (sec)				
Smoothing Factor, F				
Proportion of conflicting flow, f				
Max platooned flow, V(c,max)				
Min platooned flow, V(c,min)				
Duration of blocked period, t(p)				
Proportion time blocked, p		0.000		0.000

Computation 3-Platoon Event Periods	Result
p(2)	0.000
p(5)	0.000
p(dom)	
p(subo)	
Constrained or unconstrained?	

Proportion unblocked for minor movements, p(x)	(1)	(2)	(3)
	Single-stage Process	Two-Stage Stage I	Process Stage II
p(1)			
p(4)			
p(7)			
p(8)			
p(9)			
p(10)			
p(11)			
p(12)			

Computation 4 and 5 Single-Stage Process									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
V c,x	438	350	1319	1313	305	1452	1356	436	
s									
Px									
V c,u,x									
C r,x									
C plat,x									

	7		8		10		11	
	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2
V(c,x)	327	992	327	986	984	468	984	372
s		1500		1500		1500		1500
P(x)								
V(c,u,x)								
C(r,x)								
C(plat,x)								

Worksheet 6-Impedance and Capacity Equations

Step 1: RT from Minor St.	9	12
Conflicting Flows	305	436
Potential Capacity	740	625
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	740	625
Probability of Queue free St.	0.71	0.98
Step 2: LT from Major St.	4	1
Conflicting Flows	350	438
Potential Capacity	1220	1133
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	1220	1133
Probability of Queue free St.	0.78	0.99
Maj L-Shared Prob Q free St.		0.99
Step 3: TH from Minor St.	8	11
Conflicting Flows	1313	1356
Potential Capacity	160	151
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.77	0.77
Movement Capacity	123	116
Probability of Queue free St.	0.91	0.99
Step 4: LT from Minor St.	7	10
Conflicting Flows	1319	1452
Potential Capacity	135	110
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.76	0.70
Maj. L, Min T Adj. Imp Factor.	0.81	0.76
Cap. Adj. factor due to Impeding mvmnt	0.79	0.54
Movement Capacity	107	60

Worksheet 7-Computation of the Effect of Two-stage Gap Acceptance

Step 3: TH from Minor St.	8	11
Part 1 - First Stage		
Conflicting Flows	327	984
Potential Capacity	651	329
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.99	0.78
Movement Capacity	643	255
Probability of Queue free St.	0.97	0.99
Part 2 - Second Stage		
Conflicting Flows	986	372
Potential Capacity	328	622
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.78	0.99
Movement Capacity	254	615
Part 3 - Single Stage		
Conflicting Flows	1313	1356
Potential Capacity	160	151
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.77	0.77
Movement Capacity	123	116
Result for 2 stage process:		
a	0.95	0.95

Y	4.33	0.62
C t	226	217
Probability of Queue free St.	0.91	0.99
Step 4: LT from Minor St.	7	10
Part 1 - First Stage		
Conflicting Flows	327	984
Potential Capacity	690	302
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.99	0.78
Movement Capacity	682	234
Part 2 - Second Stage		
Conflicting Flows	992	468
Potential Capacity	299	579
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.75	0.68
Movement Capacity	225	392
Part 3 - Single Stage		
Conflicting Flows	1319	1452
Potential Capacity	135	110
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.76	0.70
Maj. L, Min T Adj. Imp Factor.	0.81	0.76
Cap. Adj. factor due to Impeding mvmnt	0.79	0.54
Movement Capacity	107	60
Results for Two-stage process:		
a	0.95	0.95
Y	5.37	3.00
C t	200	108

Worksheet 8-Shared Lane Calculations

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (vph)	56	21	216	1	2	15
Movement Capacity (vph)	200	226	740	108	217	625
Shared Lane Capacity (vph)	206				424	

Worksheet 9-Computation of Effect of Flared Minor Street Approaches

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
C sep	200	226	740	108	217	625
Volume	56	21	216	1	2	15
Delay						
Q sep						
Q sep +1						
round (Qsep +1)						
n max						
C sh	206				424	
SUM C sep						
n						
C act						

Worksheet 10-Delay, Queue Length, and Level of Service

Movement	1	4	7	8	9	10	11	12
Lane Config	LT	L	LT		R		LTR	
v (vph)	11	274	77		216		18	
C(m) (vph)	1133	1220	206		740		424	
v/c	0.01	0.22	0.37		0.29		0.04	
95% queue length	0.03	0.86	1.63		1.21		0.13	
Control Delay	8.2	8.8	32.5		11.9		13.9	
LOS	A	A	D		B		B	
Approach Delay				17.3			13.9	
Approach LOS				C			B	

Worksheet 11-Shared Major LT Impedance and Delay

	Movement 2	Movement 5
p(oj)	0.99	0.78
v(i1), Volume for stream 2 or 5	305	
v(i2), Volume for stream 3 or 6	0	
s(i1), Saturation flow rate for stream 2 or 5	1700	
s(i2), Saturation flow rate for stream 3 or 6	1700	
P*(oj)	0.99	
d(M,LT), Delay for stream 1 or 4	8.2	8.8
N, Number of major street through lanes	1	
d(rank,1) Delay for stream 2 or 5	0.1	

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS
SATURDAY (11:00-1:00) PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

\*\*\*\*\*
Intersection #1 PALOS VERDES DRIVE EAST/MIRALESTE DRIVE
\*\*\*\*\*

Average Delay (sec/veh): 11.2 Worst Case Level Of Service: D [ 25.9 ]

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 0 0 0 1 0 1 0 1 0 0 0 0 0 0 0 1 0 0 0 1

Volume Module:

Base Vol: 0 161 123 296 158 0 0 0 0 0 127 0 259
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 0 161 123 296 158 0 0 0 0 0 127 0 259
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 0 175 134 322 172 0 0 0 0 0 138 0 282
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 0 175 134 322 172 0 0 0 0 0 138 0 282

Critical Gap Module:

Critical Gp:xxxxx xxxx xxxxxx 4.1 xxxx xxxxxx xxxxxx xxxx xxxxxx 6.4 xxxx 6.2
FollowUpTim:xxxxx xxxx xxxxxx 2.2 xxxx xxxxxx xxxxxx xxxx xxxxxx 3.5 xxxx 3.3

Capacity Module:

Cnflct Vol: xxxx xxxx xxxxxx 309 xxxx xxxxxx xxxxxx xxxx xxxxxx 1057 xxxx 242
Potent Cap.: xxxx xxxx xxxxxx 1263 xxxx xxxxxx xxxxxx xxxx xxxxxx 251 xxxx 802
Move Cap.: xxxx xxxx xxxxxx 1263 xxxx xxxxxx xxxxxx xxxx xxxxxx 202 xxxx 802
Volume/Cap: xxxx xxxx xxxxxx 0.25 xxxx xxxxxx xxxxxx xxxx xxxxxx 0.68 xxxx 0.35

Level Of Service Module:

2Way95thQ: xxxx xxxx xxxxxx 1.0 xxxx xxxxxx xxxxxx xxxx xxxxxx 4.2 xxxx 1.6
Control Del:xxxxx xxxx xxxxxx 8.8 xxxx xxxxxx xxxxxx xxxx xxxxxx 54.3 xxxx 11.9
LOS by Move: \* \* \* A \* \* \* \* \* F \* \* B
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxx xxxx xxxxxx xxxx xxxx xxxxxx xxxxxx xxxx xxxxxx xxxxxx xxxx xxxxxx
SharedQueue:xxxxx xxxx xxxxxx xxxxxx xxxx xxxxxx xxxxxx xxxx xxxxxx xxxxxx xxxx xxxxxx
Shrd ConDel:xxxxx xxxx xxxxxx xxxxxx xxxx xxxxxx xxxxxx xxxx xxxxxx xxxxxx xxxx xxxxxx
Shared LOS: \* \* \* \* \* \* \* \* \* \* \* \* \* \* \* \* \*
ApproachDel: xxxxxx xxxxxx xxxxxx 25.9
ApproachLOS: \* \* \* \* \* D

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS
SATURDAY (11:00-1:00) PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

\*\*\*\*\*
Intersection #2 CREST DRIVE - COLLEGE ENTRANCE/PALOS VERDES DRIVE EAST
\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.198
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxxx
Optimal Cycle: 22 Level Of Service: A

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Permitted Permitted Permitted Permitted
Rights: Include Include Include Include
Lanes: 0 0 0 1 0 0 1 0 0 1 1 0 2 0 1 1 0 2 0 1

Volume Module:

Base Vol: 0 3 4 65 0 17 19 143 3 1 106 53
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 0 3 4 65 0 17 19 143 3 1 106 53
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 0 3 4 71 0 18 21 155 3 1 115 58
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 0 3 4 71 0 18 21 155 3 1 115 58
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 0 3 4 71 0 18 21 155 3 1 115 58

Saturation Flow Module:

Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 0.00 0.43 0.57 1.00 0.00 1.00 1.00 2.00 1.00 1.00 2.00 1.00
Final Sat.: 0 686 914 1600 0 1600 1600 3200 1600 1600 3200 1600

Capacity Analysis Module:

Vol/Sat: 0.00 0.00 0.00 0.04 0.00 0.01 0.01 0.05 0.00 0.00 0.04 0.04
Crit Moves: \*\*\*\* \*\*\*\* \*\*\*\* \*\*\*\*

\*\*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS
SATURDAY (11:00-1:00) PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

\*\*\*\*\*

Intersection #3 PALOS VERDES DRIVE EAST/PALOS VERDES DRIVE SOUTH

\*\*\*\*\*

Average Delay (sec/veh): 2.0 Worst Case Level Of Service: B[ 14.9]

\*\*\*\*\*

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled

Rights: Include Include Include Include

Lanes: 0 0 0 0 0 1 0 0 0 1 1 0 1 0 0 0 0 1 0 1

-----|-----|-----|-----|

Volume Module:

Base Vol: 0 0 0 30 0 70 75 406 0 0 412 55

Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Initial Bse: 0 0 0 30 0 70 75 406 0 0 412 55

User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92

PHF Volume: 0 0 0 33 0 76 82 441 0 0 448 60

Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0

FinalVolume: 0 0 0 33 0 76 82 441 0 0 448 60

-----|-----|-----|-----|

Critical Gap Module:

Critical Gp:xxxxx xxxx xxxxx 6.4 xxxx 6.2 4.1 xxxx xxxxxx xxxxx xxxx xxxxxx

FollowUpTim:xxxxx xxxx xxxxxx 3.5 xxxx 3.3 2.2 xxxx xxxxxx xxxxx xxxx xxxxxx

-----|-----|-----|-----|

Capacity Module:

Cnflct Vol: xxxx xxxx xxxxx 1052 xxxx 448 508 xxxx xxxxxx xxxx xxxx xxxxxx

Potent Cap.: xxxx xxxx xxxxxx 253 xxxx 615 1068 xxxx xxxxxx xxxx xxxx xxxxxx

Move Cap.: xxxx xxxx xxxxxx 238 xxxx 615 1068 xxxx xxxxxx xxxx xxxx xxxxxx

Volume/Cap: xxxx xxxx xxxxx 0.14 xxxx 0.12 0.08 xxxx xxxxxx xxxx xxxx xxxxxx

-----|-----|-----|-----|

Level Of Service Module:

2Way95thQ: xxxx xxxx xxxxxx 0.5 xxxx 0.4 0.2 xxxx xxxxxx xxxx xxxx xxxxxx

Control Del:xxxxx xxxx xxxxxx 22.5 xxxx 11.7 8.7 xxxx xxxxxx xxxxx xxxx xxxxxx

LOS by Move: \* \* \* C \* B A \* \* \* \* \* \*

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxxx xxxx xxxxxx xxxx xxxx xxxxxx xxxx xxxx xxxxxx xxxx xxxx xxxxxx

SharedQueue:xxxxx xxxx xxxxxx xxxxx xxxx xxxxxx xxxxxx xxxxx xxxx xxxxxx xxxxx xxxx xxxxxx

Shrd ConDel:xxxxx xxxx xxxxxx xxxxx xxxx xxxxxx xxxxxx xxxxx xxxx xxxxxx xxxxx xxxx xxxxxx

Shared LOS: \* \* \* \* \* \* \* \* \* \* \* \* \* \* \* \*

ApproachDel: xxxxxxx 14.9 xxxxxxx xxxxxxx

ApproachLOS: \* B \* \*

\*\*\*\*\*

Note: Queue reported is the number of cars per lane.

\*\*\*\*\*

HCS+: Unsignalized Intersections Release 5.2

TWO-WAY STOP CONTROL SUMMARY

Analyst: DK  
 Agency/Co.: RBF  
 Date Performed: 7/23/2007  
 Analysis Time Period: Saturday Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: Existing  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: Miraleste Dr  
 Intersection Orientation: NS

Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	Northbound			Southbound		
	1 L	2 T	3 R	4 L	5 T	6 R
Volume	3	259	51	176	275	7
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	3	281	55	191	298	7
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L		TR
Upstream Signal?	No			No		

Minor Street: Approach Movement	Westbound			Eastbound		
	7 L	8 T	9 R	10 L	11 T	12 R
Volume	0	8	119	0	4	0
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	0	8	129	0	4	0
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage				/ No /		
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Delay, Queue Length, and Level of Service

Approach Movement Lane Config	NB	SB	Westbound		Eastbound		
	1 LT	4 L	7 LT	8	9 R	10	11 LTR
v (vph)	3	191	8		129		4
C(m) (vph)	1267	1235	345		763		322
v/c	0.00	0.15	0.02		0.17		0.01
95% queue length	0.01	0.55	0.07		0.61		0.04
Control Delay	7.8	8.4	15.7		10.7		16.3
LOS	A	A	C		B		C
Approach Delay				11.0			
Approach LOS				B			

HCS+: Unsignalized Intersections Release 5.2

Phone:  
 E-Mail:  
 Fax:

TWO-WAY STOP CONTROL(TWSC) ANALYSIS

Analyst: DK  
 Agency/Co.: RBF  
 Date Performed: 7/23/2007  
 Analysis Time Period: Saturday Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: Existing  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: Miraleste Dr  
 Intersection Orientation: NS

Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street Movements	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	3	259	51	176	275	7
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	1	70	14	48	75	2
Hourly Flow Rate, HFR	3	281	55	191	298	7
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L		TR
Upstream Signal?	No			No		

Minor Street Movements	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	0	8	119	0	4	0
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	0	2	32	0	1	0
Hourly Flow Rate, HFR	0	8	129	0	4	0
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage				/ No /		
RT Channelized?	No					
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Pedestrian Volumes and Adjustments

Movements	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)	12.0	12.0	12.0	12.0
Walking Speed (ft/sec)	4.0	4.0	4.0	4.0
Percent Blockage	0	0	0	0

Upstream Signal Data							
	Prog. Flow vph	Sat Flow vph	Arrival Type	Green Time sec	Cycle Length sec	Prog. Speed mph	Distance to Signal feet
S2	Left-Turn						
	Through						
S5	Left-Turn						
	Through						

Worksheet 3-Data for Computing Effect of Delay to Major Street Vehicles

	Movement 2	Movement 5
Shared ln volume, major th vehicles:	281	
Shared ln volume, major rt vehicles:	0	
Sat flow rate, major th vehicles:	1700	
Sat flow rate, major rt vehicles:	1700	
Number of major street through lanes:	1	

Worksheet 4-Critical Gap and Follow-up Time Calculation

Critical Gap Calculation								
Movement	1	4	7	8	9	10	11	12
	L	L	L	T	R	L	T	R
t(c,base)	4.1	4.1	7.1	6.5	6.2	7.1	6.5	6.2
t(c,hv)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
P(hv)	0	0	0	0	0	0	0	0
t(c,g)			0.20	0.20	0.10	0.20	0.20	0.10
Grade/100			0.00	0.00	0.00	0.00	0.00	0.00
t(3,lt)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
t(c,T):	1-stage	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2-stage	0.00	0.00	1.00	1.00	0.00	1.00	0.00
t(c)	1-stage	4.1	4.1	7.1	6.5	7.1	6.5	6.2
	2-stage	4.1	4.1	6.1	5.5	6.1	5.5	6.2

Follow-Up Time Calculations								
Movement	1	4	7	8	9	10	11	12
	L	L	L	T	R	L	T	R
t(f,base)	2.20	2.20	3.50	4.00	3.30	3.50	4.00	3.30
t(f,HV)	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
P(HV)	0	0	0	0	0	0	0	0
t(f)	2.2	2.2	3.5	4.0	3.3	3.5	4.0	3.3

Worksheet 5-Effect of Upstream Signals

Computation 1-Queue Clearance Time at Upstream Signal				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
V prog				
Total Saturation Flow Rate, s (vph)				
Arrival Type				
Effective Green, g (sec)				
Cycle Length, C (sec)				
Rp (from Exhibit 16-11)				
Proportion vehicles arriving on green P				
g(q1)				
g(q2)				
g(q)				

Computation 2-Proportion of TWSC Intersection Time blocked				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
alpha				
beta				
Travel time, t(a) (sec)				
Smoothing Factor, F				
Proportion of conflicting flow, f				
Max platooned flow, V(c,max)				
Min platooned flow, V(c,min)				
Duration of blocked period, t(p)				
Proportion time blocked, p		0.000		0.000

Computation 3-Platoon Event Periods	Result
p(2)	0.000
p(5)	0.000
p(dom)	
p(subo)	
Constrained or unconstrained?	

Proportion unblocked for minor movements, p(x)	(1)	(2)	(3)
	Single-stage Process	Two-Stage Stage I	Process Stage II
p(1)			
p(4)			
p(7)			
p(8)			
p(9)			
p(10)			
p(11)			
p(12)			

Computation 4 and 5 Single-Stage Process								
Movement	1	4	7	8	9	10	11	12
	L	L	L	T	R	L	T	R
V c,x	305	336	973	974	281	1067	1026	302
s								
Px								
V c,u,x								
C r,x								
C plat,x								

	7		8		10		11	
	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2
V(c,x)	287	686	287	687	684	383	684	342
s		1500		1500		1500		1500
P(x)								
V(c,u,x)								
C(r,x)								
C(plat,x)								

Worksheet 6-Impedance and Capacity Equations

Step 1: RT from Minor St.	9	12
Conflicting Flows	281	302
Potential Capacity	763	742
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	763	742
Probability of Queue free St.	0.83	1.00
Step 2: LT from Major St.	4	1
Conflicting Flows	336	305
Potential Capacity	1235	1267
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	1235	1267
Probability of Queue free St.	0.85	1.00
Maj L-Shared Prob Q free St.		1.00
Step 3: TH from Minor St.	8	11
Conflicting Flows	974	1026
Potential Capacity	254	237
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.84	0.84
Movement Capacity	214	200
Probability of Queue free St.	0.98	0.99
Step 4: LT from Minor St.	7	10
Conflicting Flows	973	1067
Potential Capacity	233	202
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.83	0.82
Maj. L, Min T Adj. Imp Factor.	0.87	0.86
Cap. Adj. factor due to Impeding mvmnt	0.87	0.72
Movement Capacity	203	145

Worksheet 7-Computation of the Effect of Two-stage Gap Acceptance

Step 3: TH from Minor St.	8	11
Part 1 - First Stage		
Conflicting Flows	287	684
Potential Capacity	678	452
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	1.00	0.85
Movement Capacity	676	382
Probability of Queue free St.	0.99	0.99
Part 2 - Second Stage		
Conflicting Flows	687	342
Potential Capacity	450	642
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.85	1.00
Movement Capacity	380	640
Part 3 - Single Stage		
Conflicting Flows	974	1026
Potential Capacity	254	237
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.84	0.84
Movement Capacity	214	200
Result for 2 stage process:		
a	0.95	0.95

Y	2.83	0.73
C t	345	322
Probability of Queue free St.	0.98	0.99
Step 4: LT from Minor St.	7	10
Part 1 - First Stage		
Conflicting Flows	287	684
Potential Capacity	725	442
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	1.00	0.85
Movement Capacity	723	374
Part 2 - Second Stage		
Conflicting Flows	686	383
Potential Capacity	441	644
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.84	0.82
Movement Capacity	369	527
Part 3 - Single Stage		
Conflicting Flows	973	1067
Potential Capacity	233	202
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.83	0.82
Maj. L, Min T Adj. Imp Factor.	0.87	0.86
Cap. Adj. factor due to Impeding mvmnt	0.87	0.72
Movement Capacity	203	145
Results for Two-stage process:		
a	0.95	0.95
Y	3.19	1.20
C t	337	269

Worksheet 8-Shared Lane Calculations

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (vph)	0	8	129	0	4	0
Movement Capacity (vph)	337	345	763	269	322	742
Shared Lane Capacity (vph)	345				322	

Worksheet 9-Computation of Effect of Flared Minor Street Approaches

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
C sep	337	345	763	269	322	742
Volume	0	8	129	0	4	0
Delay						
Q sep						
Q sep +1						
round (Qsep +1)						
n max						
C sh	345				322	
SUM C sep						
n						
C act						

Worksheet 10-Delay, Queue Length, and Level of Service

Movement	1	4	7	8	9	10	11	12
Lane Config	LT	L	LT		R		LTR	
v (vph)	3	191	8		129		4	
C(m) (vph)	1267	1235	345		763		322	
v/c	0.00	0.15	0.02		0.17		0.01	
95% queue length	0.01	0.55	0.07		0.61		0.04	
Control Delay	7.8	8.4	15.7		10.7		16.3	
LOS	A	A	C		B		C	
Approach Delay				11.0			16.3	
Approach LOS				B			C	

Worksheet 11-Shared Major LT Impedance and Delay

	Movement 2	Movement 5
p(oj)	1.00	0.85
v(i1), Volume for stream 2 or 5	281	
v(i2), Volume for stream 3 or 6	0	
s(i1), Saturation flow rate for stream 2 or 5	1700	
s(i2), Saturation flow rate for stream 3 or 6	1700	
P*(oj)	1.00	
d(M,LT), Delay for stream 1 or 4	7.8	8.4
N, Number of major street through lanes	1	
d(rank,1) Delay for stream 2 or 5	0.0	

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS - CITY OF LOS ANGELES
AM PEAK HOUR

Level Of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

Intersection #7 WESTERN AVENUE/CAPITOL DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 0.912
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: E

Table with 4 columns: Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns representing traffic volumes for different approaches and movements. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module:

Table with 12 columns for saturation flow and adjustment factors. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns for capacity analysis. Rows include Vol/Sat, Crit Volume, and Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS - CITY OF LOS ANGELES
AM PEAK HOUR

Level Of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

Intersection #8 WESTERN AVENUE/CRESTWOOD STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.809
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 105 Level Of Service: D

Table with 4 columns: Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns representing traffic volumes for different approaches and movements. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module:

Table with 12 columns for saturation flow and adjustment factors. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns for capacity analysis. Rows include Vol/Sat, Crit Volume, and Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS - CITY OF LOS ANGELES
AM PEAK HOUR

Level Of Service Computation Report
Circular 212 Planning Method (Future Volume Alternative)

Intersection #9 WESTERN AVENUE/1ST STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 1.414
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module table with 12 columns and 14 rows including Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume.

Saturation Flow Module table with 12 columns and 4 rows including Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module table with 12 columns and 4 rows including Vol/Sat, Crit Volume, Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS - CITY OF LOS ANGELES
AM PEAK HOUR

Level Of Service Computation Report
Circular 212 Planning Method (Future Volume Alternative)

Intersection #10 WESTERN AVENUE/9TH STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.607
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 51 Level Of Service: B

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module table with 12 columns and 14 rows including Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume.

Saturation Flow Module table with 12 columns and 4 rows including Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module table with 12 columns and 4 rows including Vol/Sat, Crit Volume, Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS - CITY OF LOS ANGELES
AM PEAK HOUR

Level Of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

\*\*\*\*\*

Intersection #11 WESTERN AVENUE/WEST 25TH STREET

\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.681
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 63 Level Of Service: B

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Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 3 rows: Movement, Control, Rights. Includes lane configurations like L-T-R and l-0-2-0-1.

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Volume Module: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume.

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Saturation Flow Module: Sat/Lane, Adjustment, Lanes, Final Sat.

-----|-----|-----|-----|

Capacity Analysis Module: Vol/Sat, Crit Volume, Crit Moves.

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MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS - CITY OF LOS ANGELES
PM PEAK HOUR

Level Of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

Intersection #7 WESTERN AVENUE/CAPITOL DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 0.788
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 94 Level Of Service: C

Table with 4 columns: Approach (North, South, East, West Bound), Movement (L, T, R), Control (Protected, Permitted), Rights (Include), Min. Green, Lanes.

Volume Module:

Table with 12 columns for traffic volume and 12 columns for adjustment factors (Growth, Initial, Added, PasserBy, Initial Fut, User, PHF, Reduced, PCE, MLF, Final).

Saturation Flow Module:

Table with 12 columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns for Vol/Sat, Crit Volume, and Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS - CITY OF LOS ANGELES
PM PEAK HOUR

Level Of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

Intersection #8 WESTERN AVENUE/CRESTWOOD STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.759
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 83 Level Of Service: C

Table with 4 columns: Approach (North, South, East, West Bound), Movement (L, T, R), Control (Permitted), Rights (Include), Min. Green, Lanes.

Volume Module:

Table with 12 columns for traffic volume and 12 columns for adjustment factors (Growth, Initial, Added, PasserBy, Initial Fut, User, PHF, Reduced, PCE, MLF, Final).

Saturation Flow Module:

Table with 12 columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns for Vol/Sat, Crit Volume, and Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS - CITY OF LOS ANGELES
PM PEAK HOUR

Level of Service Computation Report
Circular 212 Planning Method (Future Volume Alternative)
Intersection #9 WESTERN AVENUE/1ST STREET
Cycle (sec): 100 Critical Vol./Cap.(X): 1.317
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Permitted Prot+Permit Permitted Prot+Permit
Rights: Ovl Include Include Ovl
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Lanes: 1 0 2 0 1 1 0 1 1 0 1 0 1 0 1 0 1
Volume Module:
Base Vol: 36 1259 215 178 1458 61 42 184 21 714 276 156
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 36 1259 215 178 1458 61 42 184 21 714 276 156
Added Vol: 0 0 0 0 0 2 1 0 0 0 1 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 36 1259 215 178 1458 63 43 184 21 714 277 156
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 39 1368 234 193 1585 68 47 200 23 776 301 170
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 39 1368 234 193 1585 68 47 200 23 776 301 170
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 39 1368 234 193 1585 68 47 200 23 776 301 170
Saturation Flow Module:
Sat/Lane: 1425 1425 1425 1425 1425 1425 1425 1425 1425 1425 1425
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 1.00 2.00 1.00 1.00 1.92 0.08 1.00 0.90 0.10 1.00 1.00 1.00
Final Sat.: 1425 2850 1425 1425 2732 118 1425 1279 146 1425 1425 1425
Capacity Analysis Module:
Vol/Sat: 0.03 0.48 0.16 0.14 0.58 0.58 0.03 0.16 0.16 0.54 0.21 0.12
Crit Volume: 684 193 223 776
Crit Moves: \*\*\*\* \*\*\*\* \*\*\*\* \*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS - CITY OF LOS ANGELES
PM PEAK HOUR

Level of Service Computation Report
Circular 212 Planning Method (Future Volume Alternative)
Intersection #10 WESTERN AVENUE/9TH STREET
Cycle (sec): 100 Critical Vol./Cap.(X): 0.804
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 102 Level Of Service: D
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Protected Protected Permitted Permitted
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Lanes: 1 0 2 0 1 1 0 2 0 1 1 0 1 0 1 0 1
Volume Module:
Base Vol: 144 640 62 72 1076 51 38 164 213 159 117 50
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 144 640 62 72 1076 51 38 164 213 159 117 50
Added Vol: 0 0 0 0 0 0 0 0 0 0 1 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 144 640 62 72 1076 51 38 164 213 159 118 50
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 157 696 67 78 1170 55 41 178 232 173 128 54
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 157 696 67 78 1170 55 41 178 232 173 128 54
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 157 696 67 78 1170 55 41 178 232 173 128 54
Saturation Flow Module:
Sat/Lane: 1425 1425 1425 1425 1425 1425 1425 1425 1425 1425 1425
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 1.00 2.00 1.00 1.00 2.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Final Sat.: 1425 2850 1425 1425 2850 1425 1425 1425 1425 1425 1425 1425
Capacity Analysis Module:
Vol/Sat: 0.11 0.24 0.05 0.05 0.41 0.04 0.03 0.13 0.16 0.12 0.09 0.04
Crit Volume: 157 585 232 173
Crit Moves: \*\*\*\* \*\*\*\* \*\*\*\* \*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS - CITY OF LOS ANGELES
PM PEAK HOUR

Level Of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

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Intersection #11 WESTERN AVENUE/WEST 25TH STREET

\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.622
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 53 Level Of Service: B

\*\*\*\*\*

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 3 sub-columns for movements (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

-----|-----|-----|-----|

Volume Module table with 12 columns representing different traffic movements and 12 rows of volume data including Base Vol, Growth Adj, Initial Bse, etc.

-----|-----|-----|-----|

Saturation Flow Module table with 12 columns for movements and 4 rows for Sat/Lane, Adjustment, Lanes, and Final Sat.

-----|-----|-----|-----|

Capacity Analysis Module table with 12 columns for movements and 4 rows for Vol/Sat, Crit Volume, and Crit Moves.

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MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS - CALTRANS
AM PEAK HOUR

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #6 WESTERN AVENUE/TOSCANINI DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 0.668
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 17.5
Optimal Cycle: 50 Level Of Service: B

Table with 4 columns: Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module table with 12 columns for different traffic movements and 12 rows for various volume metrics like Base Vol, Growth Adj, etc.

Saturation Flow Module table with 12 columns for movements and 12 rows for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module table with 12 columns for movements and 12 rows for Vol/Sat, Crit Moves, Green/Cycle, etc.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS - CALTRANS
AM PEAK HOUR

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #7 WESTERN AVENUE/CAPITOL DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 0.792
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 23.0
Optimal Cycle: 70 Level Of Service: C

Table with 4 columns: Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module table with 12 columns for different traffic movements and 12 rows for various volume metrics like Base Vol, Growth Adj, etc.

Saturation Flow Module table with 12 columns for movements and 12 rows for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module table with 12 columns for movements and 12 rows for Vol/Sat, Crit Moves, Green/Cycle, etc.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS - CALTRANS
AM PEAK HOUR

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #8 WESTERN AVENUE/CRESTWOOD STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.730
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 17.4
Optimal Cycle: 58 Level Of Service: B

Table with 4 columns: Approach (North, South, East, West Bound) and 4 sub-columns (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns for traffic volume metrics: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume.

Saturation Flow Module:

Table with 12 columns for saturation flow metrics: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with 12 columns for capacity analysis metrics: Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS - CALTRANS
AM PEAK HOUR

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #9 WESTERN AVENUE/1ST STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 1.128
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 68.9
Optimal Cycle: 180 Level Of Service: E

Table with 4 columns: Approach (North, South, East, West Bound) and 4 sub-columns (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns for traffic volume metrics: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume.

Saturation Flow Module:

Table with 12 columns for saturation flow metrics: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with 12 columns for capacity analysis metrics: Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS - CALTRANS
AM PEAK HOUR

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #10 WESTERN AVENUE/9TH STREET

Cycle (sec): 100 Critical Vol./Cap. (X): 0.504
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 21.8
Optimal Cycle: 37 Level Of Service: C

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 sub-columns for movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module table with 11 columns for traffic volume and 11 rows for various metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module table with 11 columns for saturation flow and 4 rows for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module table with 11 columns for capacity and 11 rows for Vol/Sat, Crit Moves, Green/Cycle, etc.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS - CALTRANS
AM PEAK HOUR

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #11 WESTERN AVENUE/WEST 25TH STREET

Cycle (sec): 100 Critical Vol./Cap. (X): 0.607
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 25.6
Optimal Cycle: 44 Level Of Service: C

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 sub-columns for movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module table with 11 columns for traffic volume and 11 rows for various metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module table with 11 columns for saturation flow and 4 rows for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module table with 11 columns for capacity and 11 rows for Vol/Sat, Crit Moves, Green/Cycle, etc.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS - CALTRANS
PM PEAK HOUR

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #6 WESTERN AVENUE/TOSCANINI DRIVE
\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.563
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 9.8
Optimal Cycle: 41 Level Of Service: A

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module: Table with 12 columns (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume) and 12 rows.

Saturation Flow Module: Table with 12 columns (Sat/Lane, Adjustment, Lanes, Final Sat) and 4 rows.

Capacity Analysis Module: Table with 12 columns (Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ) and 9 rows.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS - CALTRANS
PM PEAK HOUR

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #7 WESTERN AVENUE/CAPITOL DRIVE
\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.671
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 22.4
Optimal Cycle: 51 Level Of Service: C

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module: Table with 12 columns (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume) and 12 rows.

Saturation Flow Module: Table with 12 columns (Sat/Lane, Adjustment, Lanes, Final Sat) and 4 rows.

Capacity Analysis Module: Table with 12 columns (Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ) and 9 rows.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS - CALTRANS
PM PEAK HOUR

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #8 WESTERN AVENUE/CRESTWOOD STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.585
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 13.6
Optimal Cycle: 42 Level Of Service: B

Table with 4 columns: Approach (North, South, East, West Bound) and Movement (L, T, R). Includes Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns for traffic volume and 12 rows for various metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module:

Table with 12 columns for saturation flow and 4 rows for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns for capacity analysis and 12 rows for Vol/Sat, Crit Moves, Green/Cycle, etc.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS - CALTRANS
PM PEAK HOUR

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #9 WESTERN AVENUE/1ST STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 1.093
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 86.6
Optimal Cycle: 180 Level Of Service: F

Table with 4 columns: Approach (North, South, East, West Bound) and Movement (L, T, R). Includes Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns for traffic volume and 12 rows for various metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module:

Table with 12 columns for saturation flow and 4 rows for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns for capacity analysis and 12 rows for Vol/Sat, Crit Moves, Green/Cycle, etc.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS - CALTRANS
PM PEAK HOUR

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #10 WESTERN AVENUE/9TH STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.648
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 23.4
Optimal Cycle: 48 Level Of Service: C

Table with columns: Approach (North, South, East, West Bound), Movement (L, T, R), Control (Protected, Permitted), Rights (Include), Min. Green, Lanes.

Table with columns: Volume Module, Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume.

Table with columns: Saturation Flow Module, Sat/Lane, Adjustment, Lanes, Final Sat.

Table with columns: Capacity Analysis Module, Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS - CALTRANS
PM PEAK HOUR

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #11 WESTERN AVENUE/WEST 25TH STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.544
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 24.9
Optimal Cycle: 39 Level Of Service: C

Table with columns: Approach (North, South, East, West Bound), Movement (L, T, R), Control (Permitted, Protected), Rights (Include, Ovl), Min. Green, Lanes.

Table with columns: Volume Module, Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume.

Table with columns: Saturation Flow Module, Sat/Lane, Adjustment, Lanes, Final Sat.

Table with columns: Capacity Analysis Module, Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS - CMP
AM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #6 WESTERN AVENUE/TOSCANINI DRIVE
\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.812
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 69 Level Of Service: D

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module: Table with 12 columns (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume) and 12 rows.

Saturation Flow Module: Table with 12 columns (Sat/Lane, Adjustment, Lanes, Final Sat) and 4 rows.

Capacity Analysis Module: Table with 12 columns (Vol/Sat, Crit Moves) and 4 rows.

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MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING CONDITIONS - CMP
AM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #10 WESTERN AVENUE/9TH STREET
\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.641
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 44 Level Of Service: B

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module: Table with 12 columns (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume) and 12 rows.

Saturation Flow Module: Table with 12 columns (Sat/Lane, Adjustment, Lanes, Final Sat) and 4 rows.

Capacity Analysis Module: Table with 12 columns (Vol/Sat, Crit Moves) and 4 rows.

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MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) EXISTING CONDITIONS - CMP PM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\* Intersection #6 WESTERN AVENUE/TOSCANINI DRIVE \*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.701
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 50 Level Of Service: C

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume.

Saturation Flow Module: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module: Vol/Sat, Crit Moves.

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MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) EXISTING CONDITIONS - CMP PM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\* Intersection #10 WESTERN AVENUE/9TH STREET \*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.816
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 70 Level Of Service: D

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume.

Saturation Flow Module: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module: Vol/Sat, Crit Moves.

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## **Forecast Existing With Project Conditions**

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - RANCHO PALOS VERDES
AM PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #1 PALOS VERDES DRIVE EAST/MIRALESTE DRIVE

Average Delay (sec/veh): 242.5 Worst Case Level Of Service: F[724.1]

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 0 0 0 1 0 1 0 1 0 0 0 0 0 0 1 0 0 0 1

Volume Module:

Base Vol: 0 397 180 379 351 0 0 0 0 156 0 416
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 0 397 180 379 351 0 0 0 0 156 0 416
Added Vol: 0 5 35 0 16 0 0 0 0 105 0 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 0 402 215 379 367 0 0 0 0 261 0 416
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 0 437 234 412 399 0 0 0 0 284 0 452
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 0 437 234 412 399 0 0 0 0 284 0 452

Critical Gap Module:

Critical Gap:xxxxx xxxx xxxxx 4.1 xxxx xxxxx xxxxx xxxx xxxxxx 6.4 xxxx 6.2
FollowUpTim:xxxxx xxxx xxxxxx 2.2 xxxx xxxxx xxxxx xxxx xxxxxx 3.5 xxxx 3.3

Capacity Module:

Cnflct Vol: xxxx xxxx xxxxxx 671 xxxx xxxxxx xxxx xxxx xxxxxx 1777 xxxx 554
Potent Cap.: xxxx xxxx xxxxxx 929 xxxx xxxxxx xxxx xxxx xxxxxx 92 xxxxx 536
Move Cap.: xxxx xxxx xxxxxx 929 xxxx xxxxxx xxxx xxxx xxxxxx 60 xxxxx 536
Volume/Cap: xxxx xxxx xxxxx 0.44 xxxx xxxxx xxxx xxxx xxxxx 4.73 xxxxx 0.84

Level Of Service Module:

2Way95thQ: xxxx xxxx xxxxxx 2.3 xxxx xxxxxx xxxx xxxx xxxxxx 31.4 xxxxx 8.8
Control Del:xxxxx xxxxx xxxxxx 11.9 xxxx xxxxxx xxxxxx xxxx xxxxxx 1817 xxxxx 38.1

LOS by Move: \* \* \* B \* \* \* \* \* F \* \* E

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxxx xxxx xxxxxx xxxx xxxx xxxxxx xxxx xxxx xxxxxx xxxx xxxxx xxxxxx

SharedQueue:xxxxxx xxxxxx xxxxxx xxxxxx xxxxx xxxx xxxxxx xxxxx xxxxx xxxxx xxxxxx

Shrd ConDel:xxxxxx xxxx xxxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxxx

Shared LOS: \*

ApproachDel: xxxxxxx xxxxxxx xxxxxxx xxxxxxx 724.1

ApproachLOS: \* \* \* \* \* F

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - RANCHO PALOS VERDES
AM PEAK HOUR

Level Of Service Computation Report

ICU l(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #2 CREST DRIVE - COLLEGE ENTRANCE/PALOS VERDES DRIVE EAST

Cycle (sec): 100 Critical Vol./Cap.(X): 0.536

Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxxx

Optimal Cycle: 35 Level Of Service: A

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Permitted Permitted Permitted Permitted
Rights: Include Include Include Include
Lanes: 0 0 1 0 0 0 1 0 0 1 1 0 2 0 1 1 0 2 0 1

Volume Module:

Base Vol: 6 0 15 214 6 37 48 185 31 139 117 208
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 6 0 15 214 6 37 48 185 31 139 117 208
Added Vol: 14 0 40 0 0 0 0 0 41 122 0 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 20 0 55 214 6 37 48 185 72 261 117 208
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 22 0 60 233 7 40 52 201 78 284 127 226
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 22 0 60 233 7 40 52 201 78 284 127 226
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 22 0 60 233 7 40 52 201 78 284 127 226

Saturation Flow Module:

Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 0.27 0.00 0.73 0.97 0.03 1.00 1.00 2.00 1.00 1.00 2.00 1.00
Final Sat.: 427 0 1173 1556 44 1600 1600 3200 1600 1600 3200 1600

Capacity Analysis Module:

Vol/Sat: 0.01 0.00 0.05 0.15 0.15 0.03 0.03 0.06 0.05 0.18 0.04 0.14
Crit Moves: \*\*\*\* \*\*\*\* \*\*\*\* \*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - RANCHO PALOS VERDES
AM PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #3 PALOS VERDES DRIVE EAST/PALOS VERDES DRIVE SOUTH

Average Delay (sec/veh): 3.5 Worst Case Level Of Service: C[ 21.9]

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled
Rights: Include Include Include Include

Lanes: 0 0 0 0 0 1 0 0 0 1 1 0 1 0 0 0 0 1 0 1

Volume Module:

Table with 12 columns for traffic metrics: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume. Rows include data for each approach and movement.

Critical Gap Module:

Critical Gap: xxxxx xxxxx xxxxxx 6.4 xxxxx 6.2 4.1 xxxxx xxxxxx xxxxxx xxxxx xxxxxx
FollowUpTim: xxxxx xxxxx xxxxxx 3.5 xxxxx 3.3 2.2 xxxxx xxxxxx xxxxxx xxxxx xxxxxx

Capacity Module:

Cnflct Vol: xxxxx xxxxx xxxxxx 1350 xxxxx 507 598 xxxxx xxxxxx xxxxx xxxxx xxxxxx
Potent Cap.: xxxxx xxxxx xxxxxx 168 xxxxx 570 989 xxxxx xxxxxx xxxxx xxxxx xxxxxx
Move Cap.: xxxxx xxxxx xxxxxx 140 xxxxx 570 989 xxxxx xxxxxx xxxxx xxxxx xxxxxx
Volume/Cap: xxxxx xxxxx xxxxxx 0.30 xxxxx 0.15 0.22 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Level Of Service Module:

2Way95thQ: xxxxx xxxxx xxxxxx 1.2 xxxxx 0.5 0.8 xxxxx xxxxxx xxxxx xxxxx xxxxxx
Control Del: xxxxxx xxxxx xxxxxx 41.2 xxxxx 12.4 9.6 xxxxx xxxxxx xxxxxx xxxxx xxxxxx
LOS by Move: \* \* \* E \* B A \* \* \* \* \* \*
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx
SharedQueue: xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx
Shrd ConDel: xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx
Shared LOS: \*
ApproachDel: xxxxxxxx 21.9 xxxxxxxx xxxxxxxx
ApproachLOS: \* C \*

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - RANCHO PALOS VERDES
AM PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #5 MIRALESTE DRIVE/1ST STREET

Average Delay (sec/veh): 5.9 Worst Case Level Of Service: B[ 14.9]

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include

Lanes: 0 0 1 0 1 1 0 1 0 0 0 0 0 0 0 1 0 0 0 1

Volume Module:

Table with 12 columns for traffic metrics: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume. Rows include data for each approach and movement.

Critical Gap Module:

Critical Gap: xxxxx xxxxx xxxxxx 4.1 xxxxx xxxxxx xxxxxx xxxxx xxxxxx 6.4 xxxxx 6.2
FollowUpTim: xxxxxx xxxxx xxxxxx 2.2 xxxxx xxxxxx xxxxxx xxxxx xxxxxx 3.5 xxxxx 3.3

Capacity Module:

Cnflct Vol: xxxxx xxxxx xxxxxx 429 xxxxx xxxxxx xxxxx xxxxx xxxxxx 1265 xxxxx 317
Potent Cap.: xxxxx xxxxx xxxxxx 1141 xxxxx xxxxxx xxxxx xxxxx xxxxxx 189 xxxxx 728
Move Cap.: xxxxx xxxxx xxxxxx 1141 xxxxx xxxxxx xxxxx xxxxx xxxxxx 139 xxxxx 728
Volume/Cap: xxxxx xxxxx xxxxxx 0.34 xxxxx xxxxx xxxxx xxxxx xxxxxx 0.20 xxxxx 0.29

Level Of Service Module:

2Way95thQ: xxxxx xxxxx xxxxxx 1.5 xxxxx xxxxxx xxxxx xxxxx xxxxxx 0.7 xxxxx 1.2
Control Del: xxxxxx xxxxx xxxxxx 9.7 xxxxx xxxxxx xxxxxx xxxxx xxxxxx 37.3 xxxxx 12.0
LOS by Move: \* \* \* A \*
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx
SharedQueue: xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx
Shrd ConDel: xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx
Shared LOS: \*
ApproachDel: xxxxxxxx xxxxxxxx xxxxxxxx 14.9
ApproachLOS: \*

Note: Queue reported is the number of cars per lane.

HCS+: Unsignalized Intersections Release 5.2

TWO-WAY STOP CONTROL SUMMARY

Analyst: DK  
 Agency/Co.: RBF  
 Date Performed: 7/23/2007  
 Analysis Time Period: AM Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 Without Project  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: Miraleste Dr  
 Intersection Orientation: NS  
 Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	Northbound			Southbound		
	1 L	2 T	3 R	4 L	5 T	6 R
Volume	17	385	130	177	414	3
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	18	418	141	192	449	3
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L		TR
Upstream Signal?	No			No		

Minor Street: Approach Movement	Westbound			Eastbound		
	7 L	8 T	9 R	10 L	11 T	12 R
Volume	40	14	241	4	4	15
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	43	15	261	4	4	16
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage				/ No /		
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Delay, Queue Length, and Level of Service

Approach Movement Lane Config	NB	SB	Westbound		Eastbound		
	1 LT	4 L	7 LT	8 R	9 R	10 L	11 LTR
v (vph)	18	192	58		261		24
C(m) (vph)	1119	1022	246		639		219
v/c	0.02	0.19	0.24		0.41		0.11
95% queue length	0.05	0.69	0.89		1.99		0.36
Control Delay	8.3	9.3	24.1		14.5		23.5
LOS	A	A	C		B		C
Approach Delay				16.2			23.5
Approach LOS				C			C

HCS+: Unsignalized Intersections Release 5.2

Phone:  
 Fax:  
 E-Mail:

TWO-WAY STOP CONTROL(TWSC) ANALYSIS

Analyst: DK  
 Agency/Co.: RBF  
 Date Performed: 7/23/2007  
 Analysis Time Period: AM Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 Without Project  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: Miraleste Dr  
 Intersection Orientation: NS  
 Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street Movements	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	17	385	130	177	414	3
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	5	105	35	48	112	1
Hourly Flow Rate, HFR	18	418	141	192	449	3
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L		TR
Upstream Signal?	No			No		

Minor Street Movements	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	40	14	241	4	4	15
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	11	4	65	1	1	4
Hourly Flow Rate, HFR	43	15	261	4	4	16
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage				/ No /		
RT Channelized?	No					
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Pedestrian Volumes and Adjustments

Movements	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)	12.0	12.0	12.0	12.0
Walking Speed (ft/sec)	4.0	4.0	4.0	4.0
Percent Blockage	0	0	0	0

Upstream Signal Data							
	Prog. Flow vph	Sat Flow vph	Arrival Type	Green Time sec	Cycle Length sec	Prog. Speed mph	Distance to Signal feet
S2	Left-Turn						
	Through						
S5	Left-Turn						
	Through						

Worksheet 3-Data for Computing Effect of Delay to Major Street Vehicles

	Movement 2	Movement 5
Shared ln volume, major th vehicles:	418	
Shared ln volume, major rt vehicles:	0	
Sat flow rate, major th vehicles:	1700	
Sat flow rate, major rt vehicles:	1700	
Number of major street through lanes:	1	

Worksheet 4-Critical Gap and Follow-up Time Calculation

Critical Gap Calculation								
Movement	1	4	7	8	9	10	11	12
	L	L	L	T	R	L	T	R
t(c,base)	4.1	4.1	7.1	6.5	6.2	7.1	6.5	6.2
t(c,hv)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
P(hv)	0	0	0	0	0	0	0	0
t(c,g)			0.20	0.20	0.10	0.20	0.20	0.10
Grade/100			0.00	0.00	0.00	0.00	0.00	0.00
t(3,lt)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
t(c,T):	1-stage 0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2-stage 0.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00
t(c)	1-stage 4.1	4.1	7.1	6.5	6.2	7.1	6.5	6.2
	2-stage 4.1	4.1	6.1	5.5	6.2	6.1	5.5	6.2

Follow-Up Time Calculations								
Movement	1	4	7	8	9	10	11	12
	L	L	L	T	R	L	T	R
t(f,base)	2.20	2.20	3.50	4.00	3.30	3.50	4.00	3.30
t(f,HV)	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
P(HV)	0	0	0	0	0	0	0	0
t(f)	2.2	2.2	3.5	4.0	3.3	3.5	4.0	3.3

Worksheet 5-Effect of Upstream Signals

Computation 1-Queue Clearance Time at Upstream Signal				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
V prog				
Total Saturation Flow Rate, s (vph)				
Arrival Type				
Effective Green, g (sec)				
Cycle Length, C (sec)				
Rp (from Exhibit 16-11)				
Proportion vehicles arriving on green P				
g(q1)				
g(q2)				
g(q)				

Computation 2-Proportion of TWSC Intersection Time blocked				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
alpha				
beta				
Travel time, t(a) (sec)				
Smoothing Factor, F				
Proportion of conflicting flow, f				
Max platooned flow, V(c,max)				
Min platooned flow, V(c,min)				
Duration of blocked period, t(p)				
Proportion time blocked, p		0.000		0.000

Computation 3-Platoon Event Periods	Result
p(2)	0.000
p(5)	0.000
p(dom)	
p(subo)	
Constrained or unconstrained?	

Proportion unblocked for minor movements, p(x)	(1)	(2)	(3)
	Single-stage Process	Two-Stage Stage I	Process Stage II
p(1)			
p(4)			
p(7)			
p(8)			
p(9)			
p(10)			
p(11)			
p(12)			

Computation 4 and 5 Single-Stage Process								
Movement	1	4	7	8	9	10	11	12
	L	L	L	T	R	L	T	R
V c,x	452	559	1298	1290	418	1496	1429	450
s								
Px								
V c,u,x								
C r,x								
C plat,x								

	7		8		10		11	
	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2
V(c,x)	454	844	454	836	834	662	834	595
s		1500		1500		1500		1500
P(x)								
V(c,u,x)								
C(r,x)								
C(plat,x)								

Worksheet 6-Impedance and Capacity Equations

Step 1: RT from Minor St.	9	12
Conflicting Flows	418	450
Potential Capacity	639	613
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	639	613
Probability of Queue free St.	0.59	0.97
Step 2: LT from Major St.	4	1
Conflicting Flows	559	452
Potential Capacity	1022	1119
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	1022	1119
Probability of Queue free St.	0.81	0.98
Maj L-Shared Prob Q free St.		0.98
Step 3: TH from Minor St.	8	11
Conflicting Flows	1290	1429
Potential Capacity	165	136
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.79	0.79
Movement Capacity	131	108
Probability of Queue free St.	0.96	0.98
Step 4: LT from Minor St.	7	10
Conflicting Flows	1298	1496
Potential Capacity	140	102
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.78	0.76
Maj. L, Min T Adj. Imp Factor.	0.83	0.82
Cap. Adj. factor due to Impeding mvmnt	0.81	0.48
Movement Capacity	114	49

Worksheet 7-Computation of the Effect of Two-stage Gap Acceptance

Step 3: TH from Minor St.	8	11
Part 1 - First Stage		
Conflicting Flows	454	834
Potential Capacity	573	386
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.98	0.81
Movement Capacity	561	313
Probability of Queue free St.	0.97	0.99
Part 2 - Second Stage		
Conflicting Flows	836	595
Potential Capacity	385	496
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.81	0.98
Movement Capacity	313	485
Part 3 - Single Stage		
Conflicting Flows	1290	1429
Potential Capacity	165	136
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.79	0.79
Movement Capacity	131	108
Result for 2 stage process:		
a	0.95	0.95

Y	2.62	1.11
C t	265	225
Probability of Queue free St.	0.96	0.98
Step 4: LT from Minor St.	7	10
Part 1 - First Stage		
Conflicting Flows	454	834
Potential Capacity	589	365
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.98	0.81
Movement Capacity	576	296
Part 2 - Second Stage		
Conflicting Flows	844	662
Potential Capacity	361	454
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.78	0.56
Movement Capacity	282	256
Part 3 - Single Stage		
Conflicting Flows	1298	1496
Potential Capacity	140	102
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.78	0.76
Maj. L, Min T Adj. Imp Factor.	0.83	0.82
Cap. Adj. factor due to Impeding mvmnt	0.81	0.48
Movement Capacity	114	49
Results for Two-stage process:		
a	0.95	0.95
Y	3.08	16.47
C t	240	61

Worksheet 8-Shared Lane Calculations

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (vph)	43	15	261	4	4	16
Movement Capacity (vph)	240	265	639	61	225	613
Shared Lane Capacity (vph)	246				219	

Worksheet 9-Computation of Effect of Flared Minor Street Approaches

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
C sep	240	265	639	61	225	613
Volume	43	15	261	4	4	16
Delay						
Q sep						
Q sep +1						
round (Qsep +1)						
n max						
C sh	246				219	
SUM C sep						
n						
C act						

Worksheet 10-Delay, Queue Length, and Level of Service

Movement	1	4	7	8	9	10	11	12
Lane Config	LT	L	LT		R		LTR	
v (vph)	18	192	58		261		24	
C(m) (vph)	1119	1022	246		639		219	
v/c	0.02	0.19	0.24		0.41		0.11	
95% queue length	0.05	0.69	0.89		1.99		0.36	
Control Delay	8.3	9.3	24.1		14.5		23.5	
LOS	A	A	C		B		C	
Approach Delay				16.2			23.5	
Approach LOS				C			C	

Worksheet 11-Shared Major LT Impedance and Delay

	Movement 2	Movement 5
p(oj)	0.98	0.81
v(i1), Volume for stream 2 or 5	418	
v(i2), Volume for stream 3 or 6	0	
s(i1), Saturation flow rate for stream 2 or 5	1700	
s(i2), Saturation flow rate for stream 3 or 6	1700	
P*(oj)	0.98	
d(M,LT), Delay for stream 1 or 4	8.3	9.3
N, Number of major street through lanes	1	
d(rank,1) Delay for stream 2 or 5	0.2	

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - RANCHO PALOS VERDES
AM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #6 WESTERN AVENUE/TOSCANINI DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 0.819
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 71 Level Of Service: D

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 sub-columns for movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns for traffic volume metrics: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume.

Saturation Flow Module:

Table with 12 columns for saturation flow metrics: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with 12 columns for capacity analysis metrics: Vol/Sat, Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - RANCHO PALOS VERDES
AM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #7 WESTERN AVENUE/CAPITOL DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 0.933
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 119 Level Of Service: E

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 sub-columns for movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns for traffic volume metrics: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume.

Saturation Flow Module:

Table with 12 columns for saturation flow metrics: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with 12 columns for capacity analysis metrics: Vol/Sat, Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - RANCHO PALOS VERDES
AM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #8 WESTERN AVENUE/CRESTWOOD STREET
\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.862
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 84 Level Of Service: D

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 sub-columns for movements (L-T-R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module table with 11 columns for traffic volumes and 11 rows for various volume metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module table with 11 columns for saturation flow and 4 rows for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module table with 11 columns for capacity and 4 rows for Vol/Sat, Crit Moves, and other metrics.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - RANCHO PALOS VERDES
AM PEAK HOUR

Level Of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #9 WESTERN AVENUE/1ST STREET
\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 1.367
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 sub-columns for movements (L-T-R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module table with 11 columns for traffic volumes and 11 rows for various volume metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module table with 11 columns for saturation flow and 4 rows for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module table with 11 columns for capacity and 4 rows for Vol/Sat, Crit Moves, and other metrics.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - RANCHO PALOS VERDES
AM PEAK HOUR

Level Of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

Intersection #10 WESTERN AVENUE/9TH STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.609
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 51 Level Of Service: B

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module:

Table with 12 columns for traffic volumes and 12 rows for various volume metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module:

Table with 12 columns for saturation flow and 4 rows for Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with 12 columns for capacity analysis and 4 rows for Vol/Sat, Crit Volume, Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - RANCHO PALOS VERDES
AM PEAK HOUR

Level Of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

Intersection #11 WESTERN AVENUE/WEST 25TH STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.703
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 67 Level Of Service: C

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module:

Table with 12 columns for traffic volumes and 12 rows for various volume metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module:

Table with 12 columns for saturation flow and 4 rows for Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with 12 columns for capacity analysis and 4 rows for Vol/Sat, Crit Volume, Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - RANCHO PALOS VERDES
PM PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #1 PALOS VERDES DRIVE EAST/MIRALESTE DRIVE
\*\*\*\*\*

Average Delay (sec/veh): 269.0 Worst Case Level Of Service: F[643.0]

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 0 0 0 1 0 1 0 1 0 0 0 0 0 0 0 1

Volume Module:

Base Vol: 0 138 157 419 200 0 0 0 0 293 0 351
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 0 138 157 419 200 0 0 0 0 293 0 351
Added Vol: 0 10 63 0 9 0 0 0 0 61 0 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 0 148 220 419 209 0 0 0 0 354 0 351
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 0 161 239 455 227 0 0 0 0 385 0 382
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 0 161 239 455 227 0 0 0 0 385 0 382

Critical Gap Module:

Critical Gap:xxxxx xxxxx xxxxx 4.1 xxxxx xxxxx xxxxx xxxxx xxxxx 6.4 xxxxx 6.2
FollowUpTim:xxxxx xxxxx xxxxx 2.2 xxxxx xxxxx xxxxx xxxxx xxxxx 3.5 xxxxx 3.3

Capacity Module:

Cnflct Vol: xxxxx xxxxx xxxxx 400 xxxxx xxxxx xxxxx xxxxx xxxxx 1418 xxxxx 280
Potent Cap.: xxxxx xxxxx xxxxx 1170 xxxxx xxxxx xxxxx xxxxx xxxxx 152 xxxxx 763
Move Cap.: xxxxx xxxxx xxxxx 1170 xxxxx xxxxx xxxxx xxxxx xxxxx 106 xxxxx 763
Volume/Cap: xxxxx xxxxx xxxxx 0.39 xxxxx xxxxx xxxxx xxxxx xxxxx 3.63 xxxxx 0.50

Level Of Service Module:

2Way95thQ: xxxxx xxxxx xxxxx 1.9 xxxxx xxxxx xxxxx xxxxx xxxxx 38.6 xxxxx 2.8
Control Del:xxxxx xxxxx xxxxx 10.0 xxxxx xxxxx xxxxx xxxxx xxxxx 1266 xxxxx 14.3
LOS by Move: \* \* \* B \* \* \* \* \* F \* \* B
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
SharedQueue:xxxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Shrd ConDel:xxxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx
Shared LOS: \*
ApproachDel: xxxxxxx xxxxxxx xxxxxxx xxxxxxx 643.0
ApproachLOS: \* \* \* \* \* F

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - RANCHO PALOS VERDES
PM PEAK HOUR

Level Of Service Computation Report

ICU l(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #2 CREST DRIVE - COLLEGE ENTRANCE/PALOS VERDES DRIVE EAST
\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.407
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxxx
Optimal Cycle: 29 Level Of Service: A

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Permitted Permitted Permitted Permitted
Rights: Include Include Include Include
Lanes: 0 0 1! 0 0 0 1 0 0 1 1 0 2 0 1 1 0 2 0 1

Volume Module:

Base Vol: 33 2 97 71 1 24 22 103 10 30 187 103
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 33 2 97 71 1 24 22 103 10 30 187 103
Added Vol: 24 0 73 0 0 0 0 0 24 70 0 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 57 2 170 71 1 24 22 103 34 100 187 103
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 62 2 185 77 1 26 24 112 37 109 203 112
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 62 2 185 77 1 26 24 112 37 109 203 112
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 62 2 185 77 1 26 24 112 37 109 203 112

Saturation Flow Module:

Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 0.25 0.01 0.74 0.99 0.01 1.00 1.00 2.00 1.00 1.00 2.00 1.00
Final Sat.: 398 14 1188 1578 22 1600 1600 3200 1600 1600 3200 1600

Capacity Analysis Module:

Vol/Sat: 0.04 0.16 0.16 0.05 0.05 0.02 0.01 0.03 0.02 0.07 0.06 0.07
Crit Moves: \*\*\*\* \*\*\*\* \*\*\*\* \*\*\*\*

\*\*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - RANCHO PALOS VERDES
PM PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #3 PALOS VERDES DRIVE EAST/PALOS VERDES DRIVE SOUTH

Average Delay (sec/veh): 3.6 Worst Case Level Of Service: C[ 21.2]

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled
Rights: Include Include Include Include

Lanes: 0 0 0 0 0 1 0 0 0 1 1 0 1 0 0 0 0 1 0 1

Volume Module:

Table with 12 columns for traffic metrics: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume. Rows include North and South bound movements.

Critical Gap Module:

Critical Gap: xxxxx xxxxx xxxxxx 6.4 xxxxx 6.2 4.1 xxxxx xxxxxx xxxxxx xxxxx xxxxxx
FollowUpTim: xxxxx xxxxx xxxxxx 3.5 xxxxx 3.3 2.2 xxxxx xxxxxx xxxxxx xxxxx xxxxxx

Capacity Module:

Cnflct Vol: xxxxx xxxxx xxxxxx 1274 xxxxx 425 499 xxxxx xxxxxx xxxxx xxxxx xxxxxx
Potent Cap.: xxxxx xxxxx xxxxxx 186 xxxxx 634 1076 xxxxx xxxxxx xxxxx xxxxx xxxxxx
Move Cap.: xxxxx xxxxx xxxxxx 170 xxxxx 634 1076 xxxxx xxxxxx xxxxx xxxxx xxxxxx
Volume/Cap: xxxxx xxxxx xxxxxx 0.38 xxxxx 0.19 0.12 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Level Of Service Module:

2Way95thQ: xxxxx xxxxx xxxxxx 1.6 xxxxx 0.7 0.4 xxxxx xxxxxx xxxxx xxxxx xxxxxx
Control Del: xxxxxx xxxxx xxxxxx 38.6 xxxxx 12.0 8.8 xxxxx xxxxxx xxxxxx xxxxx xxxxxx
LOS by Move: \* \* \* E \* B A \* \* \* \* \* \*
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx
SharedQueue: xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx
Shrd ConDel: xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx
Shared LOS: \* \* \* \* \* \* \* \* \* \* \* \* \* \* \* \*
ApproachDel: xxxxxx 21.2 xxxxxx xxxxxx
ApproachLOS: \* C \* \* \* \* \* \* \* \* \* \* \* \*

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - RANCHO PALOS VERDES
PM PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #5 MIRALESTE DRIVE/1ST STREET

Average Delay (sec/veh): 4.8 Worst Case Level Of Service: C[ 15.1]

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include

Lanes: 0 0 1 0 1 1 0 1 0 0 0 0 0 0 0 1 0 0 0 1

Volume Module:

Table with 12 columns for traffic metrics: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume. Rows include North and South bound movements.

Critical Gap Module:

Critical Gap: xxxxx xxxxx xxxxxx 4.1 xxxxx xxxxxx xxxxxx xxxxx xxxxxx 6.4 xxxxx 6.2
FollowUpTim: xxxxx xxxxx xxxxxx 2.2 xxxxx xxxxxx xxxxxx xxxxx xxxxxx 3.5 xxxxx 3.3

Capacity Module:

Cnflct Vol: xxxxx xxxxx xxxxxx 326 xxxxx xxxxxx xxxxx xxxxx xxxxxx 1162 xxxxx 278
Potent Cap.: xxxxx xxxxx xxxxxx 1245 xxxxx xxxxxx xxxxx xxxxx xxxxxx 218 xxxxx 765
Move Cap.: xxxxx xxxxx xxxxxx 1245 xxxxx xxxxxx xxxxx xxxxx xxxxxx 186 xxxxx 765
Volume/Cap: xxxxx xxxxx xxxxxx 0.19 xxxxx xxxxx xxxxx xxxxx xxxxxx 0.26 xxxxx 0.27

Level Of Service Module:

2Way95thQ: xxxxx xxxxx xxxxxx 0.7 xxxxx xxxxxx xxxxx xxxxx xxxxxx 1.0 xxxxx 1.1
Control Del: xxxxxx xxxxx xxxxxx 8.6 xxxxx xxxxxx xxxxxx xxxxx xxxxxx 31.0 xxxxx 11.5
LOS by Move: \* \* \* A \* \* \* \* \* \* \* D \* \* B
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.: xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx
SharedQueue: xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx
Shrd ConDel: xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxxx
Shared LOS: \* \* \* \* \* \* \* \* \* \* \* \* \* \* \* \*
ApproachDel: xxxxxx xxxxxx xxxxxx 15.1
ApproachLOS: \* \* \* \* \* \* \* \* \* \* \* \*

Note: Queue reported is the number of cars per lane.

HCS+: Unsignalized Intersections Release 5.2

TWO-WAY STOP CONTROL SUMMARY

Analyst: DK  
 Agency/Co.: RBF  
 Date Performed: 7/23/2007  
 Analysis Time Period: PM Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 Without Project  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: Miraleste Dr  
 Intersection Orientation: NS  
 Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	Northbound			Southbound		
	1 L	2 T	3 R	4 L	5 T	6 R
Volume	7	402	69	166	477	2
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	7	436	74	180	518	2
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L	TR	
Upstream Signal?	No			No		

Minor Street: Approach Movement	Westbound			Eastbound		
	7 L	8 T	9 R	10 L	11 T	12 R
Volume	47	6	221	2	5	6
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	51	6	240	2	5	6
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage				/ No /		
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Delay, Queue Length, and Level of Service

Approach Movement Lane Config	NB	SB	Westbound		Eastbound	
	1 LT	4 L	7 LT	8 R	9 R	10 LTR
v (vph)	7	180	57		240	13
C(m) (vph)	1056	1065	247		625	258
v/c	0.01	0.17	0.23		0.38	0.05
95% queue length	0.02	0.61	0.87		1.80	0.16
Control Delay	8.4	9.1	23.9		14.3	19.7
LOS	A	A	C		B	C
Approach Delay				16.1	19.7	
Approach LOS				C	C	

HCS+: Unsignalized Intersections Release 5.2

Phone:  
 Fax:  
 E-Mail:

TWO-WAY STOP CONTROL(TWSC) ANALYSIS

Analyst: DK  
 Agency/Co.: RBF  
 Date Performed: 7/23/2007  
 Analysis Time Period: PM Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 Without Project  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: Miraleste Dr  
 Intersection Orientation: NS  
 Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street Movements	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	7	402	69	166	477	2
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	2	109	19	45	130	1
Hourly Flow Rate, HFR	7	436	74	180	518	2
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L	TR	
Upstream Signal?	No			No		

Minor Street Movements	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	47	6	221	2	5	6
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	13	2	60	1	1	2
Hourly Flow Rate, HFR	51	6	240	2	5	6
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage				/ No /		
RT Channelized?	No					
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Pedestrian Volumes and Adjustments

Movements	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)	12.0	12.0	12.0	12.0
Walking Speed (ft/sec)	4.0	4.0	4.0	4.0
Percent Blockage	0	0	0	0

Upstream Signal Data							
	Prog. Flow vph	Sat Flow vph	Arrival Type	Green Time sec	Cycle Length sec	Prog. Speed mph	Distance to Signal feet
S2	Left-Turn						
	Through						
S5	Left-Turn						
	Through						

Worksheet 3-Data for Computing Effect of Delay to Major Street Vehicles

	Movement 2	Movement 5
Shared ln volume, major th vehicles:	436	
Shared ln volume, major rt vehicles:	0	
Sat flow rate, major th vehicles:	1700	
Sat flow rate, major rt vehicles:	1700	
Number of major street through lanes:	1	

Worksheet 4-Critical Gap and Follow-up Time Calculation

Critical Gap Calculation									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
t(c,base)	4.1	4.1	7.1	6.5	6.2	7.1	6.5	6.2	
t(c,hv)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
P(hv)	0	0	0	0	0	0	0	0	
t(c,g)			0.20	0.20	0.10	0.20	0.20	0.10	
Grade/100			0.00	0.00	0.00	0.00	0.00	0.00	
t(3,lt)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
t(c,T):	1-stage	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
	2-stage	0.00	0.00	1.00	1.00	0.00	1.00	1.00	
t(c)	1-stage	4.1	4.1	7.1	6.5	7.1	6.5	6.2	
	2-stage	4.1	4.1	6.1	5.5	6.2	6.1	5.5	

Follow-Up Time Calculations									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
t(f,base)	2.20	2.20	3.50	4.00	3.30	3.50	4.00	3.30	
t(f,HV)	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	
P(HV)	0	0	0	0	0	0	0	0	
t(f)	2.2	2.2	3.5	4.0	3.3	3.5	4.0	3.3	

Worksheet 5-Effect of Upstream Signals

Computation 1-Queue Clearance Time at Upstream Signal				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
V prog				
Total Saturation Flow Rate, s (vph)				
Arrival Type				
Effective Green, g (sec)				
Cycle Length, C (sec)				
Rp (from Exhibit 16-11)				
Proportion vehicles arriving on green P				
g(q1)				
g(q2)				
g(q)				

Computation 2-Proportion of TWSC Intersection Time blocked				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
alpha				
beta				
Travel time, t(a) (sec)				
Smoothing Factor, F				
Proportion of conflicting flow, f				
Max platooned flow, V(c,max)				
Min platooned flow, V(c,min)				
Duration of blocked period, t(p)				
Proportion time blocked, p		0.000		0.000

Computation 3-Platoon Event Periods		Result
p(2)		0.000
p(5)		0.000
p(dom)		
p(subo)		
Constrained or unconstrained?		

Proportion unblocked for minor movements, p(x)	(1)	(2)	(3)
	Single-stage Process	Two-Stage Stage I	Process Stage II
p(1)			
p(4)			
p(7)			
p(8)			
p(9)			
p(10)			
p(11)			
p(12)			

Computation 4 and 5 Single-Stage Process									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
V c,x	520	510	1334	1330	436	1489	1403	519	
s									
Px									
V c,u,x									
C r,x									
C plat,x									

	7		8		10		11	
	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2
V(c,x)	450	884	450	880	879	610	879	524
s		1500		1500		1500		1500
P(x)								
V(c,u,x)								
C(r,x)								
C(plat,x)								

Worksheet 6-Impedance and Capacity Equations

Step 1: RT from Minor St.	9	12
Conflicting Flows	436	519
Potential Capacity	625	561
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	625	561
Probability of Queue free St.	0.62	0.99
Step 2: LT from Major St.	4	1
Conflicting Flows	510	520
Potential Capacity	1065	1056
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	1065	1056
Probability of Queue free St.	0.83	0.99
Maj L-Shared Prob Q free St.		0.99
Step 3: TH from Minor St.	8	11
Conflicting Flows	1330	1403
Potential Capacity	156	141
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.82	0.82
Movement Capacity	128	116
Probability of Queue free St.	0.98	0.98
Step 4: LT from Minor St.	7	10
Conflicting Flows	1334	1489
Potential Capacity	132	103
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.81	0.81
Maj. L, Min T Adj. Imp Factor.	0.85	0.85
Cap. Adj. factor due to Impeding mvmnt	0.84	0.53
Movement Capacity	111	54

Worksheet 7-Computation of the Effect of Two-stage Gap Acceptance

Step 3: TH from Minor St.	8	11
Part 1 - First Stage		
Conflicting Flows	450	879
Potential Capacity	575	368
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.99	0.83
Movement Capacity	570	306
Probability of Queue free St.	0.99	0.98
Part 2 - Second Stage		
Conflicting Flows	880	524
Potential Capacity	368	533
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.83	0.99
Movement Capacity	306	528
Part 3 - Single Stage		
Conflicting Flows	1330	1403
Potential Capacity	156	141
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.82	0.82
Movement Capacity	128	116
Result for 2 stage process:		
a	0.95	0.95

Y	2.58	0.82
C t	268	242
Probability of Queue free St.	0.98	0.98
Step 4: LT from Minor St.	7	10
Part 1 - First Stage		
Conflicting Flows	450	879
Potential Capacity	592	345
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.99	0.83
Movement Capacity	587	287
Part 2 - Second Stage		
Conflicting Flows	884	610
Potential Capacity	343	485
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.81	0.60
Movement Capacity	277	293
Part 3 - Single Stage		
Conflicting Flows	1334	1489
Potential Capacity	132	103
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.81	0.81
Maj. L, Min T Adj. Imp Factor.	0.85	0.85
Cap. Adj. factor due to Impeding mvmnt	0.84	0.53
Movement Capacity	111	54
Results for Two-stage process:		
a	0.95	0.95
Y	2.99	3.95
C t	245	105

Worksheet 8-Shared Lane Calculations

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (vph)	51	6	240	2	5	6
Movement Capacity (vph)	245	268	625	105	242	561
Shared Lane Capacity (vph)	247				258	

Worksheet 9-Computation of Effect of Flared Minor Street Approaches

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
C sep	245	268	625	105	242	561
Volume	51	6	240	2	5	6
Delay						
Q sep						
Q sep +1						
round (Qsep +1)						
n max						
C sh	247				258	
SUM C sep						
n						
C act						

Worksheet 10-Delay, Queue Length, and Level of Service

Movement	1	4	7	8	9	10	11	12
Lane Config	LT	L	LT		R		LTR	
v (vph)	7	180	57		240		13	
C(m) (vph)	1056	1065	247		625		258	
v/c	0.01	0.17	0.23		0.38		0.05	
95% queue length	0.02	0.61	0.87		1.80		0.16	
Control Delay	8.4	9.1	23.9		14.3		19.7	
LOS	A	A	C		B		C	
Approach Delay				16.1			19.7	
Approach LOS				C			C	

Worksheet 11-Shared Major LT Impedance and Delay

	Movement 2	Movement 5
p(oj)	0.99	0.83
v(i1), Volume for stream 2 or 5	436	
v(i2), Volume for stream 3 or 6	0	
s(i1), Saturation flow rate for stream 2 or 5	1700	
s(i2), Saturation flow rate for stream 3 or 6	1700	
P*(oj)	0.99	
d(M,LT), Delay for stream 1 or 4	8.4	9.1
N, Number of major street through lanes	1	
d(rank,1) Delay for stream 2 or 5	0.1	

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - RANCHO PALOS VERDES
PM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #6 WESTERN AVENUE/TOSCANINI DRIVE
\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.712
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 52 Level Of Service: C

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 sub-columns for movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module table with 11 columns for traffic volume and 11 rows for various metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module table with 11 columns for saturation flow and 4 rows for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module table with 11 columns for capacity and 3 rows for Vol/Sat, Crit Moves, and a summary row.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - RANCHO PALOS VERDES
PM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #7 WESTERN AVENUE/CAPITOL DRIVE
\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.832
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 75 Level Of Service: D

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 sub-columns for movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module table with 11 columns for traffic volume and 11 rows for various metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module table with 11 columns for saturation flow and 4 rows for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module table with 11 columns for capacity and 3 rows for Vol/Sat, Crit Moves, and a summary row.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - RANCHO PALOS VERDES
PM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #8 WESTERN AVENUE/CRESTWOOD STREET
\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.816
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 70 Level Of Service: D

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module: Table with 11 columns (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume) and 11 rows.

Saturation Flow Module: Table with 11 columns (Sat/Lane, Adjustment, Lanes, Final Sat) and 4 rows.

Capacity Analysis Module: Table with 11 columns (Vol/Sat, Crit Moves) and 4 rows.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - RANCHO PALOS VERDES
PM PEAK HOUR

Level Of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #9 WESTERN AVENUE/1ST STREET
\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 1.321
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module: Table with 11 columns (Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume) and 11 rows.

Saturation Flow Module: Table with 11 columns (Sat/Lane, Adjustment, Lanes, Final Sat) and 4 rows.

Capacity Analysis Module: Table with 11 columns (Vol/Sat, Crit Moves) and 4 rows.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - RANCHO PALOS VERDES
PM PEAK HOUR

Level Of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

Intersection #10 WESTERN AVENUE/9TH STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.804
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 102 Level Of Service: D

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module:

Table with 11 columns for traffic volumes and 11 rows for various volume metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module:

Table with 11 columns for saturation flow and 4 rows for Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with 11 columns for capacity analysis and 4 rows for Vol/Sat, Crit Volume, Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - RANCHO PALOS VERDES
PM PEAK HOUR

Level Of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

Intersection #11 WESTERN AVENUE/WEST 25TH STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.635
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 55 Level Of Service: B

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module:

Table with 11 columns for traffic volumes and 11 rows for various volume metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module:

Table with 11 columns for saturation flow and 4 rows for Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with 11 columns for capacity analysis and 4 rows for Vol/Sat, Crit Volume, Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - RANCHO PALOS VERDES
WEEKDAY (11:00-1:00) PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #1 PALOS VERDES DRIVE EAST/MIRALESTE DRIVE
\*\*\*\*\*

Average Delay (sec/veh): 132.7 Worst Case Level Of Service: F[345.5]

Table with columns: Approach, Movement, Control, Rights, Lanes. Rows for North, South, East, West Bound movements.

Volume Module:

Table with columns: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume.

Critical Gap Module:

Table with columns: Critical Gap, FollowUpTim.

Capacity Module:

Table with columns: Cnflct Vol, Potent Cap., Move Cap., Volume/Cap.

Level Of Service Module:

Table with columns: 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - RANCHO PALOS VERDES
WEEKDAY (11:00-1:00) PEAK HOUR

Level Of Service Computation Report

ICU l(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #2 CREST DRIVE - COLLEGE ENTRANCE/PALOS VERDES DRIVE EAST
\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.443

Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxxx

Optimal Cycle: 30 Level Of Service: A

Table with columns: Approach, Movement, Control, Rights, Lanes. Rows for North, South, East, West Bound movements.

Volume Module:

Table with columns: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume.

Saturation Flow Module:

Table with columns: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with columns: Vol/Sat, Crit Moves.

\*\*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - RANCHO PALOS VERDES
WEEKDAY (11:00-1:00) PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

\*\*\*\*\*

Intersection #3 PALOS VERDES DRIVE EAST/PALOS VERDES DRIVE SOUTH

\*\*\*\*\*

Average Delay (sec/veh): 2.9 Worst Case Level Of Service: B[ 14.6]

\*\*\*\*\*

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled

Rights: Include Include Include Include

Lanes: 0 0 0 0 0 1 0 0 0 1 1 0 1 0 0 0 0 1 0 1

-----|-----|-----|-----|

Volume Module:

Table with 16 columns and 11 rows showing traffic volume data: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume.

-----|-----|-----|-----|

Critical Gap Module:

Critical Gp:xxxxx xxxxx xxxxx 6.4 xxxxx 6.2 4.1 xxxxx xxxxxx xxxxx xxxxx xxxxxx

FollowUpTim:xxxxx xxxxx xxxxx 3.5 xxxxx 3.3 2.2 xxxxx xxxxxx xxxxx xxxxx xxxxxx

-----|-----|-----|-----|

Capacity Module:

Cnflct Vol: xxxxx xxxxx xxxxxx 832 xxxxx 342 393 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Potent Cap.: xxxxx xxxxx xxxxxx 342 xxxxx 705 1176 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Move Cap.: xxxxx xxxxx xxxxxx 328 xxxxx 705 1176 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Volume/Cap: xxxxx xxxxx xxxxx 0.22 xxxxx 0.12 0.05 xxxxx xxxxx xxxxx xxxxx xxxxxx

-----|-----|-----|-----|

Level Of Service Module:

2Way95thQ: xxxxx xxxxx xxxxxx 0.8 xxxxx 0.4 0.2 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Control Del:xxxxxx xxxxx xxxxxx 19.1 xxxxx 10.8 8.2 xxxxx xxxxxx xxxxxx xxxxx xxxxxx

LOS by Move: \* \* \* C \* B A \* \* \* \* \*

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx

SharedQueue:xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx

Shrd ConDel:xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx

Shared LOS: \* \* \* \* \* \* \* \* \* \* \* \* \* \*

ApproachDel: xxxxxxx 14.6 xxxxxxx xxxxxxx

ApproachLOS: \* B \* \*

\*\*\*\*\*

Note: Queue reported is the number of cars per lane.

\*\*\*\*\*

HCS+: Unsignalized Intersections Release 5.2

TWO-WAY STOP CONTROL SUMMARY

Analyst: DK  
 Agency/Co.: RBF  
 Date Performed: 7/23/2007  
 Analysis Time Period: Weekday Mid-day Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 Without Project  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: Miraleste Dr  
 Intersection Orientation: NS  
 Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	Northbound			Southbound		
	1 L	2 T	3 R	4 L	5 T	6 R
Volume	6	274	53	184	330	7
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	6	297	57	199	358	7
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L	TR	
Upstream Signal?	No			No		

Minor Street: Approach Movement	Westbound			Eastbound		
	7 L	8 T	9 R	10 L	11 T	12 R
Volume	36	18	223	4	3	6
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	39	19	242	4	3	6
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage	/			No /		
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Delay, Queue Length, and Level of Service

Approach Movement Lane Config	NB	SB	Westbound		Eastbound		
	1	4	7	8	9	10	11 12
	LT	L	LT		R		LTR
v (vph)	6	199	58		242		13
C(m) (vph)	1205	1216	302		747		298
v/c	0.00	0.16	0.19		0.32		0.04
95% queue length	0.02	0.58	0.70		1.41		0.14
Control Delay	8.0	8.5	19.7		12.1		17.6
LOS	A	A	C		B		C
Approach Delay				13.6			17.6
Approach LOS				B			C

HCS+: Unsignalized Intersections Release 5.2

Phone:  
 E-Mail:

Fax:

TWO-WAY STOP CONTROL(TWSC) ANALYSIS

Analyst: DK  
 Agency/Co.: RBF  
 Date Performed: 7/23/2007  
 Analysis Time Period: Weekday Mid-day Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 Without Project  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: Miraleste Dr  
 Intersection Orientation: NS  
 Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street Movements	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	6	274	53	184	330	7
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	2	74	14	50	90	2
Hourly Flow Rate, HFR	6	297	57	199	358	7
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L	TR	
Upstream Signal?	No			No		

Minor Street Movements	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	36	18	223	4	3	6
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	10	5	61	1	1	2
Hourly Flow Rate, HFR	39	19	242	4	3	6
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage	/			No /		
RT Channelized?	No					
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Pedestrian Volumes and Adjustments

Movements	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)	12.0	12.0	12.0	12.0
Walking Speed (ft/sec)	4.0	4.0	4.0	4.0
Percent Blockage	0	0	0	0

Upstream Signal Data							
	Prog. Flow vph	Sat Flow vph	Arrival Type	Green Time sec	Cycle Length sec	Prog. Speed mph	Distance to Signal feet
S2	Left-Turn						
	Through						
S5	Left-Turn						
	Through						

Worksheet 3-Data for Computing Effect of Delay to Major Street Vehicles

	Movement 2	Movement 5
Shared ln volume, major th vehicles:	297	
Shared ln volume, major rt vehicles:	0	
Sat flow rate, major th vehicles:	1700	
Sat flow rate, major rt vehicles:	1700	
Number of major street through lanes:	1	

Worksheet 4-Critical Gap and Follow-up Time Calculation

Critical Gap Calculation									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
t(c,base)	4.1	4.1	7.1	6.5	6.2	7.1	6.5	6.2	
t(c,hv)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
P(hv)	0	0	0	0	0	0	0	0	
t(c,g)			0.20	0.20	0.10	0.20	0.20	0.10	
Grade/100			0.00	0.00	0.00	0.00	0.00	0.00	
t(3,lt)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
t(c,T):	1-stage	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
	2-stage	0.00	0.00	1.00	1.00	0.00	1.00	1.00	
t(c)	1-stage	4.1	4.1	7.1	6.5	7.1	6.5	6.2	
	2-stage	4.1	4.1	6.1	5.5	6.2	6.1	5.5	

Follow-Up Time Calculations									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
t(f,base)	2.20	2.20	3.50	4.00	3.30	3.50	4.00	3.30	
t(f,HV)	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	
P(HV)	0	0	0	0	0	0	0	0	
t(f)	2.2	2.2	3.5	4.0	3.3	3.5	4.0	3.3	

Worksheet 5-Effect of Upstream Signals

Computation 1-Queue Clearance Time at Upstream Signal				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
V prog				
Total Saturation Flow Rate, s (vph)				
Arrival Type				
Effective Green, g (sec)				
Cycle Length, C (sec)				
Rp (from Exhibit 16-11)				
Proportion vehicles arriving on green P				
g(q1)				
g(q2)				
g(q)				

Computation 2-Proportion of TWSC Intersection Time blocked				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
alpha				
beta				
Travel time, t(a) (sec)				
Smoothing Factor, F				
Proportion of conflicting flow, f				
Max platooned flow, V(c,max)				
Min platooned flow, V(c,min)				
Duration of blocked period, t(p)				
Proportion time blocked, p		0.000		0.000

Computation 3-Platoon Event Periods	Result
p(2)	0.000
p(5)	0.000
p(dom)	
p(subo)	
Constrained or unconstrained?	

Proportion unblocked for minor movements, p(x)	(1)	(2)	(3)
	Single-stage Process	Two-Stage Stage I	Process Stage II
p(1)			
p(4)			
p(7)			
p(8)			
p(9)			
p(10)			
p(11)			
p(12)			

Computation 4 and 5 Single-Stage Process									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
V c,x	365	354	1073	1072	297	1228	1126	362	
s									
Px									
V c,u,x									
C r,x									
C plat,x									

	Two-Stage Process							
	7		8		10		11	
	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2
V(c,x)	309	764	309	763	760	468	760	366
s		1500		1500		1500		1500
P(x)								
V(c,u,x)								
C(r,x)								
C(plat,x)								

Worksheet 6-Impedance and Capacity Equations

Step 1: RT from Minor St.	9	12
Conflicting Flows	297	362
Potential Capacity	747	687
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	747	687
Probability of Queue free St.	0.68	0.99
Step 2: LT from Major St.	4	1
Conflicting Flows	354	365
Potential Capacity	1216	1205
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	1216	1205
Probability of Queue free St.	0.84	1.00
Maj L-Shared Prob Q free St.		0.99
Step 3: TH from Minor St.	8	11
Conflicting Flows	1072	1126
Potential Capacity	222	207
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.83	0.83
Movement Capacity	185	172
Probability of Queue free St.	0.95	0.99
Step 4: LT from Minor St.	7	10
Conflicting Flows	1073	1228
Potential Capacity	200	156
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.82	0.79
Maj. L, Min T Adj. Imp Factor.	0.86	0.84
Cap. Adj. factor due to Impeding mvmnt	0.86	0.57
Movement Capacity	171	89

Worksheet 7-Computation of the Effect of Two-stage Gap Acceptance

Step 3: TH from Minor St.	8	11
Part 1 - First Stage		
Conflicting Flows	309	760
Potential Capacity	663	417
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.99	0.84
Movement Capacity	659	349
Probability of Queue free St.	0.97	0.99
Part 2 - Second Stage		
Conflicting Flows	763	366
Potential Capacity	416	626
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.84	0.99
Movement Capacity	348	622
Part 3 - Single Stage		
Conflicting Flows	1072	1126
Potential Capacity	222	207
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.83	0.83
Movement Capacity	185	172
Result for 2 stage process:		
a	0.95	0.95

Y	3.02	0.71
C t	313	293
Probability of Queue free St.	0.95	0.99
Step 4: LT from Minor St.	7	10
Part 1 - First Stage		
Conflicting Flows	309	760
Potential Capacity	705	401
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.99	0.84
Movement Capacity	701	335
Part 2 - Second Stage		
Conflicting Flows	764	468
Potential Capacity	399	579
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.82	0.65
Movement Capacity	328	378
Part 3 - Single Stage		
Conflicting Flows	1073	1228
Potential Capacity	200	156
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.82	0.79
Maj. L, Min T Adj. Imp Factor.	0.86	0.84
Cap. Adj. factor due to Impeding mvmnt	0.86	0.57
Movement Capacity	171	89
Results for Two-stage process:		
a	0.95	0.95
Y	3.51	2.73
C t	297	162

Worksheet 8-Shared Lane Calculations

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (vph)	39	19	242	4	3	6
Movement Capacity (vph)	297	313	747	162	293	687
Shared Lane Capacity (vph)	302				298	

Worksheet 9-Computation of Effect of Flared Minor Street Approaches

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
C sep	297	313	747	162	293	687
Volume	39	19	242	4	3	6
Delay						
Q sep						
Q sep +1						
round (Qsep +1)						
n max						
C sh	302				298	
SUM C sep						
n						
C act						

Worksheet 10-Delay, Queue Length, and Level of Service

Movement	1	4	7	8	9	10	11	12
Lane Config	LT	L	LT		R		LTR	
v (vph)	6	199	58		242		13	
C(m) (vph)	1205	1216	302		747		298	
v/c	0.00	0.16	0.19		0.32		0.04	
95% queue length	0.02	0.58	0.70		1.41		0.14	
Control Delay	8.0	8.5	19.7		12.1		17.6	
LOS	A	A	C		B		C	
Approach Delay				13.6			17.6	
Approach LOS				B			C	

Worksheet 11-Shared Major LT Impedance and Delay

	Movement 2	Movement 5
p(oj)	1.00	0.84
v(i1), Volume for stream 2 or 5	297	
v(i2), Volume for stream 3 or 6	0	
s(i1), Saturation flow rate for stream 2 or 5	1700	
s(i2), Saturation flow rate for stream 3 or 6	1700	
P*(oj)	0.99	
d(M,LT), Delay for stream 1 or 4	8.0	8.5
N, Number of major street through lanes	1	
d(rank,1) Delay for stream 2 or 5	0.0	

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - RANCHO PALOS VERDES
WEEKDAY (2:00-4:00) PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #1 PALOS VERDES DRIVE EAST/MIRALESTE DRIVE
\*\*\*\*\*

Average Delay (sec/veh): 141.8 Worst Case Level Of Service: F[437.4]
\*\*\*\*\*

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 0 0 0 1 0 1 0 1 0 0 0 0 0 0 0 1

Volume Module:

Base Vol: 0 242 197 342 262 0 0 0 0 208 0 278
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 0 242 197 342 262 0 0 0 0 208 0 278
Added Vol: 0 13 81 0 8 0 0 0 0 51 0 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 0 255 278 342 270 0 0 0 0 259 0 278
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 0 277 302 372 293 0 0 0 0 282 0 302
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 0 277 302 372 293 0 0 0 0 282 0 302

Critical Gap Module:

Critical Gap:xxxxx xxxx xxxxx 4.1 xxxx xxxxx xxxxx xxxx xxxxxx 6.4 xxxx 6.2
FollowUpTim:xxxxx xxxx xxxxxx 2.2 xxxx xxxxx xxxxx xxxx xxxxxx 3.5 xxxx 3.3

Capacity Module:

Cnflct Vol: xxxx xxxx xxxxxx 579 xxxx xxxxxx xxxx xxxx xxxxxx 1465 xxxx 428
Potent Cap.: xxxx xxxx xxxxxx 1004 xxxx xxxxxx xxxx xxxx xxxxxx 143 xxxxx 631
Move Cap.: xxxx xxxx xxxxxx 1004 xxxx xxxxxx xxxx xxxx xxxxxx 102 xxxxx 631
Volume/Cap: xxxx xxxx xxxxx 0.37 xxxx xxxxx xxxx xxxx xxxxxx 2.77 xxxxx 0.48

Level Of Service Module:

2Way95thQ: xxxx xxxx xxxxxx 1.7 xxxx xxxxxx xxxx xxxx xxxxxx 26.5 xxxxx 2.6
Control Del:xxxxx xxxxx xxxxxx 10.7 xxxxx xxxxxx xxxxxx xxxx xxxxxx 889.9 xxxxx 15.8

LOS by Move: \* \* \* B \* \* \* \* \* F \* \* C
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxxx xxxx xxxxxx xxxx xxxx xxxxxx xxxx xxxx xxxxxx xxxx xxxxx xxxxxx
SharedQueue:xxxxxx xxxxx xxxxxx xxxxxx xxxx xxxxxx xxxxxx xxxx xxxxxx xxxxxx xxxxx xxxxxx
Shrd ConDel:xxxxxx xxxx xxxxxx xxxxxx xxxx xxxxxx xxxxxx xxxx xxxxxx xxxxxx xxxxx xxxxxx

Shared LOS: \*
ApproachDel: xxxxxxx xxxxxxx xxxxxxx xxxxxxx 437.4
ApproachLOS: \* \* \* \* \* F

\*\*\*\*\*
Note: Queue reported is the number of cars per lane.
\*\*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - RANCHO PALOS VERDES
WEEKDAY (2:00-4:00) PEAK HOUR

Level Of Service Computation Report

ICU l(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #2 CREST DRIVE - COLLEGE ENTRANCE/PALOS VERDES DRIVE EAST
\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.566
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxxx
Optimal Cycle: 37 Level Of Service: A

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Permitted Permitted Permitted Permitted
Rights: Include Include Include Include
Lanes: 0 0 1! 0 0 0 1 0 0 1 1 0 2 0 1 1 0 2 0 1

Volume Module:

Base Vol: 21 7 116 229 4 52 32 141 21 58 135 143
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 21 7 116 229 4 52 32 141 21 58 135 143
Added Vol: 31 0 94 0 0 0 0 0 20 59 0 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 52 7 210 229 4 52 32 141 41 117 135 143
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 57 8 228 249 4 57 35 153 45 127 147 155
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 57 8 228 249 4 57 35 153 45 127 147 155
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 57 8 228 249 4 57 35 153 45 127 147 155

Saturation Flow Module:

Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 0.19 0.03 0.78 0.98 0.02 1.00 1.00 2.00 1.00 1.00 2.00 1.00
Final Sat.: 309 42 1249 1573 27 1600 1600 3200 1600 1600 3200 1600

Capacity Analysis Module:

Vol/Sat: 0.04 0.18 0.18 0.16 0.16 0.04 0.02 0.05 0.03 0.08 0.05 0.10
Crit Moves: \*\*\*\* \*\*\*\* \*\*\*\* \*\*\*\*

\*\*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - RANCHO PALOS VERDES
WEEKDAY (2:00-4:00) PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

\*\*\*\*\*

Intersection #3 PALOS VERDES DRIVE EAST/PALOS VERDES DRIVE SOUTH

\*\*\*\*\*

Average Delay (sec/veh): 5.7 Worst Case Level Of Service: D[ 28.2]

\*\*\*\*\*

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module:

Table with 12 columns representing different traffic metrics and 12 rows of data including Base Vol, Growth Adj, Initial Bse, etc.

Critical Gap Module:

Table with 12 columns for critical gap metrics and 2 rows of data including Critical Gp and FollowUpTim.

Capacity Module:

Table with 12 columns for capacity metrics and 4 rows of data including Cnflct Vol, Potent Cap, Move Cap, and Volume/Cap.

Level Of Service Module:

Table with 12 columns for level of service metrics and 8 rows of data including 2Way95thQ, Control Del, LOS by Move, etc.

Note: Queue reported is the number of cars per lane.

\*\*\*\*\*

HCS+: Unsignalized Intersections Release 5.2

TWO-WAY STOP CONTROL SUMMARY

Analyst: DK  
 Agency/Co.: RBF  
 Date Performed: 7/23/2007  
 Analysis Time Period: Afternoon Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 Without Project  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: Miraleste  
 Intersection Orientation: NS  
 Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	Northbound			Southbound		
	1 L	2 T	3 R	4 L	5 T	6 R
Volume	11	294	42	267	413	5
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	11	319	45	290	448	5
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L		TR
Upstream Signal?	No			No		

Minor Street: Approach Movement	Westbound			Eastbound		
	7 L	8 T	9 R	10 L	11 T	12 R
Volume	52	20	212	1	2	14
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	56	21	230	1	2	15
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage				/ No /		
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Delay, Queue Length, and Level of Service

Approach Movement Lane Config	NB	SB	Westbound		Eastbound			
	1 LT	4 L	7 LT	8 R	9 R	10 L	11 LTR	12
v (vph)	11	290	77		230		18	
C(m) (vph)	1118	1206	190		726		374	
v/c	0.01	0.24	0.41		0.32		0.05	
95% queue length	0.03	0.94	1.81		1.36		0.15	
Control Delay	8.3	8.9	36.3		12.2		15.1	
LOS	A	A	E		B		C	
Approach Delay				18.3			15.1	
Approach LOS				C			C	

HCS+: Unsignalized Intersections Release 5.2

Phone:  
 E-Mail:  
 Fax:

TWO-WAY STOP CONTROL(TWSC) ANALYSIS

Analyst: DK  
 Agency/Co.: RBF  
 Date Performed: 7/23/2007  
 Analysis Time Period: Afternoon Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 Without Project  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: Miraleste  
 Intersection Orientation: NS  
 Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street Movements	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	11	294	42	267	413	5
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	3	80	11	73	112	1
Hourly Flow Rate, HFR	11	319	45	290	448	5
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L		TR
Upstream Signal?	No			No		

Minor Street Movements	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	52	20	212	1	2	14
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	14	5	58	0	1	4
Hourly Flow Rate, HFR	56	21	230	1	2	15
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage				/ No /		
RT Channelized?	No					
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Pedestrian Volumes and Adjustments

Movements	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)	12.0	12.0	12.0	12.0
Walking Speed (ft/sec)	4.0	4.0	4.0	4.0
Percent Blockage	0	0	0	0

Upstream Signal Data							
	Prog. Flow vph	Sat Flow vph	Arrival Type	Green Time sec	Cycle Length sec	Prog. Speed mph	Distance to Signal feet
S2	Left-Turn						
	Through						
S5	Left-Turn						
	Through						

Worksheet 3-Data for Computing Effect of Delay to Major Street Vehicles

	Movement 2	Movement 5
Shared ln volume, major th vehicles:	319	
Shared ln volume, major rt vehicles:	0	
Sat flow rate, major th vehicles:	1700	
Sat flow rate, major rt vehicles:	1700	
Number of major street through lanes:	1	

Worksheet 4-Critical Gap and Follow-up Time Calculation

Critical Gap Calculation									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
t(c,base)	4.1	4.1	7.1	6.5	6.2	7.1	6.5	6.2	
t(c,hv)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
P(hv)	0	0	0	0	0	0	0	0	
t(c,g)			0.20	0.20	0.10	0.20	0.20	0.10	
Grade/100			0.00	0.00	0.00	0.00	0.00	0.00	
t(3,lt)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
t(c,T):	1-stage	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
	2-stage	0.00	0.00	1.00	1.00	0.00	1.00	1.00	
t(c)	1-stage	4.1	4.1	7.1	6.5	7.1	6.5	6.2	
	2-stage	4.1	4.1	6.1	5.5	6.2	6.1	5.5	

Follow-Up Time Calculations									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
t(f,base)	2.20	2.20	3.50	4.00	3.30	3.50	4.00	3.30	
t(f,HV)	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	
P(HV)	0	0	0	0	0	0	0	0	
t(f)	2.2	2.2	3.5	4.0	3.3	3.5	4.0	3.3	

Worksheet 5-Effect of Upstream Signals

Computation 1-Queue Clearance Time at Upstream Signal				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
V prog				
Total Saturation Flow Rate, s (vph)				
Arrival Type				
Effective Green, g (sec)				
Cycle Length, C (sec)				
Rp (from Exhibit 16-11)				
Proportion vehicles arriving on green P				
g(q1)				
g(q2)				
g(q)				

Computation 2-Proportion of TWSC Intersection Time blocked				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
alpha				
beta				
Travel time, t(a) (sec)				
Smoothing Factor, F				
Proportion of conflicting flow, f				
Max platooned flow, V(c,max)				
Min platooned flow, V(c,min)				
Duration of blocked period, t(p)				
Proportion time blocked, p		0.000		0.000

Computation 3-Platoon Event Periods		Result
p(2)		0.000
p(5)		0.000
p(dom)		
p(subo)		
Constrained or unconstrained?		

Proportion unblocked for minor movements, p(x)	(1)	(2)	(3)
	Single-stage Process	Two-Stage Stage I	Process Stage II
p(1)			
p(4)			
p(7)			
p(8)			
p(9)			
p(10)			
p(11)			
p(12)			

Computation 4 and 5 Single-Stage Process									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
V c,x	453	364	1380	1374	319	1519	1416	450	
s									
Px									
V c,u,x									
C r,x									
C plat,x									

Two-Stage Process								
	7		8		10		11	
	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2
V(c,x)	341	1039	341	1033	1030	489	1030	386
s		1500		1500		1500		1500
P(x)								
V(c,u,x)								
C(r,x)								
C(plat,x)								

Worksheet 6-Impedance and Capacity Equations

Step 1: RT from Minor St.	9	12
Conflicting Flows	319	450
Potential Capacity	726	613
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	726	613
Probability of Queue free St.	0.68	0.98
Step 2: LT from Major St.	4	1
Conflicting Flows	364	453
Potential Capacity	1206	1118
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	1206	1118
Probability of Queue free St.	0.76	0.99
Maj L-Shared Prob Q free St.		0.99
Step 3: TH from Minor St.	8	11
Conflicting Flows	1374	1416
Potential Capacity	147	139
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.75	0.75
Movement Capacity	110	104
Probability of Queue free St.	0.93	0.99
Step 4: LT from Minor St.	7	10
Conflicting Flows	1380	1519
Potential Capacity	123	98
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.74	0.70
Maj. L, Min T Adj. Imp Factor.	0.80	0.77
Cap. Adj. factor due to Impeding mvmnt	0.78	0.52
Movement Capacity	96	51

Worksheet 7-Computation of the Effect of Two-stage Gap Acceptance

Step 3: TH from Minor St.	8	11
Part 1 - First Stage		
Conflicting Flows	341	1030
Potential Capacity	642	313
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.99	0.76
Movement Capacity	634	238
Probability of Queue free St.	0.97	0.99
Part 2 - Second Stage		
Conflicting Flows	1033	386
Potential Capacity	312	614
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.76	0.99
Movement Capacity	237	607
Part 3 - Single Stage		
Conflicting Flows	1374	1416
Potential Capacity	147	139
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.75	0.75
Movement Capacity	110	104
Result for 2 stage process:		
a	0.95	0.95

Y	4.52	0.63
C t	210	201
Probability of Queue free St.	0.93	0.99
Step 4: LT from Minor St.	7	10
Part 1 - First Stage		
Conflicting Flows	341	1030
Potential Capacity	678	284
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.99	0.76
Movement Capacity	670	216
Part 2 - Second Stage		
Conflicting Flows	1039	489
Potential Capacity	281	564
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.73	0.65
Movement Capacity	206	368
Part 3 - Single Stage		
Conflicting Flows	1380	1519
Potential Capacity	123	98
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.74	0.70
Maj. L, Min T Adj. Imp Factor.	0.80	0.77
Cap. Adj. factor due to Impeding mvmnt	0.78	0.52
Movement Capacity	96	51
Results for Two-stage process:		
a	0.95	0.95
Y	5.80	6.11
C t	183	73

Worksheet 8-Shared Lane Calculations

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (vph)	56	21	230	1	2	15
Movement Capacity (vph)	183	210	726	73	201	613
Shared Lane Capacity (vph)	190				374	

Worksheet 9-Computation of Effect of Flared Minor Street Approaches

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
C sep	183	210	726	73	201	613
Volume	56	21	230	1	2	15
Delay						
Q sep						
Q sep +1						
round (Qsep +1)						
n max						
C sh	190				374	
SUM C sep						
n						
C act						

Worksheet 10-Delay, Queue Length, and Level of Service

Movement	1	4	7	8	9	10	11	12
Lane Config	LT	L	LT		R		LTR	
v (vph)	11	290	77		230		18	
C(m) (vph)	1118	1206	190		726		374	
v/c	0.01	0.24	0.41		0.32		0.05	
95% queue length	0.03	0.94	1.81		1.36		0.15	
Control Delay	8.3	8.9	36.3		12.2		15.1	
LOS	A	A	E		B		C	
Approach Delay				18.3			15.1	
Approach LOS				C			C	

Worksheet 11-Shared Major LT Impedance and Delay

	Movement 2	Movement 5
p(oj)	0.99	0.76
v(i1), Volume for stream 2 or 5	319	
v(i2), Volume for stream 3 or 6	0	
s(i1), Saturation flow rate for stream 2 or 5	1700	
s(i2), Saturation flow rate for stream 3 or 6	1700	
P*(oj)	0.99	
d(M,LT), Delay for stream 1 or 4	8.3	8.9
N, Number of major street through lanes	1	
d(rank,1) Delay for stream 2 or 5	0.1	

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - RANCHO PALOS VERDES
SATURDAY (11:00-1:00) PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #1 PALOS VERDES DRIVE EAST/MIRALESTE DRIVE

Average Delay (sec/veh): 19.4 Worst Case Level Of Service: E[ 48.5]

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 0 0 0 1 0 1 0 1 0 0 0 0 0 0 0 0 1 0 0 0 1

Volume Module:

Base Vol: 0 161 123 296 158 0 0 0 0 0 127 0 259
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 0 161 123 296 158 0 0 0 0 0 127 0 259
Added Vol: 0 5 31 0 7 0 0 0 0 0 42 0 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 0 166 154 296 165 0 0 0 0 0 169 0 259
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 0 180 167 322 179 0 0 0 0 0 184 0 282
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 0 180 167 322 179 0 0 0 0 0 184 0 282

Critical Gap Module:

Critical Gap: 4.1 6.4
FollowUpTim: 2.2 3.5

Capacity Module:

Cnflct Vol: 348 1087 264
Potent Cap.: 1222 241 779
Move Cap.: 1222 192 779
Volume/Cap: 0.26 0.96 0.36

Level Of Service Module:

2Way95thQ: 1.1 7.8 1.7
Control Del: 9.0 104.1 12.2
LOS by Move: A \* \* F \* B

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT
Shared Cap.:
SharedQueue:
Shrd ConDel:
Shared LOS:
ApproachDel:
ApproachLOS:

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - RANCHO PALOS VERDES
SATURDAY (11:00-1:00) PEAK HOUR

Level Of Service Computation Report

ICU l(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #2 CREST DRIVE - COLLEGE ENTRANCE/PALOS VERDES DRIVE EAST

Cycle (sec): 100 Critical Vol./Cap.(X): 0.263

Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx

Optimal Cycle: 24 Level Of Service: A

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Permitted Permitted Permitted Permitted
Rights: Include Include Include Include
Lanes: 0 0 1 0 0 0 1 0 0 1 1 0 2 0 1 1 0 2 0 1

Volume Module:

Base Vol: 0 3 4 65 0 17 19 143 3 1 106 53
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 0 3 4 65 0 17 19 143 3 1 106 53
Added Vol: 12 0 35 0 0 0 0 0 16 49 0 0
PasserByVol: 0 0 0 0 0 0 0 0 0 0 0 0
Initial Fut: 12 3 39 65 0 17 19 143 19 50 106 53
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 13 3 42 71 0 18 21 155 21 54 115 58
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 13 3 42 71 0 18 21 155 21 54 115 58
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 13 3 42 71 0 18 21 155 21 54 115 58

Saturation Flow Module:

Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 0.22 0.06 0.72 1.00 0.00 1.00 1.00 2.00 1.00 1.00 2.00 1.00
Final Sat.: 356 89 1156 1600 0 1600 1600 3200 1600 1600 3200 1600

Capacity Analysis Module:

Vol/Sat: 0.01 0.04 0.04 0.04 0.00 0.01 0.01 0.05 0.01 0.03 0.04 0.04
Crit Moves: \*\*\*\* \*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - RANCHO PALOS VERDES
SATURDAY (11:00-1:00) PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

\*\*\*\*\*

Intersection #3 PALOS VERDES DRIVE EAST/PALOS VERDES DRIVE SOUTH

\*\*\*\*\*

Average Delay (sec/veh): 2.3 Worst Case Level Of Service: C[ 15.9]

\*\*\*\*\*

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled

Rights: Include Include Include Include

Lanes: 0 0 0 0 0 1 0 0 0 1 1 0 1 0 0 0 0 1 0 1

-----|-----|-----|-----|

Volume Module:

Table with 16 columns and 12 rows showing traffic volume metrics: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, FinalVolume.

-----|-----|-----|-----|

Critical Gap Module:

Critical Gp:xxxxx xxxxx xxxxx 6.4 xxxxx 6.2 4.1 xxxxx xxxxxx xxxxx xxxxx xxxxxx

FollowUpTim:xxxxx xxxxx xxxxx 3.5 xxxxx 3.3 2.2 xxxxx xxxxxx xxxxx xxxxx xxxxxx

-----|-----|-----|-----|

Capacity Module:

Cnflct Vol: xxxxx xxxxx xxxxxx 1059 xxxxx 448 522 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Potent Cap.: xxxxx xxxxx xxxxxx 251 xxxxx 615 1055 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Move Cap.: xxxxx xxxxx xxxxxx 235 xxxxx 615 1055 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Volume/Cap: xxxxx xxxxx xxxxx 0.18 xxxxx 0.13 0.08 xxxxx xxxxx xxxxx xxxxx xxxxxx

-----|-----|-----|-----|

Level Of Service Module:

2Way95thQ: xxxxx xxxxx xxxxxx 0.6 xxxxx 0.4 0.3 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Control Del:xxxxxx xxxxx xxxxxx 23.6 xxxxx 11.7 8.7 xxxxx xxxxxx xxxxxx xxxxx xxxxxx

LOS by Move: \* \* \* C \* B A \* \* \* \* \* \*

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx

SharedQueue:xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx

Shrd ConDel:xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx

Shared LOS: \* \* \* \* \* \* \* \* \* \* \* \* \* \* \* \*

ApproachDel: xxxxxxx 15.9 xxxxxxx xxxxxxx

ApproachLOS: \* C \* \*

\*\*\*\*\*

Note: Queue reported is the number of cars per lane.

\*\*\*\*\*

HCS+: Unsignalized Intersections Release 5.2

TWO-WAY STOP CONTROL SUMMARY

Analyst: DK  
 Agency/Co.: RBF  
 Date Performed: 7/23/2007  
 Analysis Time Period: Saturday Mid-day Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 Without Project  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: Miraleste Dr  
 Intersection Orientation: NS  
 Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	Northbound			Southbound		
	1 L	2 T	3 R	4 L	5 T	6 R
Volume	3	276	51	193	292	7
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	3	299	55	209	317	7
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L	TR	
Upstream Signal?	No			No		

Minor Street: Approach Movement	Westbound			Eastbound		
	7 L	8 T	9 R	10 L	11 T	12 R
Volume	0	8	136	0	4	0
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	0	8	147	0	4	0
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage				/ No /		
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Delay, Queue Length, and Level of Service

Approach Movement Lane Config	NB	SB	Westbound		Eastbound		
	1 LT	4 L	7 LT	8 R	9 R	10 L	11 LTR
v (vph)	3	209	8		147		4
C(m) (vph)	1247	1216	320		745		297
v/c	0.00	0.17	0.03		0.20		0.01
95% queue length	0.01	0.62	0.08		0.73		0.04
Control Delay	7.9	8.6	16.5		11.0		17.3
LOS	A	A	C		B		C
Approach Delay				11.3			
Approach LOS				B			

HCS+: Unsignalized Intersections Release 5.2

Phone:  
 Fax:  
 E-Mail:

TWO-WAY STOP CONTROL(TWSC) ANALYSIS

Analyst: DK  
 Agency/Co.: RBF  
 Date Performed: 7/23/2007  
 Analysis Time Period: Saturday Mid-day Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 Without Project  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: Miraleste Dr  
 Intersection Orientation: NS  
 Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street Movements	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	3	276	51	193	292	7
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	1	75	14	52	79	2
Hourly Flow Rate, HFR	3	299	55	209	317	7
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L	TR	
Upstream Signal?	No			No		

Minor Street Movements	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	0	8	136	0	4	0
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	0	2	37	0	1	0
Hourly Flow Rate, HFR	0	8	147	0	4	0
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage				/ No /		
RT Channelized?	No					
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Pedestrian Volumes and Adjustments

Movements	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)	12.0	12.0	12.0	12.0
Walking Speed (ft/sec)	4.0	4.0	4.0	4.0
Percent Blockage	0	0	0	0

Upstream Signal Data							
	Prog. Flow vph	Sat Flow vph	Arrival Type	Green Time sec	Cycle Length sec	Prog. Speed mph	Distance to Signal feet
S2	Left-Turn						
	Through						
S5	Left-Turn						
	Through						

Worksheet 3-Data for Computing Effect of Delay to Major Street Vehicles

	Movement 2	Movement 5
Shared ln volume, major th vehicles:	299	
Shared ln volume, major rt vehicles:	0	
Sat flow rate, major th vehicles:	1700	
Sat flow rate, major rt vehicles:	1700	
Number of major street through lanes:	1	

Worksheet 4-Critical Gap and Follow-up Time Calculation

Critical Gap Calculation									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
t(c,base)	4.1	4.1	7.1	6.5	6.2	7.1	6.5	6.2	
t(c,hv)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
P(hv)	0	0	0	0	0	0	0	0	
t(c,g)			0.20	0.20	0.10	0.20	0.20	0.10	
Grade/100			0.00	0.00	0.00	0.00	0.00	0.00	
t(3,lt)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
t(c,T): 1-stage	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
2-stage	0.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	
t(c) 1-stage	4.1	4.1	7.1	6.5	6.2	7.1	6.5	6.2	
2-stage	4.1	4.1	6.1	5.5	6.2	6.1	5.5	6.2	

Follow-Up Time Calculations									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
t(f,base)	2.20	2.20	3.50	4.00	3.30	3.50	4.00	3.30	
t(f,HV)	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	
P(HV)	0	0	0	0	0	0	0	0	
t(f)	2.2	2.2	3.5	4.0	3.3	3.5	4.0	3.3	

Worksheet 5-Effect of Upstream Signals

Computation 1-Queue Clearance Time at Upstream Signal				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
V prog				
Total Saturation Flow Rate, s (vph)				
Arrival Type				
Effective Green, g (sec)				
Cycle Length, C (sec)				
Rp (from Exhibit 16-11)				
Proportion vehicles arriving on green P				
g(q1)				
g(q2)				
g(q)				

Computation 2-Proportion of TWSC Intersection Time blocked				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
alpha				
beta				
Travel time, t(a) (sec)				
Smoothing Factor, F				
Proportion of conflicting flow, f				
Max platooned flow, V(c,max)				
Min platooned flow, V(c,min)				
Duration of blocked period, t(p)				
Proportion time blocked, p		0.000		0.000

Computation 3-Platoon Event Periods		Result
p(2)		0.000
p(5)		0.000
p(dom)		
p(subo)		
Constrained or unconstrained?		

Proportion unblocked for minor movements, p(x)	(1)	(2)	(3)
	Single-stage Process	Two-Stage Stage I	Process Stage II
p(1)			
p(4)			
p(7)			
p(8)			
p(9)			
p(10)			
p(11)			
p(12)			

Computation 4 and 5 Single-Stage Process									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
V c,x	324	354	1045	1047	299	1148	1098	320	
s									
Px									
V c,u,x									
C r,x									
C plat,x									

	Two-Stage Process							
	7		8		10		11	
	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2
V(c,x)	305	740	305	742	738	410	738	360
s		1500		1500		1500		1500
P(x)								
V(c,u,x)								
C(r,x)								
C(plat,x)								

Worksheet 6-Impedance and Capacity Equations

Step 1: RT from Minor St.	9	12
Conflicting Flows	299	320
Potential Capacity	745	725
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	745	725
Probability of Queue free St.	0.80	1.00
Step 2: LT from Major St.	4	1
Conflicting Flows	354	324
Potential Capacity	1216	1247
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	1216	1247
Probability of Queue free St.	0.83	1.00
Maj L-Shared Prob Q free St.		1.00
Step 3: TH from Minor St.	8	11
Conflicting Flows	1047	1098
Potential Capacity	230	215
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.83	0.83
Movement Capacity	190	178
Probability of Queue free St.	0.98	0.99
Step 4: LT from Minor St.	7	10
Conflicting Flows	1045	1148
Potential Capacity	209	177
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.81	0.81
Maj. L, Min T Adj. Imp Factor.	0.86	0.85
Cap. Adj. factor due to Impeding mvmnt	0.86	0.68
Movement Capacity	179	121

Worksheet 7-Computation of the Effect of Two-stage Gap Acceptance

Step 3: TH from Minor St.	8	11
Part 1 - First Stage		
Conflicting Flows	305	738
Potential Capacity	666	427
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	1.00	0.83
Movement Capacity	664	354
Probability of Queue free St.	0.99	0.99
Part 2 - Second Stage		
Conflicting Flows	742	360
Potential Capacity	425	630
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.83	1.00
Movement Capacity	352	628
Part 3 - Single Stage		
Conflicting Flows	1047	1098
Potential Capacity	230	215
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.83	0.83
Movement Capacity	190	178
Result for 2 stage process:		
a	0.95	0.95

Y	2.98	0.73
C t	320	297
Probability of Queue free St.	0.98	0.99
Step 4: LT from Minor St.	7	10
Part 1 - First Stage		
Conflicting Flows	305	738
Potential Capacity	709	413
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	1.00	0.83
Movement Capacity	707	342
Part 2 - Second Stage		
Conflicting Flows	740	410
Potential Capacity	412	623
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.82	0.79
Movement Capacity	337	493
Part 3 - Single Stage		
Conflicting Flows	1045	1148
Potential Capacity	209	177
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.81	0.81
Maj. L, Min T Adj. Imp Factor.	0.86	0.85
Cap. Adj. factor due to Impeding mvmnt	0.86	0.68
Movement Capacity	179	121
Results for Two-stage process:		
a	0.95	0.95
Y	3.41	1.36
C t	308	233

Worksheet 8-Shared Lane Calculations

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (vph)	0	8	147	0	4	0
Movement Capacity (vph)	308	320	745	233	297	725
Shared Lane Capacity (vph)	320				297	

Worksheet 9-Computation of Effect of Flared Minor Street Approaches

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
C sep	308	320	745	233	297	725
Volume	0	8	147	0	4	0
Delay						
Q sep						
Q sep +1						
round (Qsep +1)						
n max						
C sh	320				297	
SUM C sep						
n						
C act						

Worksheet 10-Delay, Queue Length, and Level of Service

Movement	1	4	7	8	9	10	11	12
Lane Config	LT	L	LT		R		LTR	
v (vph)	3	209	8		147		4	
C(m) (vph)	1247	1216	320		745		297	
v/c	0.00	0.17	0.03		0.20		0.01	
95% queue length	0.01	0.62	0.08		0.73		0.04	
Control Delay	7.9	8.6	16.5		11.0		17.3	
LOS	A	A	C		B		C	
Approach Delay				11.3			17.3	
Approach LOS				B			C	

Worksheet 11-Shared Major LT Impedance and Delay

	Movement 2	Movement 5
p(oj)	1.00	0.83
v(i1), Volume for stream 2 or 5	299	
v(i2), Volume for stream 3 or 6	0	
s(i1), Saturation flow rate for stream 2 or 5	1700	
s(i2), Saturation flow rate for stream 3 or 6	1700	
P*(oj)	1.00	
d(M,LT), Delay for stream 1 or 4	7.9	8.6
N, Number of major street through lanes	1	
d(rank,1) Delay for stream 2 or 5	0.0	

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CITY OF LOS ANGELES
AM PEAK HOUR

Level Of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

Intersection #7 WESTERN AVENUE/CAPITOL DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 0.935
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: E

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module:

Table with 12 columns for traffic volumes and 12 rows for various metrics: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume.

Saturation Flow Module:

Table with 12 columns for saturation flow and 4 rows: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with 12 columns for capacity analysis and 4 rows: Vol/Sat, Crit Volume, Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CITY OF LOS ANGELES
AM PEAK HOUR

Level Of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

Intersection #8 WESTERN AVENUE/CRESTWOOD STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.812
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 107 Level Of Service: D

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module:

Table with 12 columns for traffic volumes and 12 rows for various metrics: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume.

Saturation Flow Module:

Table with 12 columns for saturation flow and 4 rows: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with 12 columns for capacity analysis and 4 rows: Vol/Sat, Crit Volume, Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CITY OF LOS ANGELES
AM PEAK HOUR

Level Of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

Intersection #9 WESTERN AVENUE/1ST STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 1.367
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module:

Table with 12 columns and 14 rows showing traffic volume data for various scenarios like Base Vol, Growth Adj, etc.

Saturation Flow Module:

Table with 12 columns and 4 rows showing saturation flow data for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns and 4 rows showing capacity analysis data for Vol/Sat, Crit Volume, and Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CITY OF LOS ANGELES
AM PEAK HOUR

Level Of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

Intersection #10 WESTERN AVENUE/9TH STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.609
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 51 Level Of Service: B

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module:

Table with 12 columns and 14 rows showing traffic volume data for various scenarios like Base Vol, Growth Adj, etc.

Saturation Flow Module:

Table with 12 columns and 4 rows showing saturation flow data for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns and 4 rows showing capacity analysis data for Vol/Sat, Crit Volume, and Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CITY OF LOS ANGELES
AM PEAK HOUR

Level Of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

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Intersection #11 WESTERN AVENUE/WEST 25TH STREET

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Cycle (sec): 100 Critical Vol./Cap.(X): 0.703
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 67 Level Of Service: C

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Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 3 rows: Movement (L-T-R), Control (Permitted, Protected), Rights (Include, Ovl, Include). Includes Min. Green and Lanes data.

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Volume Module: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume.

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Saturation Flow Module: Sat/Lane, Adjustment, Lanes, Final Sat.

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Capacity Analysis Module: Vol/Sat, Crit Volume, Crit Moves.

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MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CITY OF LOS ANGELES
PM PEAK HOUR

Level Of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

Intersection #7 WESTERN AVENUE/CAPITOL DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 0.822
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 112 Level Of Service: D

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module:

Table with 12 columns for traffic volumes and 12 rows for various volume metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module:

Table with 12 columns for saturation flow and 4 rows for Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with 12 columns for capacity analysis and 4 rows for Vol/Sat, Crit Volume, Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CITY OF LOS ANGELES
PM PEAK HOUR

Level Of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

Intersection #8 WESTERN AVENUE/CRESTWOOD STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.764
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 85 Level Of Service: C

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module:

Table with 12 columns for traffic volumes and 12 rows for various volume metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module:

Table with 12 columns for saturation flow and 4 rows for Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with 12 columns for capacity analysis and 4 rows for Vol/Sat, Crit Volume, Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CITY OF LOS ANGELES
PM PEAK HOUR

Level Of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

Intersection #9 WESTERN AVENUE/1ST STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 1.321
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module:

Table with 12 columns for traffic volumes and 12 rows for various metrics: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume.

Saturation Flow Module:

Table with 12 columns for saturation flow and 4 rows: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with 12 columns for capacity analysis and 4 rows: Vol/Sat, Crit Volume, Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CITY OF LOS ANGELES
PM PEAK HOUR

Level Of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

Intersection #10 WESTERN AVENUE/9TH STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.804
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 102 Level Of Service: D

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module:

Table with 12 columns for traffic volumes and 12 rows for various metrics: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume.

Saturation Flow Module:

Table with 12 columns for saturation flow and 4 rows: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with 12 columns for capacity analysis and 4 rows: Vol/Sat, Crit Volume, Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CITY OF LOS ANGELES
PM PEAK HOUR

Level Of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

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Intersection #11 WESTERN AVENUE/WEST 25TH STREET

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Cycle (sec): 100 Critical Vol./Cap.(X): 0.635
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 55 Level Of Service: B

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Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 3 rows: Movement, Control, Rights, Min. Green, Lanes.

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Volume Module: Table with 12 columns and 14 rows of traffic volume and adjustment factors.

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Saturation Flow Module: Table with 12 columns and 4 rows of saturation flow and adjustment factors.

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Capacity Analysis Module: Table with 12 columns and 4 rows of capacity analysis metrics.

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MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CALTRANS
AM PEAK HOUR

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #6 WESTERN AVENUE/TOSCANINI DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 0.675
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 17.4
Optimal Cycle: 51 Level Of Service: B

Table with 4 columns: Approach (North, South, East, West Bound) and 4 sub-columns for movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module table with 12 columns for different traffic scenarios and 12 rows for various metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module table with 12 columns for scenarios and 4 rows for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module table with 12 columns for scenarios and 12 rows for Vol/Sat, Crit Moves, Green/Cycle, etc.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CALTRANS
AM PEAK HOUR

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #7 WESTERN AVENUE/CAPITOL DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 0.815
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 24.0
Optimal Cycle: 75 Level Of Service: C

Table with 4 columns: Approach (North, South, East, West Bound) and 4 sub-columns for movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module table with 12 columns for different traffic scenarios and 12 rows for various metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module table with 12 columns for scenarios and 4 rows for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module table with 12 columns for scenarios and 12 rows for Vol/Sat, Crit Moves, Green/Cycle, etc.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CALTRANS
AM PEAK HOUR

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #8 WESTERN AVENUE/CRESTWOOD STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.733
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 17.5
Optimal Cycle: 59 Level Of Service: B

Table with 4 columns: Approach (North, South, East, West Bound) and Movement (L, T, R). Includes Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns for traffic volume metrics: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume.

Saturation Flow Module:

Table with 12 columns for saturation flow metrics: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with 12 columns for capacity analysis metrics: Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CALTRANS
AM PEAK HOUR

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #9 WESTERN AVENUE/1ST STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 1.129
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 67.9
Optimal Cycle: 180 Level Of Service: E

Table with 4 columns: Approach (North, South, East, West Bound) and Movement (L, T, R). Includes Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns for traffic volume metrics: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume.

Saturation Flow Module:

Table with 12 columns for saturation flow metrics: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with 12 columns for capacity analysis metrics: Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CALTRANS
AM PEAK HOUR

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #10 WESTERN AVENUE/9TH STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.505
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 21.8
Optimal Cycle: 37 Level Of Service: C

Table with 4 columns: Approach (North, South, East, West Bound) and Movement (L, T, R). Includes Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns for traffic volume metrics: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume.

Saturation Flow Module:

Table with 12 columns for saturation flow metrics: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with 12 columns for capacity analysis metrics: Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CALTRANS
AM PEAK HOUR

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #11 WESTERN AVENUE/WEST 25TH STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.626
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 25.8
Optimal Cycle: 46 Level Of Service: C

Table with 4 columns: Approach (North, South, East, West Bound) and Movement (L, T, R). Includes Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns for traffic volume metrics: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume.

Saturation Flow Module:

Table with 12 columns for saturation flow metrics: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with 12 columns for capacity analysis metrics: Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

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MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CALTRANS PM PEAK HOUR

Level Of Service Computation Report 2000 HCM Operations Method (Future Volume Alternative) Intersection #6 WESTERN AVENUE/TOSCANINI DRIVE. Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Cycle, Loss Time, Optimal Cycle, Control, Rights, Min. Green, Lanes, Volume Module, Saturation Flow Module, Capacity Analysis Module, and HCM2kAvgQ.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CALTRANS PM PEAK HOUR

Level Of Service Computation Report 2000 HCM Operations Method (Future Volume Alternative) Intersection #7 WESTERN AVENUE/CAPITOL DRIVE. Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Cycle, Loss Time, Optimal Cycle, Control, Rights, Min. Green, Lanes, Volume Module, Saturation Flow Module, Capacity Analysis Module, and HCM2kAvgQ.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CALTRANS
PM PEAK HOUR

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #8 WESTERN AVENUE/CRESTWOOD STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.589
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 13.6
Optimal Cycle: 43 Level Of Service: B

Table with 4 columns: Approach (North, South, East, West Bound) and Movement (L, T, R). Includes Control, Rights, Min. Green, and Lanes.

Volume Module table with 12 columns for traffic volume and 12 rows for various metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module table with 12 columns for Sat/Lane and 12 rows for Adjustment, Lanes, and Final Sat.

Capacity Analysis Module table with 12 columns for Vol/Sat and 12 rows for Crit Moves, Green/Cycle, Volume/Cap, etc.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CALTRANS
PM PEAK HOUR

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #9 WESTERN AVENUE/1ST STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 1.098
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 88.8
Optimal Cycle: 180 Level Of Service: F

Table with 4 columns: Approach (North, South, East, West Bound) and Movement (L, T, R). Includes Control, Rights, Min. Green, and Lanes.

Volume Module table with 12 columns for traffic volume and 12 rows for various metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module table with 12 columns for Sat/Lane and 12 rows for Adjustment, Lanes, and Final Sat.

Capacity Analysis Module table with 12 columns for Vol/Sat and 12 rows for Crit Moves, Green/Cycle, Volume/Cap, etc.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CALTRANS
PM PEAK HOUR

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #10 WESTERN AVENUE/9TH STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.652
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 23.5
Optimal Cycle: 48 Level Of Service: C

Table with columns: Approach, Movement, Control, Rights, Min. Green, Lanes. Rows for North, South, East, West bounds.

Volume Module:

Table with columns: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume.

Saturation Flow Module:

Table with columns: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with columns: Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CALTRANS
PM PEAK HOUR

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #11 WESTERN AVENUE/WEST 25TH STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.555
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 25.0
Optimal Cycle: 40 Level Of Service: C

Table with columns: Approach, Movement, Control, Rights, Min. Green, Lanes. Rows for North, South, East, West bounds.

Volume Module:

Table with columns: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume.

Saturation Flow Module:

Table with columns: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with columns: Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

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MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CMP
AM PEAK HOUR

Level Of Service Computation Report

ICU l(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #6 WESTERN AVENUE/TOSCANINI DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 0.819
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 71 Level Of Service: D

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module table with 12 columns representing different traffic flows and 12 rows of volume data including Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module table with 12 columns for Sat/Lane and Adjustment, and 12 rows of data.

Capacity Analysis Module table with 12 columns for Vol/Sat and Crit Moves, and 12 rows of data.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CMP
AM PEAK HOUR

Level Of Service Computation Report

ICU l(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #10 WESTERN AVENUE/9TH STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.643
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 44 Level Of Service: B

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module table with 12 columns representing different traffic flows and 12 rows of volume data including Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module table with 12 columns for Sat/Lane and Adjustment, and 12 rows of data.

Capacity Analysis Module table with 12 columns for Vol/Sat and Crit Moves, and 12 rows of data.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CMP
PM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #6 WESTERN AVENUE/TOSCANINI DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 0.712
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 52 Level Of Service: C

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L-T-R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 10 columns representing different traffic movements and 10 rows of volume data including Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module:

Table with 10 columns for Sat/Lane, Adjustment, Lanes, and Final Sat. values.

Capacity Analysis Module:

Table with 10 columns for Vol/Sat and Crit Moves values.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CMP
PM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #10 WESTERN AVENUE/9TH STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.816
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 70 Level Of Service: D

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L-T-R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 10 columns representing different traffic movements and 10 rows of volume data including Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module:

Table with 10 columns for Sat/Lane, Adjustment, Lanes, and Final Sat. values.

Capacity Analysis Module:

Table with 10 columns for Vol/Sat and Crit Moves values.

**Mitigated Forecast Existing With  
Project Conditions**

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
MITIGATED EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CITY OF RPV
AM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

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Intersection #1 PALOS VERDES DRIVE EAST/MIRALESTE DRIVE
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Cycle (sec): 100 Critical Vol./Cap.(X): 0.954
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 137 Level Of Service: E

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L-T-R). Includes Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns for traffic volume metrics: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume, OvAdjVol.

Saturation Flow Module:

Table with 12 columns for saturation flow metrics: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with 12 columns for capacity analysis metrics: Vol/Sat, OvAdjV/S, Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
MITIGATED EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CITY OF RPV
AM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #7 WESTERN AVENUE/CAPITOL DRIVE
\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.862
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 84 Level Of Service: D

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L-T-R). Includes Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns for traffic volume metrics: Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume.

Saturation Flow Module:

Table with 12 columns for saturation flow metrics: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with 12 columns for capacity analysis metrics: Vol/Sat, Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
MITIGATED EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CITY OF RPV
PM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #1 PALOS VERDES DRIVE EAST/MIRALESTE DRIVE
\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.875
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 89 Level Of Service: D

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Includes Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns for traffic volume and 12 rows for various metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module:

Table with 12 columns for saturation flow and 5 rows for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns for capacity analysis and 4 rows for Vol/Sat, OvlAdjV/S, and Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
MITIGATED EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CITY OF RPV
PM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #7 WESTERN AVENUE/CAPITOL DRIVE
\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.781
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 63 Level Of Service: C

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Includes Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns for traffic volume and 12 rows for various metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module:

Table with 12 columns for saturation flow and 5 rows for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns for capacity analysis and 4 rows for Vol/Sat, Crit Moves, and other metrics.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
MITIGATED EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CITY OF RPV
WEEKDAY (11:00-1:00) PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #1 PALOS VERDES DRIVE EAST/MIRALESTE DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 0.776
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 62 Level Of Service: C

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns representing different traffic flows. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume, and OvlAdjVol.

Saturation Flow Module:

Table with 12 columns representing different traffic flows. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns representing different traffic flows. Rows include Vol/Sat, OvlAdjV/S, and Crit Moves.

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 MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)  
 MITIGATED EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CITY OF RPV  
 WEEKDAY (2:00-4:00) PEAK HOUR  
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Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\*

Intersection #1 PALOS VERDES DRIVE EAST/MIRALESTE DRIVE

\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.870  
 Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx  
 Optimal Cycle: 87 Level Of Service: D  
 \*\*\*\*\*

Approach:	North Bound			South Bound			East Bound			West Bound			
Movement:	L	T	R	L	T	R	L	T	R	L	T	R	
Control:	Protected			Protected			Protected			Protected			
Rights:	Include			Include			Include			Ovl			
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0	
Lanes:	0	0	1	0	1	0	1	0	0	0	0	0	1

Volume Module:

Base Vol:	0	242	197	342	262	0	0	0	0	208	0	278
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	242	197	342	262	0	0	0	0	208	0	278
Added Vol:	0	13	81	0	8	0	0	0	0	51	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	255	278	342	270	0	0	0	0	259	0	278
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
PHF Volume:	0	277	302	372	293	0	0	0	0	282	0	302
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	277	302	372	293	0	0	0	0	282	0	302
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	277	302	372	293	0	0	0	0	282	0	302
OvlAdjVol:	0											

Saturation Flow Module:

Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	0.00	0.48	0.52	1.00	1.00	0.00	0.00	0.00	0.00	1.00	0.00	1.00
Final Sat.:	0	765	835	1600	1600	0	0	0	0	1600	0	1600

Capacity Analysis Module:

Vol/Sat:	0.00	0.36	0.36	0.23	0.18	0.00	0.00	0.00	0.00	0.18	0.00	0.19
OvlAdjV/S:												0.00
Crit Moves:	****			****			****			****		

\*\*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
MITIGATED EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CITY OF RPV
SATURDAY (11:00-1:00) PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #1 PALOS VERDES DRIVE EAST/MIRALESTE DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 0.633
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 43 Level Of Service: B

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns representing different traffic volumes and adjustment factors. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume, and OvlAdjVol.

Saturation Flow Module:

Table with 12 columns representing saturation flow and adjustment factors. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns representing capacity analysis metrics. Rows include Vol/Sat, OvlAdjV/S, and Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
MITIGATED EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CITY OF LOS ANGELES
AM PEAK HOUR

Level Of Service Computation Report
Circular 212 Planning Method (Future Volume Alternative)

Intersection #7 WESTERN AVENUE/CAPITOL DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 0.855
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 138 Level Of Service: D

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns representing different traffic movements. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, PasserByVol, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and FinalVolume.

Saturation Flow Module:

Table with 12 columns representing different traffic movements. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns representing different traffic movements. Rows include Vol/Sat, Crit Volume, and Crit Moves.

\*\*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
MITIGATED EXISTING PLUS MARYMOUNT PROJECT CONDITIONS - CITY OF LOS ANGELES
PM PEAK HOUR

Level Of Service Computation Report
Circular 212 Planning Method (Future Volume Alternative)

Intersection #7 WESTERN AVENUE/CAPITOL DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 0.764
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 85 Level Of Service: C

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module table with 12 columns representing different traffic movements and 11 rows of volume-related metrics.

Saturation Flow Module table with 12 columns and 5 rows of saturation flow data.

Capacity Analysis Module table with 12 columns and 4 rows of capacity analysis data.

\*\*\*\*\*

## **Forecast Year 2012 Without Project Conditions**

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)  
 FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF RPV  
 AM PEAK HOUR

Level of Service Computation Report  
 2000 HCM Unsignalized Method (Future Volume Alternative)

\*\*\*\*\*  
 Intersection #1 PALOS VERDES DRIVE EAST/MIRALESTE DRIVE  
 \*\*\*\*\*

Average Delay (sec/veh): 98.3 Worst Case Level Of Service: F[311.7]  
 \*\*\*\*\*

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Uncontrolled	Uncontrolled	Stop Sign	Stop Sign
Rights:	Include	Include	Include	Include
Lanes:	0 0 0 1 0	1 0 1 0 0	0 0 0 0 0	1 0 0 0 1

Volume Module:

Base Vol:	0	397	180	379	351	0	0	0	0	156	0	416
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	397	180	379	351	0	0	0	0	156	0	416
Added Vol:	0	0	2	0	1	0	0	0	0	8	0	0
Cumulative:	0	1	4	4	3	0	0	0	0	4	0	4
Initial Fut:	0	398	186	383	355	0	0	0	0	168	0	420
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
PHF Volume:	0	433	202	416	386	0	0	0	0	183	0	457
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	433	202	416	386	0	0	0	0	183	0	457

Critical Gap Module:

Critical Gp:	xxxxx	xxxx	xxxxx	4.1	xxxx	xxxxx	xxxxx	xxxx	xxxxx	6.4	xxxx	6.2
FollowUpTim:	xxxxx	xxxx	xxxxx	2.2	xxxx	xxxxx	xxxxx	xxxx	xxxxx	3.5	xxxx	3.3

Capacity Module:

Cnflct Vol:	xxxx	xxxx	xxxxx	635	xxxx	xxxxx	xxxx	xxxx	xxxxx	1752	xxxx	534
Potent Cap.:	xxxx	xxxx	xxxxx	958	xxxx	xxxxx	xxxx	xxxx	xxxxx	95	xxxx	550
Move Cap.:	xxxx	xxxx	xxxxx	958	xxxx	xxxxx	xxxx	xxxx	xxxxx	63	xxxx	550
Volume/Cap:	xxxx	xxxx	xxxxx	0.43	xxxx	xxxxx	xxxx	xxxx	xxxxx	2.91	xxxx	0.83

Level Of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxx	2.2	xxxx	xxxxx	xxxx	xxxx	xxxxx	18.7	xxxx	8.5
Control Del:	xxxxx	xxxx	xxxxx	11.6	xxxx	xxxxx	xxxxx	xxxx	xxxxx	1002	xxxx	35.8
LOS by Move:	*	*	*	B	*	*	*	*	*	F	*	E
Movement:	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*
ApproachDel:	xxxxxx			xxxxxx			xxxxxx			311.7		
ApproachLOS:	*			*			*			F		

Note: Queue reported is the number of cars per lane.  
 \*\*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)  
 FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF RPV  
 AM PEAK HOUR

Level of Service Computation Report  
 ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\*  
 Intersection #2 CREST DRIVE - COLLEGE ENTRANCE/PALOS VERDES DRIVE EAST  
 \*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.495  
 Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx  
 Optimal Cycle: 33 Level Of Service: A  
 \*\*\*\*\*

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Permitted	Permitted	Permitted	Permitted
Rights:	Include	Include	Include	Include
Min. Green:	0 0 0	0 0 0	0 0 0	0 0 0
Lanes:	0 0 1 0 0	0 1 0 0 1	1 0 1 0 1	1 0 1 0 1

Volume Module:

Base Vol:	6	0	15	214	6	37	48	185	31	139	117	208
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	6	0	15	214	6	37	48	185	31	139	117	208
Added Vol:	1	0	2	0	0	0	0	0	3	10	0	0
Cumulative:	0	0	0	4	0	0	0	6	0	0	8	4
Initial Fut:	7	0	17	218	6	37	48	191	34	149	125	212
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
PHF Volume:	8	0	18	237	7	40	52	208	37	162	136	230
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	8	0	18	237	7	40	52	208	37	162	136	230
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	8	0	18	237	7	40	52	208	37	162	136	230

Saturation Flow Module:

Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	0.29	0.00	0.71	0.97	0.03	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Sat.:	467	0	1133	1557	43	1600	1600	1600	1600	1600	1600	1600

Capacity Analysis Module:

Vol/Sat:	0.00	0.00	0.02	0.15	0.15	0.03	0.03	0.13	0.02	0.10	0.08	0.14
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****

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MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF RPV
AM PEAK HOUR

Level of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #3 PALOS VERDES DRIVE EAST/PALOS VERDES DRIVE SOUTH

Average Delay (sec/veh): 3.6 Worst Case Level Of Service: D[ 28.2]

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 3 rows: Movement, Control, Rights, Lanes.

Volume Module: Table with 12 columns for traffic volumes and 12 rows for various metrics like Base Vol, Growth Adj, Initial Bse, etc.

Critical Gap Module: Table with 12 columns for gap metrics and 2 rows for Critical Gp and FollowUpTim.

Capacity Module: Table with 12 columns for capacity metrics and 5 rows for Cnflct Vol, Potent Cap, Move Cap, Volume/Cap.

Level Of Service Module: Table with 12 columns for LOS metrics and 5 rows for 2Way95thQ, Control Del, LOS by Move, Shared Cap, Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF RPV
AM PEAK HOUR

Level of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #5 MIRALESTE DRIVE/1ST STREET

Average Delay (sec/veh): 5.7 Worst Case Level Of Service: B[ 14.8]

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 3 rows: Movement, Control, Rights, Lanes.

Volume Module: Table with 12 columns for traffic volumes and 12 rows for various metrics like Base Vol, Growth Adj, Initial Bse, etc.

Critical Gap Module: Table with 12 columns for gap metrics and 2 rows for Critical Gp and FollowUpTim.

Capacity Module: Table with 12 columns for capacity metrics and 5 rows for Cnflct Vol, Potent Cap, Move Cap, Volume/Cap.

Level Of Service Module: Table with 12 columns for LOS metrics and 5 rows for 2Way95thQ, Control Del, LOS by Move, Shared Cap, Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

HCS+: Unsignalized Intersections Release 5.2

TWO-WAY STOP CONTROL SUMMARY

Analyst: DK  
 Agency/Co.: RBF  
 Date Performed: 7/23/2007  
 Analysis Time Period: AM Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 Without Project  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: Miraleste Dr  
 Intersection Orientation: NS  
 Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	Northbound			Southbound		
	1 L	2 T	3 R	4 L	5 T	6 R
Volume	17	385	130	177	414	3
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	18	418	141	192	449	3
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L	TR	
Upstream Signal?	No			No		

Minor Street: Approach Movement	Westbound			Eastbound		
	7 L	8 T	9 R	10 L	11 T	12 R
Volume	40	14	241	4	4	15
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	43	15	261	4	4	16
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage				/ No /		
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Delay, Queue Length, and Level of Service

Approach Movement Lane Config	NB	SB	Westbound		Eastbound		
	1 LT	4 L	7 LT	8 R	9 R	10 L	11 LTR
v (vph)	18	192	58		261		24
C(m) (vph)	1119	1022	246		639		219
v/c	0.02	0.19	0.24		0.41		0.11
95% queue length	0.05	0.69	0.89		1.99		0.36
Control Delay	8.3	9.3	24.1		14.5		23.5
LOS	A	A	C		B		C
Approach Delay				16.2			23.5
Approach LOS				C			C

HCS+: Unsignalized Intersections Release 5.2

Phone:  
 Fax:  
 E-Mail:

TWO-WAY STOP CONTROL(TWSC) ANALYSIS

Analyst: DK  
 Agency/Co.: RBF  
 Date Performed: 7/23/2007  
 Analysis Time Period: AM Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 Without Project  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: Miraleste Dr  
 Intersection Orientation: NS  
 Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street Movements	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	17	385	130	177	414	3
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	5	105	35	48	112	1
Hourly Flow Rate, HFR	18	418	141	192	449	3
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L	TR	
Upstream Signal?	No			No		

Minor Street Movements	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	40	14	241	4	4	15
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	11	4	65	1	1	4
Hourly Flow Rate, HFR	43	15	261	4	4	16
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage				/ No /		
RT Channelized?	No					
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Pedestrian Volumes and Adjustments

Movements	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)	12.0	12.0	12.0	12.0
Walking Speed (ft/sec)	4.0	4.0	4.0	4.0
Percent Blockage	0	0	0	0

Upstream Signal Data							
	Prog. Flow vph	Sat Flow vph	Arrival Type	Green Time sec	Cycle Length sec	Prog. Speed mph	Distance to Signal feet
S2	Left-Turn						
	Through						
S5	Left-Turn						
	Through						

Worksheet 3-Data for Computing Effect of Delay to Major Street Vehicles

	Movement 2	Movement 5
Shared ln volume, major th vehicles:	418	
Shared ln volume, major rt vehicles:	0	
Sat flow rate, major th vehicles:	1700	
Sat flow rate, major rt vehicles:	1700	
Number of major street through lanes:	1	

Worksheet 4-Critical Gap and Follow-up Time Calculation

Critical Gap Calculation									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
t(c,base)	4.1	4.1	7.1	6.5	6.2	7.1	6.5	6.2	
t(c,hv)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
P(hv)	0	0	0	0	0	0	0	0	
t(c,g)			0.20	0.20	0.10	0.20	0.20	0.10	
Grade/100			0.00	0.00	0.00	0.00	0.00	0.00	
t(3,lt)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
t(c,T):	1-stage	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
	2-stage	0.00	0.00	1.00	1.00	0.00	1.00	1.00	
t(c)	1-stage	4.1	4.1	7.1	6.5	7.1	6.5	6.2	
	2-stage	4.1	4.1	6.1	5.5	6.2	6.1	5.5	

Follow-Up Time Calculations									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
t(f,base)	2.20	2.20	3.50	4.00	3.30	3.50	4.00	3.30	
t(f,HV)	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	
P(HV)	0	0	0	0	0	0	0	0	
t(f)	2.2	2.2	3.5	4.0	3.3	3.5	4.0	3.3	

Worksheet 5-Effect of Upstream Signals

Computation 1-Queue Clearance Time at Upstream Signal				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
V prog				
Total Saturation Flow Rate, s (vph)				
Arrival Type				
Effective Green, g (sec)				
Cycle Length, C (sec)				
Rp (from Exhibit 16-11)				
Proportion vehicles arriving on green P				
g(q1)				
g(q2)				
g(q)				

Computation 2-Proportion of TWSC Intersection Time blocked				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
alpha				
beta				
Travel time, t(a) (sec)				
Smoothing Factor, F				
Proportion of conflicting flow, f				
Max platooned flow, V(c,max)				
Min platooned flow, V(c,min)				
Duration of blocked period, t(p)				
Proportion time blocked, p		0.000		0.000

Computation 3-Platoon Event Periods	Result
p(2)	0.000
p(5)	0.000
p(dom)	
p(subo)	
Constrained or unconstrained?	

Proportion unblocked for minor movements, p(x)	(1)	(2)	(3)
	Single-stage Process	Two-Stage Stage I	Process Stage II
p(1)			
p(4)			
p(7)			
p(8)			
p(9)			
p(10)			
p(11)			
p(12)			

Computation 4 and 5 Single-Stage Process									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
V c,x	452	559	1298	1290	418	1496	1429	450	
s									
Px									
V c,u,x									
C r,x									
C plat,x									

	7		8		10		11	
	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2
V(c,x)	454	844	454	836	834	662	834	595
s		1500		1500		1500		1500
P(x)								
V(c,u,x)								
C(r,x)								
C(plat,x)								

Worksheet 6-Impedance and Capacity Equations

Step 1: RT from Minor St.	9	12
Conflicting Flows	418	450
Potential Capacity	639	613
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	639	613
Probability of Queue free St.	0.59	0.97
Step 2: LT from Major St.	4	1
Conflicting Flows	559	452
Potential Capacity	1022	1119
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	1022	1119
Probability of Queue free St.	0.81	0.98
Maj L-Shared Prob Q free St.		0.98
Step 3: TH from Minor St.	8	11
Conflicting Flows	1290	1429
Potential Capacity	165	136
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.79	0.79
Movement Capacity	131	108
Probability of Queue free St.	0.96	0.98
Step 4: LT from Minor St.	7	10
Conflicting Flows	1298	1496
Potential Capacity	140	102
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.78	0.76
Maj. L, Min T Adj. Imp Factor.	0.83	0.82
Cap. Adj. factor due to Impeding mvmnt	0.81	0.48
Movement Capacity	114	49

Worksheet 7-Computation of the Effect of Two-stage Gap Acceptance

Step 3: TH from Minor St.	8	11
Part 1 - First Stage		
Conflicting Flows	454	834
Potential Capacity	573	386
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.98	0.81
Movement Capacity	561	313
Probability of Queue free St.	0.97	0.99
Part 2 - Second Stage		
Conflicting Flows	836	595
Potential Capacity	385	496
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.81	0.98
Movement Capacity	313	485
Part 3 - Single Stage		
Conflicting Flows	1290	1429
Potential Capacity	165	136
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.79	0.79
Movement Capacity	131	108
Result for 2 stage process:		
a	0.95	0.95

Y	2.62	1.11
C t	265	225
Probability of Queue free St.	0.96	0.98
Step 4: LT from Minor St.	7	10
Part 1 - First Stage		
Conflicting Flows	454	834
Potential Capacity	589	365
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.98	0.81
Movement Capacity	576	296
Part 2 - Second Stage		
Conflicting Flows	844	662
Potential Capacity	361	454
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.78	0.56
Movement Capacity	282	256
Part 3 - Single Stage		
Conflicting Flows	1298	1496
Potential Capacity	140	102
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.78	0.76
Maj. L, Min T Adj. Imp Factor.	0.83	0.82
Cap. Adj. factor due to Impeding mvmnt	0.81	0.48
Movement Capacity	114	49
Results for Two-stage process:		
a	0.95	0.95
Y	3.08	16.47
C t	240	61

Worksheet 8-Shared Lane Calculations

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (vph)	43	15	261	4	4	16
Movement Capacity (vph)	240	265	639	61	225	613
Shared Lane Capacity (vph)	246				219	

Worksheet 9-Computation of Effect of Flared Minor Street Approaches

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
C sep	240	265	639	61	225	613
Volume	43	15	261	4	4	16
Delay						
Q sep						
Q sep +1						
round (Qsep +1)						
n max						
C sh	246				219	
SUM C sep						
n						
C act						

Worksheet 10-Delay, Queue Length, and Level of Service

Movement	1	4	7	8	9	10	11	12
Lane Config	LT	L	LT		R		LTR	
v (vph)	18	192	58		261		24	
C(m) (vph)	1119	1022	246		639		219	
v/c	0.02	0.19	0.24		0.41		0.11	
95% queue length	0.05	0.69	0.89		1.99		0.36	
Control Delay	8.3	9.3	24.1		14.5		23.5	
LOS	A	A	C		B		C	
Approach Delay				16.2			23.5	
Approach LOS				C			C	

Worksheet 11-Shared Major LT Impedance and Delay

	Movement 2	Movement 5
p(oj)	0.98	0.81
v(i1), Volume for stream 2 or 5	418	
v(i2), Volume for stream 3 or 6	0	
s(i1), Saturation flow rate for stream 2 or 5	1700	
s(i2), Saturation flow rate for stream 3 or 6	1700	
P*(oj)	0.98	
d(M,LT), Delay for stream 1 or 4	8.3	9.3
N, Number of major street through lanes	1	
d(rank,1) Delay for stream 2 or 5	0.2	

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF RPV
AM PEAK HOUR

Level of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #6 WESTERN AVENUE/TOSCANINI DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 0.901
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 100 Level Of Service: E

Table with 4 columns: Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns for traffic volume and 12 columns for adjustment factors. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module:

Table with 12 columns for saturation flow. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns for capacity analysis. Rows include Vol/Sat and Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF RPV
AM PEAK HOUR

Level of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #7 WESTERN AVENUE/CAPITOL DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 1.057
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F

Table with 4 columns: Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns for traffic volume and 12 columns for adjustment factors. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module:

Table with 12 columns for saturation flow. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns for capacity analysis. Rows include Vol/Sat and Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF RPV
AM PEAK HOUR

Level of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #8 WESTERN AVENUE/CRESTWOOD STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.864
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 85 Level Of Service: D

Table with 4 columns: Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 11 columns for traffic volume and 11 rows for various adjustment factors like Growth Adj, Initial Bse, Added Vol, etc.

Saturation Flow Module:

Table with 11 columns for saturation flow and 4 rows for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 11 columns for capacity analysis and 3 rows for Vol/Sat, Crit Moves, and other metrics.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF RPV
AM PEAK HOUR

Level of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

Intersection #9 WESTERN AVENUE/1ST STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 1.516
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F

Table with 4 columns: Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 11 columns for traffic volume and 11 rows for various adjustment factors like Growth Adj, Initial Bse, Added Vol, etc.

Saturation Flow Module:

Table with 11 columns for saturation flow and 4 rows for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 11 columns for capacity analysis and 3 rows for Vol/Sat, Crit Volume, Crit Moves, and other metrics.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF RPV
AM PEAK HOUR

Level of Service Computation Report
Circular 212 Planning Method (Future Volume Alternative)

Intersection #10 WESTERN AVENUE/9TH STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.659
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 59 Level Of Service: B

Table with 4 columns: Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module table with 12 columns for traffic volumes and 12 columns for various adjustment factors (Growth, Initial, Added, Cumulative, Initial Fut, User, PHF, PCE, MLF, Final).

Saturation Flow Module table with 12 columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module table with 12 columns for Vol/Sat, Crit Volume, and Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF RPV
AM PEAK HOUR

Level of Service Computation Report
Circular 212 Planning Method (Future Volume Alternative)

Intersection #11 WESTERN AVENUE/WEST 25TH STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.813
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 107 Level Of Service: D

Table with 4 columns: Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module table with 12 columns for traffic volumes and 12 columns for various adjustment factors (Growth, Initial, Added, Cumulative, Initial Fut, User, PHF, PCE, MLF, Final).

Saturation Flow Module table with 12 columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module table with 12 columns for Vol/Sat, Crit Volume, and Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF RPV PM PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

\*\*\*\*\* Intersection #1 PALOS VERDES DRIVE EAST/MIRALESTE DRIVE \*\*\*\*\*

Average Delay (sec/veh): 198.0 Worst Case Level Of Service: F[469.2]

Table with columns: Approach, North Bound, South Bound, East Bound, West Bound, Movement, Control, Rights, Lanes. Rows include Uncontrolled, Stop Sign, and lane configurations.

Volume Module table with columns: Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume. Rows for North, South, East, West bounds.

Critical Gap Module table with columns: Critical Gap, FollowUpTim. Rows for North, South, East, West bounds.

Capacity Module table with columns: Cnflct Vol, Potent Cap., Move Cap., Volume/Cap. Rows for North, South, East, West bounds.

Level Of Service Module table with columns: 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS. Rows for North, South, East, West bounds.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF RPV PM PEAK HOUR

Level Of Service Computation Report

ICU l(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\* Intersection #2 CREST DRIVE - COLLEGE ENTRANCE/PALOS VERDES DRIVE EAST \*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.393 Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx Optimal Cycle: 28 Level Of Service: A

Table with columns: Approach, North Bound, South Bound, East Bound, West Bound, Movement, Control, Rights, Lanes. Rows include Permitted, Include and lane configurations.

Volume Module table with columns: Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume. Rows for North, South, East, West bounds.

Critical Gap Module table with columns: Critical Gap, FollowUpTim. Rows for North, South, East, West bounds.

Capacity Module table with columns: Sat/Lane, Adjustment, Lanes, Final Sat. Rows for North, South, East, West bounds.

Capacity Analysis Module table with columns: Vol/Sat, Crit Moves. Rows for North, South, East, West bounds.

\*\*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF RPV
PM PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #3 PALOS VERDES DRIVE EAST/PALOS VERDES DRIVE SOUTH
\*\*\*\*\*

Average Delay (sec/veh): 3.7 Worst Case Level Of Service: D[ 31.9]

Table with columns: Approach, Movement, Control, Rights, Lanes. Rows for North, South, East, West bounds.

Volume Module:

Table with columns: Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume.

Critical Gap Module:

Table with columns: Critical Gap, FollowUpTim.

Capacity Module:

Table with columns: Cnflct Vol, Potent Cap., Move Cap., Volume/Cap.

Level Of Service Module:

Table with columns: 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., Shared Queue, Shrd ConDel, Shared LOS, Approach Del, Approach LOS.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF RPV
PM PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #5 MIRALESTE DRIVE/1ST STREET
\*\*\*\*\*

Average Delay (sec/veh): 4.6 Worst Case Level Of Service: B[ 14.9]

Table with columns: Approach, Movement, Control, Rights, Lanes. Rows for North, South, East, West bounds.

Volume Module:

Table with columns: Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume.

Critical Gap Module:

Table with columns: Critical Gap, FollowUpTim.

Capacity Module:

Table with columns: Cnflct Vol, Potent Cap., Move Cap., Volume/Cap.

Level Of Service Module:

Table with columns: 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., Shared Queue, Shrd ConDel, Shared LOS, Approach Del, Approach LOS.

Note: Queue reported is the number of cars per lane.

HCS+: Unsignalized Intersections Release 5.2

TWO-WAY STOP CONTROL SUMMARY

Analyst: DK  
 Agency/Co.: RBF  
 Date Performed: 7/23/2007  
 Analysis Time Period: PM Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 Without Project  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: Miraleste Dr  
 Intersection Orientation: NS  
 Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	Northbound			Southbound		
	1 L	2 T	3 R	4 L	5 T	6 R
Volume	7	402	69	166	477	2
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	7	436	74	180	518	2
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L	TR	
Upstream Signal?	No			No		

Minor Street: Approach Movement	Westbound			Eastbound		
	7 L	8 T	9 R	10 L	11 T	12 R
Volume	47	6	221	2	5	6
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	51	6	240	2	5	6
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage				/ No /		
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Delay, Queue Length, and Level of Service

Approach Movement Lane Config	NB	SB	Westbound		Eastbound		
	1 LT	4 L	7 LT	8 R	9 R	10 L	11 LTR
v (vph)	7	180	57		240		13
C(m) (vph)	1056	1065	247		625		258
v/c	0.01	0.17	0.23		0.38		0.05
95% queue length	0.02	0.61	0.87		1.80		0.16
Control Delay	8.4	9.1	23.9		14.3		19.7
LOS	A	A	C		B		C
Approach Delay				16.1			
Approach LOS				C			

HCS+: Unsignalized Intersections Release 5.2

Phone:  
 Fax:  
 E-Mail:

TWO-WAY STOP CONTROL(TWSC) ANALYSIS

Analyst: DK  
 Agency/Co.: RBF  
 Date Performed: 7/23/2007  
 Analysis Time Period: PM Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 Without Project  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: Miraleste Dr  
 Intersection Orientation: NS  
 Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street Movements	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	7	402	69	166	477	2
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	2	109	19	45	130	1
Hourly Flow Rate, HFR	7	436	74	180	518	2
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L	TR	
Upstream Signal?	No			No		

Minor Street Movements	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	47	6	221	2	5	6
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	13	2	60	1	1	2
Hourly Flow Rate, HFR	51	6	240	2	5	6
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage				/ No /		
RT Channelized?	No					
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Pedestrian Volumes and Adjustments

Movements	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)	12.0	12.0	12.0	12.0
Walking Speed (ft/sec)	4.0	4.0	4.0	4.0
Percent Blockage	0	0	0	0

Upstream Signal Data							
	Prog. Flow vph	Sat Flow vph	Arrival Type	Green Time sec	Cycle Length sec	Prog. Speed mph	Distance to Signal feet
S2	Left-Turn						
	Through						
S5	Left-Turn						
	Through						

Worksheet 3-Data for Computing Effect of Delay to Major Street Vehicles

	Movement 2	Movement 5
Shared ln volume, major th vehicles:	436	
Shared ln volume, major rt vehicles:	0	
Sat flow rate, major th vehicles:	1700	
Sat flow rate, major rt vehicles:	1700	
Number of major street through lanes:	1	

Worksheet 4-Critical Gap and Follow-up Time Calculation

Critical Gap Calculation									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
t(c,base)	4.1	4.1	7.1	6.5	6.2	7.1	6.5	6.2	
t(c,hv)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
P(hv)	0	0	0	0	0	0	0	0	
t(c,g)			0.20	0.20	0.10	0.20	0.20	0.10	
Grade/100			0.00	0.00	0.00	0.00	0.00	0.00	
t(3,lt)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
t(c,T): 1-stage	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
2-stage	0.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	
t(c) 1-stage	4.1	4.1	7.1	6.5	6.2	7.1	6.5	6.2	
2-stage	4.1	4.1	6.1	5.5	6.2	6.1	5.5	6.2	

Follow-Up Time Calculations									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
t(f,base)	2.20	2.20	3.50	4.00	3.30	3.50	4.00	3.30	
t(f,HV)	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	
P(HV)	0	0	0	0	0	0	0	0	
t(f)	2.2	2.2	3.5	4.0	3.3	3.5	4.0	3.3	

Worksheet 5-Effect of Upstream Signals

Computation 1-Queue Clearance Time at Upstream Signal				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
V prog				
Total Saturation Flow Rate, s (vph)				
Arrival Type				
Effective Green, g (sec)				
Cycle Length, C (sec)				
Rp (from Exhibit 16-11)				
Proportion vehicles arriving on green P				
g(q1)				
g(q2)				
g(q)				

Computation 2-Proportion of TWSC Intersection Time blocked				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
alpha				
beta				
Travel time, t(a) (sec)				
Smoothing Factor, F				
Proportion of conflicting flow, f				
Max platooned flow, V(c,max)				
Min platooned flow, V(c,min)				
Duration of blocked period, t(p)				
Proportion time blocked, p		0.000		0.000

Computation 3-Platoon Event Periods		Result
p(2)		0.000
p(5)		0.000
p(dom)		
p(subo)		
Constrained or unconstrained?		

Proportion unblocked for minor movements, p(x)	(1)	(2)	(3)
	Single-stage Process	Two-Stage Stage I	Process Stage II
p(1)			
p(4)			
p(7)			
p(8)			
p(9)			
p(10)			
p(11)			
p(12)			

Computation 4 and 5 Single-Stage Process									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
V c,x	520	510	1334	1330	436	1489	1403	519	
s									
Px									
V c,u,x									
C r,x									
C plat,x									

	7		8		10		11	
	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2
V(c,x)	450	884	450	880	879	610	879	524
s		1500		1500		1500		1500
P(x)								
V(c,u,x)								
C(r,x)								
C(plat,x)								

Worksheet 6-Impedance and Capacity Equations

Step 1: RT from Minor St.	9	12
Conflicting Flows	436	519
Potential Capacity	625	561
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	625	561
Probability of Queue free St.	0.62	0.99
Step 2: LT from Major St.	4	1
Conflicting Flows	510	520
Potential Capacity	1065	1056
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	1065	1056
Probability of Queue free St.	0.83	0.99
Maj L-Shared Prob Q free St.		0.99
Step 3: TH from Minor St.	8	11
Conflicting Flows	1330	1403
Potential Capacity	156	141
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.82	0.82
Movement Capacity	128	116
Probability of Queue free St.	0.98	0.98
Step 4: LT from Minor St.	7	10
Conflicting Flows	1334	1489
Potential Capacity	132	103
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.81	0.81
Maj. L, Min T Adj. Imp Factor.	0.85	0.85
Cap. Adj. factor due to Impeding mvmnt	0.84	0.53
Movement Capacity	111	54

Worksheet 7-Computation of the Effect of Two-stage Gap Acceptance

Step 3: TH from Minor St.	8	11
Part 1 - First Stage		
Conflicting Flows	450	879
Potential Capacity	575	368
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.99	0.83
Movement Capacity	570	306
Probability of Queue free St.	0.99	0.98
Part 2 - Second Stage		
Conflicting Flows	880	524
Potential Capacity	368	533
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.83	0.99
Movement Capacity	306	528
Part 3 - Single Stage		
Conflicting Flows	1330	1403
Potential Capacity	156	141
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.82	0.82
Movement Capacity	128	116
Result for 2 stage process:		
a	0.95	0.95

Y	2.58	0.82
C t	268	242
Probability of Queue free St.	0.98	0.98
Step 4: LT from Minor St.	7	10
Part 1 - First Stage		
Conflicting Flows	450	879
Potential Capacity	592	345
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.99	0.83
Movement Capacity	587	287
Part 2 - Second Stage		
Conflicting Flows	884	610
Potential Capacity	343	485
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.81	0.60
Movement Capacity	277	293
Part 3 - Single Stage		
Conflicting Flows	1334	1489
Potential Capacity	132	103
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.81	0.81
Maj. L, Min T Adj. Imp Factor.	0.85	0.85
Cap. Adj. factor due to Impeding mvmnt	0.84	0.53
Movement Capacity	111	54
Results for Two-stage process:		
a	0.95	0.95
Y	2.99	3.95
C t	245	105

Worksheet 8-Shared Lane Calculations

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (vph)	51	6	240	2	5	6
Movement Capacity (vph)	245	268	625	105	242	561
Shared Lane Capacity (vph)	247				258	

Worksheet 9-Computation of Effect of Flared Minor Street Approaches

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
C sep	245	268	625	105	242	561
Volume	51	6	240	2	5	6
Delay						
Q sep						
Q sep +1						
round (Qsep +1)						
n max						
C sh	247				258	
SUM C sep						
n						
C act						

Worksheet 10-Delay, Queue Length, and Level of Service

Movement	1	4	7	8	9	10	11	12
Lane Config	LT	L	LT		R		LTR	
v (vph)	7	180	57		240		13	
C(m) (vph)	1056	1065	247		625		258	
v/c	0.01	0.17	0.23		0.38		0.05	
95% queue length	0.02	0.61	0.87		1.80		0.16	
Control Delay	8.4	9.1	23.9		14.3		19.7	
LOS	A	A	C		B		C	
Approach Delay				16.1			19.7	
Approach LOS				C			C	

Worksheet 11-Shared Major LT Impedance and Delay

	Movement 2	Movement 5
p(oj)	0.99	0.83
v(i1), Volume for stream 2 or 5	436	
v(i2), Volume for stream 3 or 6	0	
s(i1), Saturation flow rate for stream 2 or 5	1700	
s(i2), Saturation flow rate for stream 3 or 6	1700	
P*(oj)	0.99	
d(M,LT), Delay for stream 1 or 4	8.4	9.1
N, Number of major street through lanes	1	
d(rank,1) Delay for stream 2 or 5	0.1	

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF RPV
PM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #6 WESTERN AVENUE/TOSCANINI DRIVE
\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.787
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 64 Level Of Service: C

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module:

Table with 11 columns and 14 rows showing traffic volume data for various approaches and movements.

Saturation Flow Module:

Table with 11 columns and 4 rows showing saturation flow and adjustment factors.

Capacity Analysis Module:

Table with 11 columns and 3 rows showing capacity analysis results.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF RPV
PM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #7 WESTERN AVENUE/CAPITOL DRIVE
\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 1.034
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module:

Table with 11 columns and 14 rows showing traffic volume data for various approaches and movements.

Saturation Flow Module:

Table with 11 columns and 4 rows showing saturation flow and adjustment factors.

Capacity Analysis Module:

Table with 11 columns and 3 rows showing capacity analysis results.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF RPV PM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\* Intersection #8 WESTERN AVENUE/CRESTWOOD STREET \*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.895
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 98 Level Of Service: D

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Lanes.

Volume Module: Table with 12 columns and 12 rows showing traffic volume data for various approaches and movements.

Saturation Flow Module: Table with 12 columns and 4 rows showing saturation flow and adjustment factors.

Capacity Analysis Module: Table with 12 columns and 4 rows showing capacity analysis metrics.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF RPV PM PEAK HOUR

Level Of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

\*\*\*\*\* Intersection #9 WESTERN AVENUE/1ST STREET \*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 1.438
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Lanes.

Volume Module: Table with 12 columns and 12 rows showing traffic volume data for various approaches and movements.

Saturation Flow Module: Table with 12 columns and 4 rows showing saturation flow and adjustment factors.

Capacity Analysis Module: Table with 12 columns and 4 rows showing capacity analysis metrics.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF RPV PM PEAK HOUR

Level Of Service Computation Report Circular 212 Planning Method (Future Volume Alternative)

Intersection #10 WESTERN AVENUE/9TH STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.868
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 152 Level Of Service: D

Table with 4 columns: Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module table with 12 columns for traffic volumes and 12 rows for various metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module table with 12 columns for saturation flow and 4 rows for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module table with 12 columns for capacity analysis and 4 rows for Vol/Sat, Crit Volume, and Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF RPV PM PEAK HOUR

Level Of Service Computation Report Circular 212 Planning Method (Future Volume Alternative)

Intersection #11 WESTERN AVENUE/WEST 25TH STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.805
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 102 Level Of Service: D

Table with 4 columns: Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module table with 12 columns for traffic volumes and 12 rows for various metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module table with 12 columns for saturation flow and 4 rows for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module table with 12 columns for capacity analysis and 4 rows for Vol/Sat, Crit Volume, and Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF RPV
WEEKDAY (11:00-1:00) PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #1 PALOS VERDES DRIVE EAST/MIRALESTE DRIVE
\*\*\*\*\*

Average Delay (sec/veh): 78.2 Worst Case Level Of Service: F[205.6]
\*\*\*\*\*

Table with 5 columns: Approach, North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module table with 12 columns for traffic volume and delay metrics across four approaches.

Critical Gap Module table with 4 columns for gap metrics across four approaches.

Capacity Module table with 4 columns for capacity metrics across four approaches.

Level Of Service Module table with 4 columns for LOS metrics across four approaches.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF RPV
WEEKDAY (11:00-1:00) PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #2 CREST DRIVE - COLLEGE ENTRANCE/PALOS VERDES DRIVE EAST
\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.357
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 27 Level Of Service: A
\*\*\*\*\*

Table with 5 columns: Approach, North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module table with 12 columns for traffic volume and delay metrics across four approaches.

Saturation Flow Module table with 12 columns for saturation flow metrics across four approaches.

Capacity Analysis Module table with 12 columns for capacity analysis metrics across four approaches.

\*\*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF RPV
WEEKDAY (11:00-1:00) PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

\*\*\*\*\*

Intersection #3 PALOS VERDES DRIVE EAST/PALOS VERDES DRIVE SOUTH

\*\*\*\*\*

Average Delay (sec/veh): 2.6 Worst Case Level Of Service: C[ 19.7]

\*\*\*\*\*

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled

Rights: Include Include Include Include

Lanes: 0 0 0 0 0 1 0 0 0 1 1 0 1 0 0 0 0 1 0 1

-----|-----|-----|-----|

Volume Module:

Table with 12 columns: Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume. Rows include various traffic volume and adjustment factors.

-----|-----|-----|-----|

Critical Gap Module:

Critical Gp:xxxxx xxxxx xxxxx 6.4 xxxxx 6.2 4.1 xxxxx xxxxxx xxxxx xxxxx xxxxxx

FollowUpTim:xxxxx xxxxx xxxxx 3.5 xxxxx 3.3 2.2 xxxxx xxxxxx xxxxx xxxxx xxxxxx

-----|-----|-----|-----|

Capacity Module:

Cnflct Vol: xxxxx xxxxx xxxxxx 1227 xxxxx 508 536 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Potent Cap.: xxxxx xxxxx xxxxxx 199 xxxxx 569 1042 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Move Cap.: xxxxx xxxxx xxxxxx 189 xxxxx 569 1042 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Volume/Cap: xxxxx xxxxx xxxxx 0.29 xxxxx 0.16 0.07 xxxxx xxxxx xxxxx xxxxx xxxxxx

-----|-----|-----|-----|

Level Of Service Module:

2Way95thQ: xxxxx xxxxx xxxxxx 1.2 xxxxx 0.6 0.2 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Control Del:xxxxxx xxxxx xxxxxx 31.8 xxxxx 12.6 8.7 xxxxx xxxxxx xxxxxx xxxxx xxxxxx

LOS by Move: \* \* \* D \* B A \* \* \* \* \* \*

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx

SharedQueue:xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx

Shrd ConDel:xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx

Shared LOS: \* \* \* \* \* \* \* \* \* \* \* \* \* \* \* \*

ApproachDel: xxxxxxx 19.7 xxxxxxx xxxxxxx

ApproachLOS: \* C \*

\*\*\*\*\*

Note: Queue reported is the number of cars per lane.

\*\*\*\*\*

HCS+: Unsignalized Intersections Release 5.2

TWO-WAY STOP CONTROL SUMMARY

Analyst: DK  
 Agency/Co.: RBF  
 Date Performed: 7/23/2007  
 Analysis Time Period: Weekday Mid-day Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 Without Project  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: Miraleste Dr  
 Intersection Orientation: NS  
 Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	Northbound			Southbound		
	1 L	2 T	3 R	4 L	5 T	6 R
Volume	6	274	53	184	330	7
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	6	297	57	199	358	7
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L	TR	
Upstream Signal?	No			No		

Minor Street: Approach Movement	Westbound			Eastbound		
	7 L	8 T	9 R	10 L	11 T	12 R
Volume	36	18	223	4	3	6
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	39	19	242	4	3	6
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage	/			No /		
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Delay, Queue Length, and Level of Service

Approach Movement Lane Config	NB	SB	Westbound		Eastbound		
	1	4	7	8	9	10	11 12
	LT	L	LT		R		LTR
v (vph)	6	199	58		242		13
C(m) (vph)	1205	1216	302		747		298
v/c	0.00	0.16	0.19		0.32		0.04
95% queue length	0.02	0.58	0.70		1.41		0.14
Control Delay	8.0	8.5	19.7		12.1		17.6
LOS	A	A	C		B		C
Approach Delay				13.6			17.6
Approach LOS				B			C

HCS+: Unsignalized Intersections Release 5.2

Phone:  
 E-Mail:

Fax:

TWO-WAY STOP CONTROL(TWSC) ANALYSIS

Analyst: DK  
 Agency/Co.: RBF  
 Date Performed: 7/23/2007  
 Analysis Time Period: Weekday Mid-day Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 Without Project  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: Miraleste Dr  
 Intersection Orientation: NS  
 Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street Movements	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	6	274	53	184	330	7
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	2	74	14	50	90	2
Hourly Flow Rate, HFR	6	297	57	199	358	7
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L	TR	
Upstream Signal?	No			No		

Minor Street Movements	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	36	18	223	4	3	6
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	10	5	61	1	1	2
Hourly Flow Rate, HFR	39	19	242	4	3	6
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage	/			No /		
RT Channelized?	No					
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Pedestrian Volumes and Adjustments

Movements	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)	12.0	12.0	12.0	12.0
Walking Speed (ft/sec)	4.0	4.0	4.0	4.0
Percent Blockage	0	0	0	0

Upstream Signal Data							
	Prog. Flow vph	Sat Flow vph	Arrival Type	Green Time sec	Cycle Length sec	Prog. Speed mph	Distance to Signal feet
S2	Left-Turn						
	Through						
S5	Left-Turn						
	Through						

Worksheet 3-Data for Computing Effect of Delay to Major Street Vehicles

	Movement 2	Movement 5
Shared ln volume, major th vehicles:	297	
Shared ln volume, major rt vehicles:	0	
Sat flow rate, major th vehicles:	1700	
Sat flow rate, major rt vehicles:	1700	
Number of major street through lanes:	1	

Worksheet 4-Critical Gap and Follow-up Time Calculation

Critical Gap Calculation									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
t(c,base)	4.1	4.1	7.1	6.5	6.2	7.1	6.5	6.2	
t(c,hv)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
P(hv)	0	0	0	0	0	0	0	0	
t(c,g)			0.20	0.20	0.10	0.20	0.20	0.10	
Grade/100			0.00	0.00	0.00	0.00	0.00	0.00	
t(3,lt)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
t(c,T): 1-stage	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
2-stage	0.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	
t(c) 1-stage	4.1	4.1	7.1	6.5	6.2	7.1	6.5	6.2	
2-stage	4.1	4.1	6.1	5.5	6.2	6.1	5.5	6.2	

Follow-Up Time Calculations									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
t(f,base)	2.20	2.20	3.50	4.00	3.30	3.50	4.00	3.30	
t(f,HV)	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	
P(HV)	0	0	0	0	0	0	0	0	
t(f)	2.2	2.2	3.5	4.0	3.3	3.5	4.0	3.3	

Worksheet 5-Effect of Upstream Signals

Computation 1-Queue Clearance Time at Upstream Signal				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
V prog				
Total Saturation Flow Rate, s (vph)				
Arrival Type				
Effective Green, g (sec)				
Cycle Length, C (sec)				
Rp (from Exhibit 16-11)				
Proportion vehicles arriving on green P				
g(q1)				
g(q2)				
g(q)				

Computation 2-Proportion of TWSC Intersection Time blocked				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
alpha				
beta				
Travel time, t(a) (sec)				
Smoothing Factor, F				
Proportion of conflicting flow, f				
Max platooned flow, V(c,max)				
Min platooned flow, V(c,min)				
Duration of blocked period, t(p)				
Proportion time blocked, p		0.000		0.000

Computation 3-Platoon Event Periods		Result
p(2)		0.000
p(5)		0.000
p(dom)		
p(subo)		
Constrained or unconstrained?		

Proportion unblocked for minor movements, p(x)	(1)	(2)	(3)
	Single-stage Process	Two-Stage Stage I	Process Stage II
p(1)			
p(4)			
p(7)			
p(8)			
p(9)			
p(10)			
p(11)			
p(12)			

Computation 4 and 5 Single-Stage Process									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
V c,x	365	354	1073	1072	297	1228	1126	362	
s									
Px									
V c,u,x									
C r,x									
C plat,x									

	Two-Stage Process							
	7		8		10		11	
	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2
V(c,x)	309	764	309	763	760	468	760	366
s		1500		1500		1500		1500
P(x)								
V(c,u,x)								
C(r,x)								
C(plat,x)								

Worksheet 6-Impedance and Capacity Equations

Step 1: RT from Minor St.	9	12
Conflicting Flows	297	362
Potential Capacity	747	687
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	747	687
Probability of Queue free St.	0.68	0.99
Step 2: LT from Major St.	4	1
Conflicting Flows	354	365
Potential Capacity	1216	1205
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	1216	1205
Probability of Queue free St.	0.84	1.00
Maj L-Shared Prob Q free St.		0.99
Step 3: TH from Minor St.	8	11
Conflicting Flows	1072	1126
Potential Capacity	222	207
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.83	0.83
Movement Capacity	185	172
Probability of Queue free St.	0.95	0.99
Step 4: LT from Minor St.	7	10
Conflicting Flows	1073	1228
Potential Capacity	200	156
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.82	0.79
Maj. L, Min T Adj. Imp Factor.	0.86	0.84
Cap. Adj. factor due to Impeding mvmnt	0.86	0.57
Movement Capacity	171	89

Worksheet 7-Computation of the Effect of Two-stage Gap Acceptance

Step 3: TH from Minor St.	8	11
Part 1 - First Stage		
Conflicting Flows	309	760
Potential Capacity	663	417
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.99	0.84
Movement Capacity	659	349
Probability of Queue free St.	0.97	0.99
Part 2 - Second Stage		
Conflicting Flows	763	366
Potential Capacity	416	626
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.84	0.99
Movement Capacity	348	622
Part 3 - Single Stage		
Conflicting Flows	1072	1126
Potential Capacity	222	207
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.83	0.83
Movement Capacity	185	172
Result for 2 stage process:		
a	0.95	0.95

Y	3.02	0.71
C t	313	293
Probability of Queue free St.	0.95	0.99
Step 4: LT from Minor St.	7	10
Part 1 - First Stage		
Conflicting Flows	309	760
Potential Capacity	705	401
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.99	0.84
Movement Capacity	701	335
Part 2 - Second Stage		
Conflicting Flows	764	468
Potential Capacity	399	579
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.82	0.65
Movement Capacity	328	378
Part 3 - Single Stage		
Conflicting Flows	1073	1228
Potential Capacity	200	156
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.82	0.79
Maj. L, Min T Adj. Imp Factor.	0.86	0.84
Cap. Adj. factor due to Impeding mvmnt	0.86	0.57
Movement Capacity	171	89
Results for Two-stage process:		
a	0.95	0.95
Y	3.51	2.73
C t	297	162

Worksheet 8-Shared Lane Calculations

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (vph)	39	19	242	4	3	6
Movement Capacity (vph)	297	313	747	162	293	687
Shared Lane Capacity (vph)	302				298	

Worksheet 9-Computation of Effect of Flared Minor Street Approaches

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
C sep	297	313	747	162	293	687
Volume	39	19	242	4	3	6
Delay						
Q sep						
Q sep +1						
round (Qsep +1)						
n max						
C sh	302				298	
SUM C sep						
n						
C act						

Worksheet 10-Delay, Queue Length, and Level of Service

Movement	1	4	7	8	9	10	11	12
Lane Config	LT	L	LT		R		LTR	
v (vph)	6	199	58		242		13	
C(m) (vph)	1205	1216	302		747		298	
v/c	0.00	0.16	0.19		0.32		0.04	
95% queue length	0.02	0.58	0.70		1.41		0.14	
Control Delay	8.0	8.5	19.7		12.1		17.6	
LOS	A	A	C		B		C	
Approach Delay				13.6			17.6	
Approach LOS				B			C	

Worksheet 11-Shared Major LT Impedance and Delay

	Movement 2	Movement 5
p(oj)	1.00	0.84
v(i1), Volume for stream 2 or 5	297	
v(i2), Volume for stream 3 or 6	0	
s(i1), Saturation flow rate for stream 2 or 5	1700	
s(i2), Saturation flow rate for stream 3 or 6	1700	
P*(oj)	0.99	
d(M,LT), Delay for stream 1 or 4	8.0	8.5
N, Number of major street through lanes	1	
d(rank,1) Delay for stream 2 or 5	0.0	



MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF RPV
WEEKDAY (2:00-4:00) PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

\*\*\*\*\*

Intersection #3 PALOS VERDES DRIVE EAST/PALOS VERDES DRIVE SOUTH

\*\*\*\*\*

Average Delay (sec/veh): 6.4 Worst Case Level Of Service: E[ 46.4]

\*\*\*\*\*

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module:

Table with 12 columns representing different traffic volumes and adjustment factors like Base Vol, Growth Adj, PHF Adj, etc.

Critical Gap Module:

Table with 12 columns showing critical gap and follow-up time values for different movements.

Capacity Module:

Table with 12 columns showing capacity-related metrics like Cnflct Vol, Potent Cap., Move Cap., etc.

Level Of Service Module:

Table with 12 columns showing level of service metrics like 2Way95thQ, Control Del, Shared Queue, etc.

Note: Queue reported is the number of cars per lane.

\*\*\*\*\*

HCS+: Unsignalized Intersections Release 5.2

TWO-WAY STOP CONTROL SUMMARY

Analyst: DK  
 Agency/Co.: RBF  
 Date Performed: 7/23/2007  
 Analysis Time Period: Afternoon Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 Without Project  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: Miraleste  
 Intersection Orientation: NS  
 Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	Northbound			Southbound		
	1 L	2 T	3 R	4 L	5 T	6 R
Volume	11	294	42	267	413	5
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	11	319	45	290	448	5
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L		TR
Upstream Signal?	No			No		

Minor Street: Approach Movement	Westbound			Eastbound		
	7 L	8 T	9 R	10 L	11 T	12 R
Volume	52	20	212	1	2	14
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	56	21	230	1	2	15
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage				/ No /		
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Delay, Queue Length, and Level of Service

Approach Movement Lane Config	NB	SB	Westbound		Eastbound		
	1 LT	4 L	7 LT	8 R	9 R	10 L	11 LTR
v (vph)	11	290	77		230		18
C(m) (vph)	1118	1206	190		726		374
v/c	0.01	0.24	0.41		0.32		0.05
95% queue length	0.03	0.94	1.81		1.36		0.15
Control Delay	8.3	8.9	36.3		12.2		15.1
LOS	A	A	E		B		C
Approach Delay				18.3			15.1
Approach LOS				C			C

HCS+: Unsignalized Intersections Release 5.2

Phone:  
 E-Mail:  
 Fax:

TWO-WAY STOP CONTROL(TWSC) ANALYSIS

Analyst: DK  
 Agency/Co.: RBF  
 Date Performed: 7/23/2007  
 Analysis Time Period: Afternoon Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 Without Project  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: Miraleste  
 Intersection Orientation: NS  
 Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street Movements	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	11	294	42	267	413	5
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	3	80	11	73	112	1
Hourly Flow Rate, HFR	11	319	45	290	448	5
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L		TR
Upstream Signal?	No			No		

Minor Street Movements	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	52	20	212	1	2	14
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	14	5	58	0	1	4
Hourly Flow Rate, HFR	56	21	230	1	2	15
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage				/ No /		
RT Channelized?	No					
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Pedestrian Volumes and Adjustments

Movements	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)	12.0	12.0	12.0	12.0
Walking Speed (ft/sec)	4.0	4.0	4.0	4.0
Percent Blockage	0	0	0	0

Upstream Signal Data							
	Prog. Flow vph	Sat Flow vph	Arrival Type	Green Time sec	Cycle Length sec	Prog. Speed mph	Distance to Signal feet
S2	Left-Turn						
	Through						
S5	Left-Turn						
	Through						

Worksheet 3-Data for Computing Effect of Delay to Major Street Vehicles

	Movement 2	Movement 5
Shared ln volume, major th vehicles:	319	
Shared ln volume, major rt vehicles:	0	
Sat flow rate, major th vehicles:	1700	
Sat flow rate, major rt vehicles:	1700	
Number of major street through lanes:	1	

Worksheet 4-Critical Gap and Follow-up Time Calculation

Critical Gap Calculation									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
t(c,base)	4.1	4.1	7.1	6.5	6.2	7.1	6.5	6.2	
t(c,hv)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
P(hv)	0	0	0	0	0	0	0	0	
t(c,g)			0.20	0.20	0.10	0.20	0.20	0.10	
Grade/100			0.00	0.00	0.00	0.00	0.00	0.00	
t(3,lt)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
t(c,T):	1-stage	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
	2-stage	0.00	0.00	1.00	1.00	0.00	1.00	1.00	
t(c)	1-stage	4.1	4.1	7.1	6.5	7.1	6.5	6.2	
	2-stage	4.1	4.1	6.1	5.5	6.2	6.1	5.5	

Follow-Up Time Calculations									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
t(f,base)	2.20	2.20	3.50	4.00	3.30	3.50	4.00	3.30	
t(f,HV)	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	
P(HV)	0	0	0	0	0	0	0	0	
t(f)	2.2	2.2	3.5	4.0	3.3	3.5	4.0	3.3	

Worksheet 5-Effect of Upstream Signals

Computation 1-Queue Clearance Time at Upstream Signal				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)

V prog  
 Total Saturation Flow Rate, s (vph)  
 Arrival Type  
 Effective Green, g (sec)  
 Cycle Length, C (sec)  
 Rp (from Exhibit 16-11)  
 Proportion vehicles arriving on green P  
 g(q1)  
 g(q2)  
 g(q)

Computation 2-Proportion of TWSC Intersection Time blocked				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)

alpha  
 beta  
 Travel time, t(a) (sec)  
 Smoothing Factor, F  
 Proportion of conflicting flow, f  
 Max platooned flow, V(c,max)  
 Min platooned flow, V(c,min)  
 Duration of blocked period, t(p)  
 Proportion time blocked, p

Computation 3-Platoon Event Periods	Result
-------------------------------------	--------

p(2) 0.000  
 p(5) 0.000  
 p(dom)  
 p(subo)  
 Constrained or unconstrained?

Proportion unblocked for minor movements, p(x)	(1)	(2)	(3)
	Single-stage Process	Two-Stage Stage I	Process Stage II

p(1)  
 p(4)  
 p(7)  
 p(8)  
 p(9)  
 p(10)  
 p(11)  
 p(12)

Computation 4 and 5 Single-Stage Process									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	

V c,x  
 s  
 Px  
 V c,u,x  
 C r,x  
 C plat,x

	7		8		10		11	
	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2

V(c,x) 341 1039 341 1033 1030 489 1030 386  
 s 1500 1500 1500  
 P(x)  
 V(c,u,x)

C(r,x)  
 C(plat,x)

Worksheet 6-Impedance and Capacity Equations

Step 1: RT from Minor St.	9	12
Conflicting Flows	319	450
Potential Capacity	726	613
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	726	613
Probability of Queue free St.	0.68	0.98
Step 2: LT from Major St.	4	1
Conflicting Flows	364	453
Potential Capacity	1206	1118
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	1206	1118
Probability of Queue free St.	0.76	0.99
Maj L-Shared Prob Q free St.		0.99
Step 3: TH from Minor St.	8	11
Conflicting Flows	1374	1416
Potential Capacity	147	139
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.75	0.75
Movement Capacity	110	104
Probability of Queue free St.	0.93	0.99
Step 4: LT from Minor St.	7	10
Conflicting Flows	1380	1519
Potential Capacity	123	98
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.74	0.70
Maj. L, Min T Adj. Imp Factor.	0.80	0.77
Cap. Adj. factor due to Impeding mvmnt	0.78	0.52
Movement Capacity	96	51

Worksheet 7-Computation of the Effect of Two-stage Gap Acceptance

Step 3: TH from Minor St.	8	11
Part 1 - First Stage		
Conflicting Flows	341	1030
Potential Capacity	642	313
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.99	0.76
Movement Capacity	634	238
Probability of Queue free St.	0.97	0.99
Part 2 - Second Stage		
Conflicting Flows	1033	386
Potential Capacity	312	614
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.76	0.99
Movement Capacity	237	607
Part 3 - Single Stage		
Conflicting Flows	1374	1416
Potential Capacity	147	139
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.75	0.75
Movement Capacity	110	104
Result for 2 stage process:		
a	0.95	0.95

Y	4.52	0.63
C t	210	201
Probability of Queue free St.	0.93	0.99
Step 4: LT from Minor St.	7	10
Part 1 - First Stage		
Conflicting Flows	341	1030
Potential Capacity	678	284
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.99	0.76
Movement Capacity	670	216
Part 2 - Second Stage		
Conflicting Flows	1039	489
Potential Capacity	281	564
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.73	0.65
Movement Capacity	206	368
Part 3 - Single Stage		
Conflicting Flows	1380	1519
Potential Capacity	123	98
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.74	0.70
Maj. L, Min T Adj. Imp Factor.	0.80	0.77
Cap. Adj. factor due to Impeding mvmnt	0.78	0.52
Movement Capacity	96	51
Results for Two-stage process:		
a	0.95	0.95
Y	5.80	6.11
C t	183	73

Worksheet 8-Shared Lane Calculations

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (vph)	56	21	230	1	2	15
Movement Capacity (vph)	183	210	726	73	201	613
Shared Lane Capacity (vph)	190				374	

Worksheet 9-Computation of Effect of Flared Minor Street Approaches

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
C sep	183	210	726	73	201	613
Volume	56	21	230	1	2	15
Delay						
Q sep						
Q sep +1						
round (Qsep +1)						
n max						
C sh	190				374	
SUM C sep						
n						
C act						

Worksheet 10-Delay, Queue Length, and Level of Service

Movement	1	4	7	8	9	10	11	12
Lane Config	LT	L	LT		R		LTR	
v (vph)	11	290	77		230		18	
C(m) (vph)	1118	1206	190		726		374	
v/c	0.01	0.24	0.41		0.32		0.05	
95% queue length	0.03	0.94	1.81		1.36		0.15	
Control Delay	8.3	8.9	36.3		12.2		15.1	
LOS	A	A	E		B		C	
Approach Delay				18.3			15.1	
Approach LOS				C			C	

Worksheet 11-Shared Major LT Impedance and Delay

	Movement 2	Movement 5
p(oj)	0.99	0.76
v(i1), Volume for stream 2 or 5	319	
v(i2), Volume for stream 3 or 6	0	
s(i1), Saturation flow rate for stream 2 or 5	1700	
s(i2), Saturation flow rate for stream 3 or 6	1700	
P*(oj)	0.99	
d(M,LT), Delay for stream 1 or 4	8.3	8.9
N, Number of major street through lanes	1	
d(rank,1) Delay for stream 2 or 5	0.1	

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF RPV
SATURDAY (11:00-1:00) PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #1 PALOS VERDES DRIVE EAST/MIRALESTE DRIVE
\*\*\*\*\*

Average Delay (sec/veh): 15.2 Worst Case Level Of Service: E[ 36.5]

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include
Lanes: 0 0 0 1 0 1 0 1 0 0 0 0 0 0 0 0 1 0 0 0 1

Volume Module:

Base Vol: 0 161 123 296 158 0 0 0 0 0 127 0 259
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 0 161 123 296 158 0 0 0 0 0 127 0 259
Added Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 0
Cumulative: 0 3 17 17 1 0 0 0 0 0 17 0 17
Initial Fut: 0 164 140 313 159 0 0 0 0 0 144 0 276
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 0 178 152 340 173 0 0 0 0 0 157 0 300
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 0 178 152 340 173 0 0 0 0 0 157 0 300

Critical Gap Module:

Critical Gap:xxxxx xxxx xxxxx 4.1 xxxx xxxxxx xxxxxx xxxx xxxxxx 6.4 xxxx 6.2
FollowUpTim:xxxxx xxxx xxxxxx 2.2 xxxx xxxxxx xxxxxx xxxx xxxxxx 3.5 xxxx 3.3

Capacity Module:

Cnflct Vol: xxxx xxxx xxxxxx 330 xxxx xxxxxx xxxx xxxx xxxxxx 1108 xxxx 254
Potent Cap.: xxxx xxxx xxxxxx 1240 xxxx xxxxxx xxxx xxxx xxxxxx 234 xxxx 789
Move Cap.: xxxx xxxx xxxxxx 1240 xxxx xxxxxx xxxx xxxx xxxxxx 185 xxxx 789
Volume/Cap: xxxx xxxx xxxxxx 0.27 xxxx xxxxxx xxxx xxxx xxxxxx 0.85 xxxx 0.38

Level Of Service Module:

2Way95thQ: xxxx xxxx xxxxxx 1.1 xxxx xxxxxx xxxx xxxx xxxxxx 6.1 xxxx 1.8
Control Del:xxxxx xxxx xxxxxx 9.0 xxxx xxxxxx xxxxxx xxxx xxxxxx 82.9 xxxx 12.3

LOS by Move: \* \* \* A \* \* \* \* \* F \* \* B
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxxx xxxx xxxxxx xxxx xxxx xxxxxx xxxx xxxx xxxxxx xxxx xxxx xxxxxx
SharedQueue:xxxxx xxxx xxxxxx xxxxxx xxxx xxxxxx xxxxxx xxxx xxxxxx xxxxxx xxxx xxxxxx
Shrd ConDel:xxxxx xxxx xxxxxx xxxxxx xxxx xxxxxx xxxxxx xxxx xxxxxx xxxxxx xxxx xxxxxx

Shared LOS: \*
ApproachDel: xxxxxx xxxxxx xxxxxx xxxxxx 36.5
ApproachLOS: \* \* \* \* \* E

\*\*\*\*\*
Note: Queue reported is the number of cars per lane.
\*\*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF RPV
SATURDAY (11:00-1:00) PEAK HOUR

Level Of Service Computation Report

ICU l(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #2 CREST DRIVE - COLLEGE ENTRANCE/PALOS VERDES DRIVE EAST
\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.264

Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxxx

Optimal Cycle: 24 Level Of Service: A

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Permitted Permitted Permitted Permitted
Rights: Include Include Include Include
Lanes: 0 0 0 1 0 0 1 0 0 1 1 0 1 0 1 1 0 1 0 1

Volume Module:

Base Vol: 0 3 4 65 0 17 19 143 3 1 106 53
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 0 3 4 65 0 17 19 143 3 1 106 53
Added Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Cumulative: 0 0 0 17 0 0 0 8 0 0 6 17
Initial Fut: 0 3 4 82 0 17 19 151 3 1 112 70
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 0 3 4 89 0 18 21 164 3 1 122 76
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 0 3 4 89 0 18 21 164 3 1 122 76
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 0 3 4 89 0 18 21 164 3 1 122 76

Saturation Flow Module:

Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 0.00 0.43 0.57 1.00 0.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Final Sat.: 0 686 914 1600 0 1600 1600 1600 1600 1600 1600 1600

Capacity Analysis Module:

Vol/Sat: 0.00 0.00 0.00 0.06 0.00 0.01 0.01 0.10 0.00 0.00 0.08 0.05
Crit Moves: \*\*\*\* \*\*\*\* \*\*\*\* \*\*\*\*

\*\*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF RPV
SATURDAY (11:00-1:00) PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

\*\*\*\*\*

Intersection #3 PALOS VERDES DRIVE EAST/PALOS VERDES DRIVE SOUTH

\*\*\*\*\*

Average Delay (sec/veh): 2.7 Worst Case Level Of Service: D[ 31.8]

\*\*\*\*\*

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled

Rights: Include Include Include Include

Lanes: 0 0 0 0 0 1 0 0 0 1 1 0 1 0 0 0 0 1 0 1

-----|-----|-----|-----|

Volume Module:

Table with 16 columns and 11 rows showing traffic volume metrics: Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume.

-----|-----|-----|-----|

Critical Gap Module:

Critical Gp:xxxxx xxxxx xxxxx 6.4 xxxxx 6.2 4.1 xxxxx xxxxxx xxxxx xxxxx xxxxxx

FollowUpTim:xxxxx xxxxx xxxxx 3.5 xxxxx 3.3 2.2 xxxxx xxxxxx xxxxx xxxxx xxxxxx

-----|-----|-----|-----|

Capacity Module:

Cnflct Vol: xxxxx xxxxx xxxxxx 1749 xxxxx 800 863 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Potent Cap.: xxxxx xxxxx xxxxxx 96 xxxxx 388 788 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Move Cap.: xxxxx xxxxx xxxxxx 87 xxxxx 388 788 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Volume/Cap: xxxxx xxxxx xxxxx 0.39 xxxxx 0.23 0.12 xxxxx xxxxx xxxxx xxxxx xxxxxx

-----|-----|-----|-----|

Level Of Service Module:

2Way95thQ: xxxxx xxxxx xxxxxx 1.5 xxxxx 0.9 0.4 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Control Del:xxxxxx xxxxx xxxxxx 71.0 xxxxx 17.0 10.2 xxxxx xxxxxx xxxxx xxxxx xxxxxx

LOS by Move: \* \* \* F \* C B \* \* \* \* \* \*

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx

SharedQueue:xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx

Shrd ConDel:xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx

Shared LOS: \* \* \* \* \* \* \* \* \* \* \* \* \* \* \* \*

ApproachDel: xxxxxxx 31.8 xxxxxxx xxxxxxx

ApproachLOS: \* D \* \*

\*\*\*\*\*

Note: Queue reported is the number of cars per lane.

\*\*\*\*\*

HCS+: Unsignalized Intersections Release 5.2

TWO-WAY STOP CONTROL SUMMARY

Analyst: DK  
 Agency/Co.: RBF  
 Date Performed: 7/23/2007  
 Analysis Time Period: Saturday Mid-day Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 Without Project  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: Miraleste Dr  
 Intersection Orientation: NS  
 Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	Northbound			Southbound		
	1 L	2 T	3 R	4 L	5 T	6 R
Volume	3	276	51	193	292	7
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	3	299	55	209	317	7
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L		TR
Upstream Signal?	No			No		

Minor Street: Approach Movement	Westbound			Eastbound		
	7 L	8 T	9 R	10 L	11 T	12 R
Volume	0	8	136	0	4	0
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	0	8	147	0	4	0
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage				/ No /		
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Delay, Queue Length, and Level of Service

Approach Movement Lane Config	NB	SB	Westbound		Eastbound			
	1 LT	4 L	7 LT	8 R	9 R	10 L	11 L	12 R
v (vph)	3	209	8		147		4	
C(m) (vph)	1247	1216	320		745		297	
v/c	0.00	0.17	0.03		0.20		0.01	
95% queue length	0.01	0.62	0.08		0.73		0.04	
Control Delay	7.9	8.6	16.5		11.0		17.3	
LOS	A	A	C		B		C	
Approach Delay				11.3				
Approach LOS				B				

HCS+: Unsignalized Intersections Release 5.2

Phone:  
 Fax:  
 E-Mail:

TWO-WAY STOP CONTROL(TWSC) ANALYSIS

Analyst: DK  
 Agency/Co.: RBF  
 Date Performed: 7/23/2007  
 Analysis Time Period: Saturday Mid-day Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 Without Project  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: Miraleste Dr  
 Intersection Orientation: NS  
 Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street Movements	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	3	276	51	193	292	7
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	1	75	14	52	79	2
Hourly Flow Rate, HFR	3	299	55	209	317	7
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L		TR
Upstream Signal?	No			No		

Minor Street Movements	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	0	8	136	0	4	0
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	0	2	37	0	1	0
Hourly Flow Rate, HFR	0	8	147	0	4	0
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage				/ No /		
RT Channelized?	No					
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Pedestrian Volumes and Adjustments

Movements	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)	12.0	12.0	12.0	12.0
Walking Speed (ft/sec)	4.0	4.0	4.0	4.0
Percent Blockage	0	0	0	0

Upstream Signal Data							
	Prog. Flow vph	Sat Flow vph	Arrival Type	Green Time sec	Cycle Length sec	Prog. Speed mph	Distance to Signal feet
S2	Left-Turn						
	Through						
S5	Left-Turn						
	Through						

Worksheet 3-Data for Computing Effect of Delay to Major Street Vehicles

	Movement 2	Movement 5
Shared ln volume, major th vehicles:	299	
Shared ln volume, major rt vehicles:	0	
Sat flow rate, major th vehicles:	1700	
Sat flow rate, major rt vehicles:	1700	
Number of major street through lanes:	1	

Worksheet 4-Critical Gap and Follow-up Time Calculation

Critical Gap Calculation									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
t(c,base)	4.1	4.1	7.1	6.5	6.2	7.1	6.5	6.2	
t(c,hv)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
P(hv)	0	0	0	0	0	0	0	0	
t(c,g)			0.20	0.20	0.10	0.20	0.20	0.10	
Grade/100			0.00	0.00	0.00	0.00	0.00	0.00	
t(3,lt)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
t(c,T):	1-stage	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
	2-stage	0.00	0.00	1.00	1.00	0.00	1.00	1.00	
t(c)	1-stage	4.1	4.1	7.1	6.5	7.1	6.5	6.2	
	2-stage	4.1	4.1	6.1	5.5	6.1	5.5	6.2	

Follow-Up Time Calculations									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
t(f,base)	2.20	2.20	3.50	4.00	3.30	3.50	4.00	3.30	
t(f,HV)	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	
P(HV)	0	0	0	0	0	0	0	0	
t(f)	2.2	2.2	3.5	4.0	3.3	3.5	4.0	3.3	

Worksheet 5-Effect of Upstream Signals

Computation 1-Queue Clearance Time at Upstream Signal				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)

V prog  
 Total Saturation Flow Rate, s (vph)  
 Arrival Type  
 Effective Green, g (sec)  
 Cycle Length, C (sec)  
 Rp (from Exhibit 16-11)  
 Proportion vehicles arriving on green P  
 g(q1)  
 g(q2)  
 g(q)

Computation 2-Proportion of TWSC Intersection Time blocked				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)

alpha  
 beta  
 Travel time, t(a) (sec)  
 Smoothing Factor, F  
 Proportion of conflicting flow, f  
 Max platooned flow, V(c,max)  
 Min platooned flow, V(c,min)  
 Duration of blocked period, t(p)  
 Proportion time blocked, p

Computation 3-Platoon Event Periods	Result
-------------------------------------	--------

p(2) 0.000  
 p(5) 0.000  
 p(dom)  
 p(subo)  
 Constrained or unconstrained?

Proportion unblocked for minor movements, p(x)	(1)	(2)	(3)
	Single-stage Process	Two-Stage Stage I	Process Stage II

p(1)  
 p(4)  
 p(7)  
 p(8)  
 p(9)  
 p(10)  
 p(11)  
 p(12)

Computation 4 and 5 Single-Stage Process									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	

V c,x 324 354 1045 1047 299 1148 1098 320  
 s  
 Px  
 V c,u,x

C r,x  
 C plat,x

Two-Stage Process	7		8		10		11	
	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2
V(c,x)	305	740	305	742	738	410	738	360
s		1500		1500		1500		1500
P(x)								
V(c,u,x)								

C(r,x)  
 C(plat,x)

Worksheet 6-Impedance and Capacity Equations

Step 1: RT from Minor St.	9	12
Conflicting Flows	299	320
Potential Capacity	745	725
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	745	725
Probability of Queue free St.	0.80	1.00
Step 2: LT from Major St.	4	1
Conflicting Flows	354	324
Potential Capacity	1216	1247
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	1216	1247
Probability of Queue free St.	0.83	1.00
Maj L-Shared Prob Q free St.		1.00
Step 3: TH from Minor St.	8	11
Conflicting Flows	1047	1098
Potential Capacity	230	215
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.83	0.83
Movement Capacity	190	178
Probability of Queue free St.	0.98	0.99
Step 4: LT from Minor St.	7	10
Conflicting Flows	1045	1148
Potential Capacity	209	177
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.81	0.81
Maj. L, Min T Adj. Imp Factor.	0.86	0.85
Cap. Adj. factor due to Impeding mvmnt	0.86	0.68
Movement Capacity	179	121

Worksheet 7-Computation of the Effect of Two-stage Gap Acceptance

Step 3: TH from Minor St.	8	11
Part 1 - First Stage		
Conflicting Flows	305	738
Potential Capacity	666	427
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	1.00	0.83
Movement Capacity	664	354
Probability of Queue free St.	0.99	0.99
Part 2 - Second Stage		
Conflicting Flows	742	360
Potential Capacity	425	630
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.83	1.00
Movement Capacity	352	628
Part 3 - Single Stage		
Conflicting Flows	1047	1098
Potential Capacity	230	215
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.83	0.83
Movement Capacity	190	178
Result for 2 stage process:		
a	0.95	0.95

Y	2.98	0.73
C t	320	297
Probability of Queue free St.	0.98	0.99
Step 4: LT from Minor St.	7	10
Part 1 - First Stage		
Conflicting Flows	305	738
Potential Capacity	709	413
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	1.00	0.83
Movement Capacity	707	342
Part 2 - Second Stage		
Conflicting Flows	740	410
Potential Capacity	412	623
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.82	0.79
Movement Capacity	337	493
Part 3 - Single Stage		
Conflicting Flows	1045	1148
Potential Capacity	209	177
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.81	0.81
Maj. L, Min T Adj. Imp Factor.	0.86	0.85
Cap. Adj. factor due to Impeding mvmnt	0.86	0.68
Movement Capacity	179	121
Results for Two-stage process:		
a	0.95	0.95
Y	3.41	1.36
C t	308	233

Worksheet 8-Shared Lane Calculations

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (vph)	0	8	147	0	4	0
Movement Capacity (vph)	308	320	745	233	297	725
Shared Lane Capacity (vph)	320				297	

Worksheet 9-Computation of Effect of Flared Minor Street Approaches

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
C sep	308	320	745	233	297	725
Volume	0	8	147	0	4	0
Delay						
Q sep						
Q sep +1						
round (Qsep +1)						
n max						
C sh	320				297	
SUM C sep						
n						
C act						

Worksheet 10-Delay, Queue Length, and Level of Service

Movement	1	4	7	8	9	10	11	12
Lane Config	LT	L	LT		R		LTR	
v (vph)	3	209	8		147		4	
C(m) (vph)	1247	1216	320		745		297	
v/c	0.00	0.17	0.03		0.20		0.01	
95% queue length	0.01	0.62	0.08		0.73		0.04	
Control Delay	7.9	8.6	16.5		11.0		17.3	
LOS	A	A	C		B		C	
Approach Delay				11.3			17.3	
Approach LOS				B			C	

Worksheet 11-Shared Major LT Impedance and Delay

	Movement 2	Movement 5
p(oj)	1.00	0.83
v(i1), Volume for stream 2 or 5	299	
v(i2), Volume for stream 3 or 6	0	
s(i1), Saturation flow rate for stream 2 or 5	1700	
s(i2), Saturation flow rate for stream 3 or 6	1700	
P*(oj)	1.00	
d(M,LT), Delay for stream 1 or 4	7.9	8.6
N, Number of major street through lanes	1	
d(rank,1) Delay for stream 2 or 5	0.0	

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF LOS ANGELES AM PEAK HOUR

Level of Service Computation Report Circular 212 Planning Method (Future Volume Alternative) Intersection #7 WESTERN AVENUE/CAPITOL DRIVE. Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Cycle, Loss Time, Optimal Cycle, Control, Rights, Min. Green, Lanes, Volume Module, Saturation Flow Module, and Capacity Analysis Module.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF LOS ANGELES AM PEAK HOUR

Level of Service Computation Report Circular 212 Planning Method (Future Volume Alternative) Intersection #8 WESTERN AVENUE/CRESTWOOD STREET. Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Cycle, Loss Time, Optimal Cycle, Control, Rights, Min. Green, Lanes, Volume Module, Saturation Flow Module, and Capacity Analysis Module.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF LOS ANGELES
AM PEAK HOUR

Level Of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

Intersection #9 WESTERN AVENUE/1ST STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 1.516
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module:

Table with 12 columns and 14 rows showing traffic volume data for various approaches and conditions.

Saturation Flow Module:

Table with 12 columns and 4 rows showing saturation flow and adjustment factors.

Capacity Analysis Module:

Table with 12 columns and 4 rows showing capacity analysis results.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF LOS ANGELES
AM PEAK HOUR

Level Of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

Intersection #10 WESTERN AVENUE/9TH STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.659
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 59 Level Of Service: B

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module:

Table with 12 columns and 14 rows showing traffic volume data for various approaches and conditions.

Saturation Flow Module:

Table with 12 columns and 4 rows showing saturation flow and adjustment factors.

Capacity Analysis Module:

Table with 12 columns and 4 rows showing capacity analysis results.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF LOS ANGELES
AM PEAK HOUR

Level Of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

\*\*\*\*\*

Intersection #11 WESTERN AVENUE/WEST 25TH STREET

\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.814
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 108 Level Of Service: D

\*\*\*\*\*

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Includes Control, Rights, Min. Green, and Lanes.

-----|-----|-----|-----|

Volume Module: Table with 12 columns for volume metrics (Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume).

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Saturation Flow Module: Table with 12 columns for saturation flow metrics (Sat/Lane, Adjustment, Lanes, Final Sat.).

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Capacity Analysis Module: Table with 12 columns for capacity analysis metrics (Vol/Sat, Crit Volume, Crit Moves).

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MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF LOS ANGELES PM PEAK HOUR

Level Of Service Computation Report Circular 212 Planning Method (Future Volume Alternative) Intersection #7 WESTERN AVENUE/CAPITOL DRIVE. Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Cycle, Loss Time, Optimal Cycle, Control, Rights, Min. Green, Lanes, Volume Module, Saturation Flow Module, and Capacity Analysis Module.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF LOS ANGELES PM PEAK HOUR

Level Of Service Computation Report Circular 212 Planning Method (Future Volume Alternative) Intersection #8 WESTERN AVENUE/CRESTWOOD STREET. Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Cycle, Loss Time, Optimal Cycle, Control, Rights, Min. Green, Lanes, Volume Module, Saturation Flow Module, and Capacity Analysis Module.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF LOS ANGELES PM PEAK HOUR

Level of Service Computation Report
Circular 212 Planning Method (Future Volume Alternative)
Intersection #9 WESTERN AVENUE/1ST STREET
Cycle (sec): 100 Critical Vol./Cap. (X): 1.438
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Permitted Prot+Permit Permitted Prot+Permit
Rights: Ovl Include Include Ovl
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Lanes: 1 0 2 0 1 1 0 1 1 0 1 0 0 1 1
Volume Module:
Base Vol: 36 1259 215 178 1458 61 42 184 21 714 276 156
Growth Adj: 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04
Initial Bse: 38 1314 224 186 1522 64 44 192 22 745 288 163
Added Vol: 0 0 0 0 0 2 1 0 0 0 1 0
Cumulative: 0 116 0 25 111 18 18 0 0 0 0 26
Initial Fut: 38 1430 224 211 1633 84 63 192 22 745 289 189
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 41 1555 244 229 1775 91 68 209 24 810 314 205
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 41 1555 244 229 1775 91 68 209 24 810 314 205
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 41 1555 244 229 1775 91 68 209 24 810 314 205
Saturation Flow Module:
Sat/Lane: 1425 1425 1425 1425 1425 1425 1425 1425 1425 1425 1425
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 1.00 2.00 1.00 1.00 1.90 0.10 1.00 0.90 0.10 1.00 1.00 1.00
Final Sat.: 1425 2850 1425 1425 2711 139 1425 1279 146 1425 1425 1425
Capacity Analysis Module:
Vol/Sat: 0.03 0.55 0.17 0.16 0.65 0.65 0.05 0.16 0.16 0.57 0.22 0.14
Crit Volume: 777 229 233 810
Crit Moves: \*\*\*\* \*\*\*\* \*\*\*\* \*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF LOS ANGELES PM PEAK HOUR

Level of Service Computation Report
Circular 212 Planning Method (Future Volume Alternative)
Intersection #10 WESTERN AVENUE/9TH STREET
Cycle (sec): 100 Critical Vol./Cap. (X): 0.868
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 152 Level Of Service: D
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Protected Protected Permitted Permitted
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Lanes: 1 0 2 0 1 1 0 2 0 1 1 0 1 0 1
Volume Module:
Base Vol: 144 640 62 72 1076 51 38 164 213 159 117 50
Growth Adj: 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04
Initial Bse: 150 668 65 75 1123 53 40 171 222 166 122 52
Added Vol: 0 0 0 0 0 0 0 0 0 0 1 0
Cumulative: 0 78 0 23 76 5 5 0 0 0 0 23
Initial Fut: 150 746 65 98 1199 58 45 171 222 166 123 75
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 163 811 70 107 1304 63 49 186 242 180 134 82
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 163 811 70 107 1304 63 49 186 242 180 134 82
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 163 811 70 107 1304 63 49 186 242 180 134 82
Saturation Flow Module:
Sat/Lane: 1425 1425 1425 1425 1425 1425 1425 1425 1425 1425 1425
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 1.00 2.00 1.00 1.00 2.00 1.00 1.00 1.00 1.00 1.00 1.00
Final Sat.: 1425 2850 1425 1425 2850 1425 1425 1425 1425 1425 1425
Capacity Analysis Module:
Vol/Sat: 0.11 0.28 0.05 0.07 0.46 0.04 0.03 0.13 0.17 0.13 0.09 0.06
Crit Volume: 163 652 242 180
Crit Moves: \*\*\*\* \*\*\*\* \*\*\*\* \*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CITY OF LOS ANGELES
PM PEAK HOUR

Level Of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

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Intersection #11 WESTERN AVENUE/WEST 25TH STREET

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Cycle (sec): 100 Critical Vol./Cap.(X): 0.805
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 102 Level Of Service: D

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Table with columns: Approach (North Bound, South Bound, East Bound, West Bound), Movement (L, T, R), Control (Permitted, Protected), Rights (Include, Ovl), Min. Green, Lanes.

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Volume Module: Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume.

-----|-----|-----|-----|

Saturation Flow Module: Sat/Lane, Adjustment, Lanes, Final Sat.

-----|-----|-----|-----|

Capacity Analysis Module: Vol/Sat, Crit Volume, Crit Moves.

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MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CALTRANS AM PEAK HOUR

Level Of Service Computation Report 2000 HCM Operations Method (Future Volume Alternative)

\*\*\*\*\* Intersection #6 WESTERN AVENUE/TOSCANINI DRIVE \*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.754
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 18.3
Optimal Cycle: 62 Level Of Service: B

Table with 4 columns: Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Table with 12 columns for Volume Module. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Table with 12 columns for Saturation Flow Module. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Table with 12 columns for Capacity Analysis Module. Rows include Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CALTRANS AM PEAK HOUR

Level Of Service Computation Report 2000 HCM Operations Method (Future Volume Alternative)

\*\*\*\*\* Intersection #7 WESTERN AVENUE/CAPITOL DRIVE \*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.968
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 34.9
Optimal Cycle: 156 Level Of Service: C

Table with 4 columns: Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Table with 12 columns for Volume Module. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Table with 12 columns for Saturation Flow Module. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Table with 12 columns for Capacity Analysis Module. Rows include Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CALTRANS AM PEAK HOUR

Level Of Service Computation Report 2000 HCM Operations Method (Future Volume Alternative) Intersection #8 WESTERN AVENUE/CRESTWOOD STREET. Table with columns for Approach, Movement, Control, Rights, Min. Green, Lanes, Volume Module, Saturation Flow Module, Capacity Analysis Module.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CALTRANS AM PEAK HOUR

Level Of Service Computation Report 2000 HCM Operations Method (Future Volume Alternative) Intersection #9 WESTERN AVENUE/1ST STREET. Table with columns for Approach, Movement, Control, Rights, Min. Green, Lanes, Volume Module, Saturation Flow Module, Capacity Analysis Module.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CALTRANS
AM PEAK HOUR

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #10 WESTERN AVENUE/9TH STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.551
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 22.2
Optimal Cycle: 40 Level Of Service: C

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 3 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module:

Table with 12 columns and 12 rows showing traffic volume metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module:

Table with 12 columns and 4 rows showing saturation flow metrics like Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with 12 columns and 12 rows showing capacity analysis metrics like Vol/Sat, Crit Moves, Green/Cycle, etc.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CALTRANS
AM PEAK HOUR

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #11 WESTERN AVENUE/WEST 25TH STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.724
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 27.3
Optimal Cycle: 57 Level Of Service: C

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 3 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module:

Table with 12 columns and 12 rows showing traffic volume metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module:

Table with 12 columns and 4 rows showing saturation flow metrics like Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with 12 columns and 12 rows showing capacity analysis metrics like Vol/Sat, Crit Moves, Green/Cycle, etc.

Note: Queue reported is the number of cars per lane.

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MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CALTRANS PM PEAK HOUR

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #6 WESTERN AVENUE/TOSCANINI DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 0.647
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 10.3
Optimal Cycle: 48 Level Of Service: B

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 3 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module:

Table with 12 columns for traffic flows and 12 rows for various metrics: Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume.

Saturation Flow Module:

Table with 12 columns for traffic flows and 4 rows: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with 12 columns for traffic flows and 12 rows: Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CALTRANS PM PEAK HOUR

Level Of Service Computation Report

2000 HCM Operations Method (Future Volume Alternative)

Intersection #7 WESTERN AVENUE/CAPITOL DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 0.942
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 35.9
Optimal Cycle: 131 Level Of Service: D

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 3 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module:

Table with 12 columns for traffic flows and 12 rows for various metrics: Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume.

Saturation Flow Module:

Table with 12 columns for traffic flows and 4 rows: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with 12 columns for traffic flows and 12 rows: Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CALTRANS PM PEAK HOUR

Level Of Service Computation Report 2000 HCM Operations Method (Future Volume Alternative)

\*\*\*\*\* Intersection #8 WESTERN AVENUE/CRESTWOOD STREET \*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.843
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 21.9
Optimal Cycle: 83 Level Of Service: C

Table with 4 columns: Approach (North, South, East, West Bound) and Movement (L, T, R). Includes Control, Rights, Min. Green, and Lanes.

Volume Module table with 12 columns for traffic volume and 12 columns for HCM metrics (Base Vol, Growth Adj, etc.).

Saturation Flow Module table with 12 columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module table with 12 columns for Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CALTRANS PM PEAK HOUR

Level Of Service Computation Report 2000 HCM Operations Method (Future Volume Alternative)

\*\*\*\*\* Intersection #9 WESTERN AVENUE/1ST STREET \*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 1.227
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 117.6
Optimal Cycle: 180 Level Of Service: F

Table with 4 columns: Approach (North, South, East, West Bound) and Movement (L, T, R). Includes Control, Rights, Min. Green, and Lanes.

Volume Module table with 12 columns for traffic volume and 12 columns for HCM metrics (Base Vol, Growth Adj, etc.).

Saturation Flow Module table with 12 columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module table with 12 columns for Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CALTRANS PM PEAK HOUR

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)
Intersection #10 WESTERN AVENUE/9TH STREET
Cycle (sec): 100 Critical Vol./Cap. (X): 0.709
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 24.5
Optimal Cycle: 55 Level Of Service: C
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Protected Protected Permitted Permitted
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Lanes: 1 0 2 0 1 1 0 2 0 1 1 0 1 0 1
Volume Module:
Base Vol: 144 640 62 72 1076 51 38 164 213 159 117 50
Growth Adj: 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04
Initial Bse: 150 668 65 75 1123 53 40 171 222 166 122 52
Added Vol: 0 0 0 0 0 0 0 0 0 0 1 0
Cumulative: 0 78 0 23 76 5 5 0 0 0 0 23
Initial Fut: 150 746 65 98 1199 58 45 171 222 166 123 75
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 163 811 70 107 1304 63 49 186 242 180 134 82
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 163 811 70 107 1304 63 49 186 242 180 134 82
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 163 811 70 107 1304 63 49 186 242 180 134 82
Saturation Flow Module:
Sat/Lane: 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900
Adjustment: 0.95 0.95 0.85 0.95 0.95 0.85 0.60 1.00 0.85 0.51 1.00 0.85
Lanes: 1.00 2.00 1.00 1.00 2.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Final Sat.: 1805 3610 1615 1805 3610 1615 1142 1900 1615 967 1900 1615
Capacity Analysis Module:
Vol/Sat: 0.09 0.22 0.04 0.06 0.36 0.04 0.04 0.10 0.15 0.19 0.07 0.05
Crit Moves: \*\*\*\*
Green/Cycle: 0.13 0.50 0.50 0.13 0.51 0.51 0.26 0.26 0.26 0.26 0.26 0.26
Volume/Cap: 0.71 0.45 0.09 0.45 0.71 0.08 0.16 0.37 0.57 0.71 0.27 0.19
Delay/Veh: 51.6 16.0 12.9 41.3 20.2 12.6 28.6 30.6 33.8 42.3 29.5 28.8
User DelAdj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
AdjDel/Veh: 51.6 16.0 12.9 41.3 20.2 12.6 28.6 30.6 33.8 42.3 29.5 28.8
LOS by Move: D B B D C B C C C D C C
HCM2kAvgQ: 6 8 1 4 17 1 1 5 7 7 3 2

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CALTRANS PM PEAK HOUR

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)
Intersection #11 WESTERN AVENUE/WEST 25TH STREET
Cycle (sec): 100 Critical Vol./Cap. (X): 0.703
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 27.0
Optimal Cycle: 54 Level Of Service: C
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Permitted Permitted Protected Protected
Rights: Include Ovl Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Lanes: 1 0 2 0 1 1 0 2 0 1 2 0 2 0 1 1 0 1 0 1
Volume Module:
Base Vol: 54 106 55 206 215 369 477 306 35 71 290 176
Growth Adj: 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04
Initial Bse: 56 111 57 215 224 385 498 319 37 74 303 184
Added Vol: 0 0 0 0 0 0 0 1 0 0 2 0
Cumulative: 0 1 0 15 1 49 47 135 0 0 163 16
Initial Fut: 56 112 57 230 225 434 545 455 37 74 468 200
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 61 121 62 250 245 472 592 495 40 81 508 217
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 61 121 62 250 245 472 592 495 40 81 508 217
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 61 121 62 250 245 472 592 495 40 81 508 217
Saturation Flow Module:
Sat/Lane: 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900
Adjustment: 0.55 0.95 0.85 0.67 0.95 0.85 0.92 0.95 0.85 0.95 1.00 0.85
Lanes: 1.00 2.00 1.00 1.00 2.00 1.00 2.00 2.00 1.00 1.00 1.00 1.00
Final Sat.: 1036 3610 1615 1275 3610 1615 3502 3610 1615 1805 1900 1615
Capacity Analysis Module:
Vol/Sat: 0.06 0.03 0.04 0.20 0.07 0.29 0.17 0.14 0.02 0.04 0.27 0.13
Crit Moves: \*\*\*\*
Green/Cycle: 0.28 0.28 0.28 0.28 0.28 0.52 0.24 0.47 0.47 0.15 0.38 0.38
Volume/Cap: 0.21 0.12 0.14 0.70 0.24 0.56 0.70 0.29 0.05 0.29 0.70 0.35
Delay/Veh: 28.0 27.0 27.2 38.6 28.0 17.2 37.4 16.5 14.5 38.2 29.3 22.5
User DelAdj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
AdjDel/Veh: 28.0 27.0 27.2 38.6 28.0 17.2 37.4 16.5 14.5 38.2 29.3 22.5
LOS by Move: C C C D C B D B D C C
HCM2kAvgQ: 2 1 1 8 3 10 10 5 1 2 14 5

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CMP
AM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #6 WESTERN AVENUE/TOSCANINI DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 0.901
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 100 Level Of Service: E

Table with 4 columns: Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns representing traffic volumes and 12 rows for various adjustment factors like Growth Adj, Initial Bse, etc.

Saturation Flow Module:

Table with 12 columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns for Vol/Sat and Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CMP
AM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #10 WESTERN AVENUE/9TH STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.687
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 48 Level Of Service: B

Table with 4 columns: Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns representing traffic volumes and 12 rows for various adjustment factors like Growth Adj, Initial Bse, etc.

Saturation Flow Module:

Table with 12 columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns for Vol/Sat and Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CMP PM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\* Intersection #6 WESTERN AVENUE/TOSCANINI DRIVE \*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.787
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 64 Level Of Service: C

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module: Table with 11 columns (Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume) and 11 rows.

Saturation Flow Module: Table with 11 columns (Sat/Lane, Adjustment, Lanes, Final Sat) and 4 rows.

Capacity Analysis Module: Table with 11 columns (Vol/Sat, Crit Moves) and 2 rows.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITHOUT PROJECT CONDITIONS - CMP PM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\* Intersection #10 WESTERN AVENUE/9TH STREET \*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.873
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 88 Level Of Service: D

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module: Table with 11 columns (Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, FinalVolume) and 11 rows.

Saturation Flow Module: Table with 11 columns (Sat/Lane, Adjustment, Lanes, Final Sat) and 4 rows.

Capacity Analysis Module: Table with 11 columns (Vol/Sat, Crit Moves) and 2 rows.

## **Forecast Year 2012 With Project Conditions**



MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF REV AM PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

\*\*\*\*\* Intersection #3 PALOS VERDES DRIVE EAST/PALOS VERDES DRIVE SOUTH \*\*\*\*\*

Average Delay (sec/veh): 4.5 Worst Case Level Of Service: E[ 36.1]

Table with columns: Approach, Movement, Control, Rights, Lanes. Rows for North, South, East, West bounds.

Volume Module table with columns: Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume.

Critical Gap Module table with columns: Critical Gap, FollowUpTim.

Capacity Module table with columns: Cnflct Vol, Potent Cap., Move Cap., Volume/Cap.

Level Of Service Module table with columns: 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., Shared Queue, Shrd ConDel, Shared LOS, Approach Del, Approach LOS.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF REV AM PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

\*\*\*\*\* Intersection #5 MIRALESTE DRIVE/1ST STREET \*\*\*\*\*

Average Delay (sec/veh): 6.0 Worst Case Level Of Service: C[ 15.0]

Table with columns: Approach, Movement, Control, Rights, Lanes. Rows for North, South, East, West bounds.

Volume Module table with columns: Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume.

Critical Gap Module table with columns: Critical Gap, FollowUpTim.

Capacity Module table with columns: Cnflct Vol, Potent Cap., Move Cap., Volume/Cap.

Level Of Service Module table with columns: 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., Shared Queue, Shrd ConDel, Shared LOS, Approach Del, Approach LOS.

Note: Queue reported is the number of cars per lane.

HCS+: Unsignalized Intersections Release 5.21

TWO-WAY STOP CONTROL SUMMARY

Analyst: PM  
 Agency/Co.: RBF  
 Date Performed: 11-18-2009  
 Analysis Time Period: AM Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 WITH PROJECT  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: MIRALESTE  
 Intersection Orientation: NS Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	Northbound			Southbound		
	1 L	2 T	3 R	4 L	5 T	6 R
Volume	17	422	130	198	426	3
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	18	458	141	215	463	3
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L	TR	
Upstream Signal?	No			No		

Minor Street: Approach Movement	Westbound			Eastbound		
	7 L	8 T	9 R	10 L	11 T	12 R
Volume	40	14	301	4	4	15
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	43	15	327	4	4	16
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage				/ No /		
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Delay, Queue Length, and Level of Service

Approach Movement Lane Config	NB	SB	Westbound		Eastbound			
	1 LT	4 L	7 LT	8 R	9 R	10 L	11 LTR	12
v (vph)	18	215	58		327		24	
C(m) (vph)	1106	988	218		607		0	
v/c	0.02	0.22	0.27		0.54			
95% queue length	0.05	0.83	1.03		3.21			
Control Delay	8.3	9.7	27.4		17.6			
LOS	A	A	D		C		F	
Approach Delay				19.1				
Approach LOS				C				

HCS+: Unsignalized Intersections Release 5.21

Phone:  
 Fax:  
 E-Mail:

TWO-WAY STOP CONTROL(TWSC) ANALYSIS

Analyst: PM  
 Agency/Co.: RBF  
 Date Performed: 11-18-2009  
 Analysis Time Period: AM Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 WITH PROJECT  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: MIRALESTE  
 Intersection Orientation: NS Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street Movements	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	17	422	130	198	426	3
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	5	115	35	54	116	1
Hourly Flow Rate, HFR	18	458	141	215	463	3
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L	TR	
Upstream Signal?	No			No		

Minor Street Movements	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	40	14	301	4	4	15
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	11	4	82	1	1	4
Hourly Flow Rate, HFR	43	15	327	4	4	16
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage				/ No /		
RT Channelized?	No					
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Pedestrian Volumes and Adjustments

Movements	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)	12.0	12.0	12.0	12.0
Walking Speed (ft/sec)	4.0	4.0	4.0	4.0
Percent Blockage	0	0	0	0

Upstream Signal Data							
	Prog. Flow vph	Sat Flow vph	Arrival Type	Green Time sec	Cycle Length sec	Prog. Speed mph	Distance to Signal feet
S2	Left-Turn						
	Through						
S5	Left-Turn						
	Through						

Worksheet 3-Data for Computing Effect of Delay to Major Street Vehicles

	Movement 2	Movement 5
Shared ln volume, major th vehicles:	458	
Shared ln volume, major rt vehicles:	0	
Sat flow rate, major th vehicles:	1700	
Sat flow rate, major rt vehicles:	1700	
Number of major street through lanes:	1	

Worksheet 4-Critical Gap and Follow-up Time Calculation

Critical Gap Calculation									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
t(c,base)	4.1	4.1	7.1	6.5	6.2	7.1	6.5	6.2	
t(c,hv)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
P(hv)	0	0	0	0	0	0	0	0	
t(c,g)			0.20	0.20	0.10	0.20	0.20	0.10	
Grade/100			0.00	0.00	0.00	0.00	0.00	0.00	
t(3,lt)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
t(c,T):	1-stage	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
	2-stage	0.00	0.00	1.00	1.00	0.00	1.00	1.00	
t(c)	1-stage	4.1	4.1	7.1	6.5	7.1	6.5	6.2	
	2-stage	4.1	4.1	6.1	5.5	6.1	5.5	6.2	

Follow-Up Time Calculations									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
t(f,base)	2.20	2.20	3.50	4.00	3.30	3.50	4.00	3.30	
t(f,HV)	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	
P(HV)	0	0	0	0	0	0	0	0	
t(f)	2.2	2.2	3.5	4.0	3.3	3.5	4.0	3.3	

Worksheet 5-Effect of Upstream Signals

Computation 1-Queue Clearance Time at Upstream Signal				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
V prog				
Total Saturation Flow Rate, s (vph)				
Arrival Type				
Effective Green, g (sec)				
Cycle Length, C (sec)				
Rp (from Exhibit 16-11)				
Proportion vehicles arriving on green P				
g(q1)				
g(q2)				
g(q)				

Computation 2-Proportion of TWSC Intersection Time blocked				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
alpha				
beta				
Travel time, t(a) (sec)				
Smoothing Factor, F				
Proportion of conflicting flow, f				
Max platooned flow, V(c,max)				
Min platooned flow, V(c,min)				
Duration of blocked period, t(p)				
Proportion time blocked, p		0.000		0.000

Computation 3-Platoon Event Periods		Result
p(2)		0.000
p(5)		0.000
p(dom)		
p(subo)		
Constrained or unconstrained?		

Proportion unblocked for minor movements, p(x)	(1)	(2)	(3)
	Single-stage Process	Two-Stage Stage I	Process Stage II
p(1)			
p(4)			
p(7)			
p(8)			
p(9)			
p(10)			
p(11)			
p(12)			

Computation 4 and 5 Single-Stage Process									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
V c,x	466	599	1398	1390	458	1630	1529	464	
s									
Px									
V c,u,x									
C r,x									
C plat,x									

	Two-Stage Process							
	7		8		10		11	
	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2
V(c,x)	494	904	494	896	894	736	894	635
s		1500		1500		1500		1500
P(x)								
V(c,u,x)								
C(r,x)								
C(plat,x)								

Worksheet 6-Impedance and Capacity Equations

Step 1: RT from Minor St.	9	12
Conflicting Flows	458	464
Potential Capacity	607	602
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	607	602
Probability of Queue free St.	0.46	0.97
Step 2: LT from Major St.	4	1
Conflicting Flows	599	466
Potential Capacity	988	1106
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	988	1106
Probability of Queue free St.	0.78	0.98
Maj L-Shared Prob Q free St.		0.98
Step 3: TH from Minor St.	8	11
Conflicting Flows	1390	1529
Potential Capacity	144	118
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.76	0.76
Movement Capacity	110	90
Probability of Queue free St.	0.94	0.98
Step 4: LT from Minor St.	7	10
Conflicting Flows	1398	1630
Potential Capacity	119	82
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.75	0.72
Maj. L, Min T Adj. Imp Factor.	0.81	0.78
Cap. Adj. factor due to Impeding mvmnt	0.79	0.36
Movement Capacity	93	30

Worksheet 7-Computation of the Effect of Two-stage Gap Acceptance

Step 3: TH from Minor St.	8	11
Part 1 - First Stage		
Conflicting Flows	494	894
Potential Capacity	550	362
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.98	0.78
Movement Capacity	538	283
Probability of Queue free St.	0.97	0.99
Part 2 - Second Stage		
Conflicting Flows	896	635
Potential Capacity	362	476
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.78	0.98
Movement Capacity	283	465
Part 3 - Single Stage		
Conflicting Flows	1390	1529
Potential Capacity	144	118
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.76	0.76
Movement Capacity	110	90
Result for 2 stage process:		
a	0.95	0.95

Y	2.76	1.21
C t	239	196
Probability of Queue free St.	0.94	0.98
Step 4: LT from Minor St.	7	10
Part 1 - First Stage		
Conflicting Flows	494	894
Potential Capacity	561	338
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.98	0.78
Movement Capacity	549	264
Part 2 - Second Stage		
Conflicting Flows	904	736
Potential Capacity	334	414
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.75	0.44
Movement Capacity	251	182
Part 3 - Single Stage		
Conflicting Flows	1398	1630
Potential Capacity	119	82
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.75	0.72
Maj. L, Min T Adj. Imp Factor.	0.81	0.78
Cap. Adj. factor due to Impeding mvmnt	0.79	0.36
Movement Capacity	93	30
Results for Two-stage process:		
a	0.95	0.95
Y	3.26	-3.71
C t	212	0

Worksheet 8-Shared Lane Calculations

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (vph)	43	15	327	4	4	16
Movement Capacity (vph)	212	239	607	0	196	602
Shared Lane Capacity (vph)	218				0	

Worksheet 9-Computation of Effect of Flared Minor Street Approaches

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
C sep	212	239	607	0	196	602
Volume	43	15	327	4	4	16
Delay						
Q sep						
Q sep +1						
round (Qsep +1)						
n max						
C sh	218				0	
SUM C sep						
n						
C act						

Worksheet 10-Delay, Queue Length, and Level of Service

Movement	1	4	7	8	9	10	11	12
Lane Config	LT	L	LT		R		LTR	
v (vph)	18	215	58		327		24	
C(m) (vph)	1106	988	218		607		0	
v/c	0.02	0.22	0.27		0.54			
95% queue length	0.05	0.83	1.03		3.21			
Control Delay	8.3	9.7	27.4		17.6			
LOS	A	A	D		C		F	
Approach Delay				19.1				
Approach LOS				C				

Worksheet 11-Shared Major LT Impedance and Delay

	Movement 2	Movement 5
p(oj)	0.98	0.78
v(i1), Volume for stream 2 or 5	458	
v(i2), Volume for stream 3 or 6	0	
s(i1), Saturation flow rate for stream 2 or 5	1700	
s(i2), Saturation flow rate for stream 3 or 6	1700	
P*(oj)	0.98	
d(M,LT), Delay for stream 1 or 4	8.3	9.7
N, Number of major street through lanes	1	
d(rank,1) Delay for stream 2 or 5	0.2	

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF RPV AM PEAK HOUR

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)
Intersection #6 WESTERN AVENUE/TOSCANINI DRIVE
Cycle (sec): 100 Critical Vol./Cap.(X): 0.908
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 104 Level Of Service: E
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Protected Protected Permitted Permitted
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Lanes: 1 0 1 1 0 1 0 1 1 0 0 1 0 0 1
Volume Module:
Base Vol: 59 1382 21 16 878 15 65 21 249 81 47 46
Growth Adj: 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04
Initial Bse: 62 1443 22 17 917 16 68 22 260 85 49 48
Added Vol: 0 22 0 0 65 0 0 0 0 0 0 0
Cumulative: 0 165 0 2 175 2 3 0 0 0 0 3
Initial Fut: 62 1630 22 19 1157 18 71 22 260 85 49 51
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 67 1772 24 20 1257 19 77 24 283 92 53 55
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 67 1772 24 20 1257 19 77 24 283 92 53 55
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 67 1772 24 20 1257 19 77 24 283 92 53 55
Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 1.00 1.97 0.03 1.00 1.97 0.03 0.76 0.24 1.00 0.63 0.37 1.00
Final Sat.: 1600 3158 42 1600 3152 48 1222 378 1600 1013 587 1600
Capacity Analysis Module:
Vol/Sat: 0.04 0.56 0.56 0.01 0.40 0.40 0.05 0.06 0.18 0.06 0.09 0.03
Crit Moves: \*\*\*\* \*\*\*\* \*\*\*\* \*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF RPV AM PEAK HOUR

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)
Intersection #7 WESTERN AVENUE/CAPITOL DRIVE
Cycle (sec): 100 Critical Vol./Cap.(X): 1.077
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Protected Protected Permitted Permitted
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Lanes: 1 0 2 0 1 1 0 2 0 1 0 1 0 0 1
Volume Module:
Base Vol: 13 1332 208 132 907 85 132 74 44 190 53 165
Growth Adj: 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04
Initial Bse: 14 1391 217 138 947 89 138 77 46 198 55 172
Added Vol: 0 5 3 0 16 49 16 5 0 8 16 0
Cumulative: 1 96 39 52 113 2 3 9 0 48 7 63
Initial Fut: 15 1492 259 190 1076 140 157 91 46 254 78 235
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 16 1621 282 206 1169 152 170 99 50 276 85 256
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 16 1621 282 206 1169 152 170 99 50 276 85 256
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 16 1621 282 206 1169 152 170 99 50 276 85 256
Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 1.00 2.00 1.00 1.00 2.00 1.00 0.63 0.37 1.00 1.00 1.00 1.00
Final Sat.: 1600 3200 1600 1600 3200 1600 1011 589 1600 1600 1600 1600
Capacity Analysis Module:
Vol/Sat: 0.01 0.51 0.18 0.13 0.37 0.09 0.11 0.17 0.03 0.17 0.05 0.16
Crit Moves: \*\*\*\* \*\*\*\* \*\*\*\* \*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF RPV AM PEAK HOUR

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)
Intersection #8 WESTERN AVENUE/CRESTWOOD STREET
Cycle (sec): 100 Critical Vol./Cap.(X): 0.866
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 86 Level Of Service: D
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Permitted Permitted Split Phase Split Phase
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Lanes: 1 0 2 0 1 1 0 2 0 1 1 0 0 1 0
Volume Module:
Base Vol: 93 1394 149 23 1107 128 179 31 104 83 10 25
Growth Adj: 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04
Initial Bse: 97 1455 156 24 1156 134 187 32 109 87 10 26
Added Vol: 0 8 0 0 24 0 0 0 0 0 0 0
Cumulative: 23 86 15 15 98 25 26 8 23 15 7 15
Initial Fut: 120 1549 171 39 1278 159 213 40 132 102 17 41
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 131 1684 185 42 1389 172 231 44 143 110 19 45
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 131 1684 185 42 1389 172 231 44 143 110 19 45
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 131 1684 185 42 1389 172 231 44 143 110 19 45
Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 1.00 2.00 1.00 1.00 2.00 1.00 1.00 0.23 0.77 1.00 0.30 0.70
Final Sat.: 1600 3200 1600 1600 3200 1600 1600 376 1224 1600 477 1123
Capacity Analysis Module:
Vol/Sat: 0.08 0.53 0.12 0.03 0.43 0.11 0.14 0.12 0.12 0.07 0.04 0.04
Crit Moves: \*\*\*\* \*\*\*\* \*\*\*\* \*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF RPV AM PEAK HOUR

Level Of Service Computation Report
Circular 212 Planning Method (Future Volume Alternative)
Intersection #9 WESTERN AVENUE/1ST STREET
Cycle (sec): 100 Critical Vol./Cap.(X): 1.465
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Permitted Prot+Permit Permitted Prot+Permit
Rights: Ovl Include Include Ovl
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Lanes: 1 0 2 0 1 1 0 1 1 0 1 0 0 1 0 1
Volume Module:
Base Vol: 6 1612 635 189 1193 20 60 402 32 360 211 176
Growth Adj: 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04
Initial Bse: 6 1683 663 197 1245 21 63 420 33 376 220 184
Added Vol: 0 0 0 0 0 24 8 3 0 0 8 0
Cumulative: 0 64 0 18 90 14 9 0 0 0 0 15
Initial Fut: 6 1747 663 215 1335 59 80 423 33 376 228 199
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 7 1899 721 234 1452 64 87 459 36 409 248 216
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 7 1899 721 234 1452 64 87 459 36 409 248 216
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 7 1899 721 234 1452 64 87 459 36 409 248 216
Saturation Flow Module:
Sat/Lane: 1425 1425 1425 1425 1425 1425 1425 1425 1425 1425 1425
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 1.00 2.00 1.00 1.00 1.92 0.08 1.00 0.93 0.07 1.00 1.00 1.00
Final Sat.: 1425 2850 1425 1425 2730 120 1425 1321 104 1425 1425 1425
Capacity Analysis Module:
Vol/Sat: 0.00 0.67 0.51 0.16 0.53 0.53 0.06 0.35 0.35 0.29 0.17 0.15
Crit Volume: 949 234 496 409
Crit Moves: \*\*\*\* \*\*\*\* \*\*\*\* \*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF RPV AM PEAK HOUR

Level Of Service Computation Report Circular 212 Planning Method (Future Volume Alternative)

Intersection #10 WESTERN AVENUE/9TH STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.661
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 59 Level Of Service: B

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module table with 11 columns (Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume) and 11 rows.

Saturation Flow Module table with 11 columns (Sat/Lane, Adjustment, Lanes, Final Sat) and 4 rows.

Capacity Analysis Module table with 11 columns (Vol/Sat, Crit Volume, Crit Moves) and 4 rows.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF RPV AM PEAK HOUR

Level Of Service Computation Report Circular 212 Planning Method (Future Volume Alternative)

Intersection #11 WESTERN AVENUE/WEST 25TH STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.835
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 121 Level Of Service: D

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module table with 11 columns (Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume) and 11 rows.

Saturation Flow Module table with 11 columns (Sat/Lane, Adjustment, Lanes, Final Sat) and 4 rows.

Capacity Analysis Module table with 11 columns (Vol/Sat, Crit Volume, Crit Moves) and 4 rows.



MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF REV
PM PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #3 PALOS VERDES DRIVE EAST/PALOS VERDES DRIVE SOUTH

Average Delay (sec/veh): 5.8 Worst Case Level Of Service: E[ 47.5]

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled
Rights: Include Include Include Include

Lanes: 0 0 0 0 0 1 0 0 0 1 1 0 1 0 0 0 0 1 0 1

Volume Module:

Table with 20 columns and 10 rows showing traffic volume data for various approaches and movements.

Critical Gap Module:

Table with 20 columns and 2 rows showing critical gap and follow-up time data.

Capacity Module:

Table with 20 columns and 4 rows showing capacity and volume/capacity data.

Level Of Service Module:

Table with 20 columns and 10 rows showing level of service, control delay, and approach delay data.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF REV
PM PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

Intersection #5 MIRALESTE DRIVE/1ST STREET

Average Delay (sec/veh): 4.9 Worst Case Level Of Service: C[ 15.3]

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Stop Sign Stop Sign
Rights: Include Include Include Include

Lanes: 0 0 1 0 1 1 0 1 0 0 0 0 0 0 0 1 0 0 0 1

Volume Module:

Table with 20 columns and 10 rows showing traffic volume data for various approaches and movements.

Critical Gap Module:

Table with 20 columns and 2 rows showing critical gap and follow-up time data.

Capacity Module:

Table with 20 columns and 4 rows showing capacity and volume/capacity data.

Level Of Service Module:

Table with 20 columns and 10 rows showing level of service, control delay, and approach delay data.

Note: Queue reported is the number of cars per lane.

HCS+: Unsignalized Intersections Release 5.21

TWO-WAY STOP CONTROL SUMMARY

Analyst: PM  
 Agency/Co.: RBF  
 Date Performed: 11-18-2009  
 Analysis Time Period: PM Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 WITH PROJECT  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street:  
 Intersection Orientation: NS Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	Northbound			Southbound		
	1 L	2 T	3 R	4 L	5 T	6 R
Volume	7	422	69	203	500	2
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	7	458	74	220	543	2
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L	TR	
Upstream Signal?	No			No		

Minor Street: Approach Movement	Westbound			Eastbound		
	7 L	8 T	9 R	10 L	11 T	12 R
Volume	47	6	255	2	5	6
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	51	6	277	2	5	6
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage				/ No /		
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Delay, Queue Length, and Level of Service

Approach Movement Lane Config	NB	SB	Westbound		Eastbound			
	1 LT	4 L	7 LT	8 R	9 R	10 L	11 LTR	12
v (vph)	7	220	57		277		13	
C(m) (vph)	1034	1046	205		607		0	
v/c	0.01	0.21	0.28		0.46			
95% queue length	0.02	0.79	1.09		2.38			
Control Delay	8.5	9.4	29.2		15.8			
LOS	A	A	D		C		F	
Approach Delay				18.1				
Approach LOS				C				

HCS+: Unsignalized Intersections Release 5.21

Phone:  
 E-Mail: Fax:

TWO-WAY STOP CONTROL(TWSC) ANALYSIS

Analyst: PM  
 Agency/Co.: RBF  
 Date Performed: 11-18-2009  
 Analysis Time Period: PM Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 WITH PROJECT  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street:  
 Intersection Orientation: NS Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street Movements	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	7	422	69	203	500	2
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	2	115	19	55	136	1
Hourly Flow Rate, HFR	7	458	74	220	543	2
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L	TR	
Upstream Signal?	No			No		

Minor Street Movements	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	47	6	255	2	5	6
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	13	2	69	1	1	2
Hourly Flow Rate, HFR	51	6	277	2	5	6
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage				/ No /		
RT Channelized?	No					
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Pedestrian Volumes and Adjustments

Movements	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)	12.0	12.0	12.0	12.0
Walking Speed (ft/sec)	4.0	4.0	4.0	4.0
Percent Blockage	0	0	0	0

Upstream Signal Data							
	Prog. Flow vph	Sat Flow vph	Arrival Type	Green Time sec	Cycle Length sec	Prog. Speed mph	Distance to Signal feet
S2	Left-Turn						
	Through						
S5	Left-Turn						
	Through						

Worksheet 3-Data for Computing Effect of Delay to Major Street Vehicles

	Movement 2	Movement 5
Shared ln volume, major th vehicles:	458	
Shared ln volume, major rt vehicles:	0	
Sat flow rate, major th vehicles:	1700	
Sat flow rate, major rt vehicles:	1700	
Number of major street through lanes:	1	

Worksheet 4-Critical Gap and Follow-up Time Calculation

Critical Gap Calculation								
Movement	1	4	7	8	9	10	11	12
	L	L	L	T	R	L	T	R
t(c,base)	4.1	4.1	7.1	6.5	6.2	7.1	6.5	6.2
t(c,hv)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
P(hv)	0	0	0	0	0	0	0	0
t(c,g)			0.20	0.20	0.10	0.20	0.20	0.10
Grade/100			0.00	0.00	0.00	0.00	0.00	0.00
t(3,lt)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
t(c,T):	1-stage 0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2-stage 0.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00
t(c)	1-stage 4.1	4.1	7.1	6.5	6.2	7.1	6.5	6.2
	2-stage 4.1	4.1	6.1	5.5	6.2	6.1	5.5	6.2

Follow-Up Time Calculations								
Movement	1	4	7	8	9	10	11	12
	L	L	L	T	R	L	T	R
t(f,base)	2.20	2.20	3.50	4.00	3.30	3.50	4.00	3.30
t(f,HV)	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
P(HV)	0	0	0	0	0	0	0	0
t(f)	2.2	2.2	3.5	4.0	3.3	3.5	4.0	3.3

Worksheet 5-Effect of Upstream Signals

Computation 1-Queue Clearance Time at Upstream Signal				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
V prog				
Total Saturation Flow Rate, s (vph)				
Arrival Type				
Effective Green, g (sec)				
Cycle Length, C (sec)				
Rp (from Exhibit 16-11)				
Proportion vehicles arriving on green P				
g(q1)				
g(q2)				
g(q)				

Computation 2-Proportion of TWSC Intersection Time blocked				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
alpha				
beta				
Travel time, t(a) (sec)				
Smoothing Factor, F				
Proportion of conflicting flow, f				
Max platooned flow, V(c,max)				
Min platooned flow, V(c,min)				
Duration of blocked period, t(p)				
Proportion time blocked, p		0.000		0.000

Computation 3-Platoon Event Periods		Result
p(2)		0.000
p(5)		0.000
p(dom)		
p(subo)		
Constrained or unconstrained?		

Proportion unblocked for minor movements, p(x)	(1)	(2)	(3)
	Single-stage Process	Two-Stage Stage I	Process Stage II
p(1)			
p(4)			
p(7)			
p(8)			
p(9)			
p(10)			
p(11)			
p(12)			

Computation 4 and 5 Single-Stage Process								
Movement	1	4	7	8	9	10	11	12
	L	L	L	T	R	L	T	R
V c,x	545	532	1462	1457	458	1634	1530	544
s								
Px								
V c,u,x								
C r,x								
C plat,x								

	7		8		10		11	
	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2
V(c,x)	472	990	472	985	984	650	984	546
s		1500		1500		1500		1500
P(x)								
V(c,u,x)								
C(r,x)								
C(plat,x)								

Worksheet 6-Impedance and Capacity Equations

Step 1: RT from Minor St.	9	12
Conflicting Flows	458	544
Potential Capacity	607	543
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	607	543
Probability of Queue free St.	0.54	0.99
Step 2: LT from Major St.	4	1
Conflicting Flows	532	545
Potential Capacity	1046	1034
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	1046	1034
Probability of Queue free St.	0.79	0.99
Maj L-Shared Prob Q free St.		0.99
Step 3: TH from Minor St.	8	11
Conflicting Flows	1457	1530
Potential Capacity	131	118
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.78	0.78
Movement Capacity	102	92
Probability of Queue free St.	0.97	0.98
Step 4: LT from Minor St.	7	10
Conflicting Flows	1462	1634
Potential Capacity	108	82
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.76	0.76
Maj. L, Min T Adj. Imp Factor.	0.82	0.82
Cap. Adj. factor due to Impeding mvmnt	0.81	0.44
Movement Capacity	87	36

Worksheet 7-Computation of the Effect of Two-stage Gap Acceptance

Step 3: TH from Minor St.	8	11
Part 1 - First Stage		
Conflicting Flows	472	984
Potential Capacity	562	329
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.99	0.79
Movement Capacity	557	260
Probability of Queue free St.	0.99	0.98
Part 2 - Second Stage		
Conflicting Flows	985	546
Potential Capacity	329	521
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.79	0.99
Movement Capacity	260	516
Part 3 - Single Stage		
Conflicting Flows	1457	1530
Potential Capacity	131	118
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.78	0.78
Movement Capacity	102	92
Result for 2 stage process:		
a	0.95	0.95

Y	3.01	0.82
C t	229	204
Probability of Queue free St.	0.97	0.98
Step 4: LT from Minor St.	7	10
Part 1 - First Stage		
Conflicting Flows	472	984
Potential Capacity	576	302
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.99	0.79
Movement Capacity	571	238
Part 2 - Second Stage		
Conflicting Flows	990	650
Potential Capacity	299	461
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.77	0.53
Movement Capacity	229	246
Part 3 - Single Stage		
Conflicting Flows	1462	1634
Potential Capacity	108	82
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.76	0.76
Maj. L, Min T Adj. Imp Factor.	0.82	0.82
Cap. Adj. factor due to Impeding mvmnt	0.81	0.44
Movement Capacity	87	36
Results for Two-stage process:		
a	0.95	0.95
Y	3.59	-20.20
C t	203	0

Worksheet 8-Shared Lane Calculations

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (vph)	51	6	277	2	5	6
Movement Capacity (vph)	203	229	607	0	204	543
Shared Lane Capacity (vph)	205				0	

Worksheet 9-Computation of Effect of Flared Minor Street Approaches

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
C sep	203	229	607	0	204	543
Volume	51	6	277	2	5	6
Delay						
Q sep						
Q sep +1						
round (Qsep +1)						
n max						
C sh	205				0	
SUM C sep						
n						
C act						

Worksheet 10-Delay, Queue Length, and Level of Service

Movement	1	4	7	8	9	10	11	12
Lane Config	LT	L	LT		R		LTR	
v (vph)	7	220	57		277		13	
C(m) (vph)	1034	1046	205		607		0	
v/c	0.01	0.21	0.28		0.46			
95% queue length	0.02	0.79	1.09		2.38			
Control Delay	8.5	9.4	29.2		15.8			
LOS	A	A	D		C		F	
Approach Delay				18.1				
Approach LOS				C				

Worksheet 11-Shared Major LT Impedance and Delay

	Movement 2	Movement 5
p(oj)	0.99	0.79
v(i1), Volume for stream 2 or 5	458	
v(i2), Volume for stream 3 or 6	0	
s(i1), Saturation flow rate for stream 2 or 5	1700	
s(i2), Saturation flow rate for stream 3 or 6	1700	
P*(oj)	0.99	
d(M,LT), Delay for stream 1 or 4	8.5	9.4
N, Number of major street through lanes	1	
d(rank,1) Delay for stream 2 or 5	0.1	

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF RPV PM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\* Intersection #6 WESTERN AVENUE/TOSCANINI DRIVE \*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.799
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 66 Level Of Service: C

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 sub-columns for movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module table with 12 columns for volume and 12 rows for various traffic metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module table with 12 columns for Sat/Lane and 12 rows for Adjustment, Lanes, and Final Sat.

Capacity Analysis Module table with 12 columns for Vol/Sat and 12 rows for Crit Moves.

\*\*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF RPV PM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\* Intersection #7 WESTERN AVENUE/CAPITOL DRIVE \*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 1.064
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 sub-columns for movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module table with 12 columns for volume and 12 rows for various traffic metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module table with 12 columns for Sat/Lane and 12 rows for Adjustment, Lanes, and Final Sat.

Capacity Analysis Module table with 12 columns for Vol/Sat and 12 rows for Crit Moves.

\*\*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF REV PM PEAK HOUR

Level of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)
Intersection #8 WESTERN AVENUE/CRESTWOOD STREET
Cycle (sec): 100 Critical Vol./Cap.(X): 0.900
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 100 Level Of Service: D
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Permitted Permitted Split Phase Split Phase
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Lanes: 1 0 2 0 1 1 0 2 0 1 1 0 0 1 0 1 0 0 1 0
Volume Module:
Base Vol: 47 1197 302 50 1246 117 89 49 45 194 82 61
Growth Adj: 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04
Initial Bse: 49 1250 315 52 1301 122 93 51 47 203 86 64
Added Vol: 0 15 0 0 14 0 0 0 0 0 0 0
Cumulative: 31 128 39 39 92 32 31 19 30 40 20 40
Initial Fut: 80 1393 354 91 1407 154 124 70 77 243 106 104
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 87 1514 385 99 1529 168 135 76 84 264 115 113
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 87 1514 385 99 1529 168 135 76 84 264 115 113
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 87 1514 385 99 1529 168 135 76 84 264 115 113
Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 1.00 2.00 1.00 1.00 2.00 1.00 1.00 0.48 0.52 1.00 0.50 0.50
Final Sat.: 1600 3200 1600 1600 3200 1600 1600 763 837 1600 807 793
Capacity Analysis Module:
Vol/Sat: 0.05 0.47 0.24 0.06 0.48 0.10 0.08 0.10 0.10 0.16 0.14 0.14
Crit Moves: \*\*\*\* \*\*\*\* \*\*\*\* \*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF REV PM PEAK HOUR

Level of Service Computation Report
Circular 212 Planning Method (Future Volume Alternative)
Intersection #9 WESTERN AVENUE/1ST STREET
Cycle (sec): 100 Critical Vol./Cap.(X): 1.442
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Permitted Prot+Permit Permitted Prot+Permit
Rights: Ovl Include Include Ovl
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Lanes: 1 0 2 0 1 1 0 1 1 0 1 0 0 1 0 1 0 1 0 1
Volume Module:
Base Vol: 36 1259 215 178 1458 61 42 184 21 714 276 156
Growth Adj: 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04
Initial Bse: 38 1314 224 186 1522 64 44 192 22 745 288 163
Added Vol: 0 0 0 0 0 14 15 5 0 0 5 0
Cumulative: 0 116 0 25 111 18 18 0 0 0 0 26
Initial Fut: 38 1430 224 211 1633 96 77 197 22 745 293 189
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 41 1555 244 229 1775 104 84 214 24 810 319 205
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 41 1555 244 229 1775 104 84 214 24 810 319 205
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 41 1555 244 229 1775 104 84 214 24 810 319 205
Saturation Flow Module:
Sat/Lane: 1425 1425 1425 1425 1425 1425 1425 1425 1425 1425 1425 1425
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 1.00 2.00 1.00 1.00 1.89 0.11 1.00 0.90 0.10 1.00 1.00 1.00
Final Sat.: 1425 2850 1425 1425 2692 158 1425 1282 143 1425 1425 1425
Capacity Analysis Module:
Vol/Sat: 0.03 0.55 0.17 0.16 0.66 0.66 0.06 0.17 0.17 0.57 0.22 0.14
Crit Volume: 777 229 238 810
Crit Moves: \*\*\*\* \*\*\*\* \*\*\*\* \*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF RPV PM PEAK HOUR

Level Of Service Computation Report Circular 212 Planning Method (Future Volume Alternative) Intersection #10 WESTERN AVENUE/9TH STREET. Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Cycle, Loss Time, Optimal Cycle, Control, Rights, Min. Green, Lanes, Volume Module, Saturation Flow Module, and Capacity Analysis Module.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF RPV PM PEAK HOUR

Level Of Service Computation Report Circular 212 Planning Method (Future Volume Alternative) Intersection #11 WESTERN AVENUE/WEST 25TH STREET. Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Cycle, Loss Time, Optimal Cycle, Control, Rights, Min. Green, Lanes, Volume Module, Saturation Flow Module, and Capacity Analysis Module.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF RPV
WEEKDAY (11:00-1:00) PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #1 PALOS VERDES DRIVE EAST/MIRALESTE DRIVE
\*\*\*\*\*

Average Delay (sec/veh): 153.4 Worst Case Level Of Service: F[398.2]

Table with columns: Approach, Movement, Control, Rights, Lanes. Rows for North, South, East, West Bound movements.

Volume Module:

Table with columns: Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume.

Critical Gap Module:

Table with columns: Critical Gap, FollowUpTim.

Capacity Module:

Table with columns: Cnflct Vol, Potent Cap., Move Cap., Volume/Cap.

Level Of Service Module:

Table with columns: 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF RPV
WEEKDAY (11:00-1:00) PEAK HOUR

Level Of Service Computation Report

ICU l(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #2 CREST DRIVE - COLLEGE ENTRANCE/PALOS VERDES DRIVE EAST
\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.478

Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 32 Level Of Service: A

Table with columns: Approach, Movement, Control, Rights, Lanes. Rows for North, South, East, West Bound movements.

Volume Module:

Table with columns: Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume.

Saturation Flow Module:

Table with columns: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with columns: Vol/Sat, Crit Moves.

\*\*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF RPV
WEEKDAY (11:00-1:00) PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

\*\*\*\*\*

Intersection #3 PALOS VERDES DRIVE EAST/PALOS VERDES DRIVE SOUTH

\*\*\*\*\*

Average Delay (sec/veh): 3.4 Worst Case Level Of Service: C[ 23.4]

\*\*\*\*\*

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled

Rights: Include Include Include Include

Lanes: 0 0 0 0 0 1 0 0 0 1 1 0 1 0 0 0 0 1 0 1

-----|-----|-----|-----|

Volume Module:

Table with 16 columns and 12 rows showing traffic volume data: Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume.

-----|-----|-----|-----|

Critical Gap Module:

Critical Gp:xxxxx xxxxx xxxxx 6.4 xxxxx 6.2 4.1 xxxxx xxxxxx xxxxx xxxxx xxxxxx

FollowUpTim:xxxxx xxxxx xxxxx 3.5 xxxxx 3.3 2.2 xxxxx xxxxxx xxxxx xxxxx xxxxxx

-----|-----|-----|-----|

Capacity Module:

Cnflct Vol: xxxxx xxxxx xxxxxx 1238 xxxxx 508 560 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Potent Cap.: xxxxx xxxxx xxxxxx 196 xxxxx 569 1021 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Move Cap.: xxxxx xxxxx xxxxxx 185 xxxxx 569 1021 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Volume/Cap: xxxxx xxxxx xxxxx 0.41 xxxxx 0.17 0.07 xxxxx xxxxx xxxxx xxxxx xxxxxx

-----|-----|-----|-----|

Level Of Service Module:

2Way95thQ: xxxxx xxxxx xxxxxx 1.8 xxxxx 0.6 0.2 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Control Del:xxxxxx xxxxx xxxxxx 37.4 xxxxx 12.7 8.8 xxxxx xxxxxx xxxxxx xxxxx xxxxxx

LOS by Move: \* \* \* E \* B A \* \* \* \* \* \*

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx

SharedQueue:xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx

Shrd ConDel:xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx

Shared LOS: \* \* \* \* \* \* \* \* \* \* \* \* \* \* \* \*

ApproachDel: xxxxxxx 23.4 xxxxxxx xxxxxxx

ApproachLOS: \* C \*

\*\*\*\*\*

Note: Queue reported is the number of cars per lane.

\*\*\*\*\*

TWO-WAY STOP CONTROL SUMMARY

Analyst: PM  
 Agency/Co.: RBF  
 Date Performed: 11-18-2009  
 Analysis Time Period: Weekday Mid-day Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 WITH PROJECT  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: MIRALESTE  
 Intersection Orientation: NS  
 Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	Northbound			Southbound		
	1 L	2 T	3 R	4 L	5 T	6 R
Volume	6	301	53	222	354	7
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	6	327	57	241	384	7
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L		TR
Upstream Signal?	No			No		

Minor Street: Approach Movement	Westbound			Eastbound		
	7 L	8 T	9 R	10 L	11 T	12 R
Volume	36	18	267	4	3	6
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	39	19	290	4	3	6
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage				/ No /		
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Delay, Queue Length, and Level of Service

Approach Movement Lane Config	NB	SB	Westbound			Eastbound		
	1 LT	4 L	7 LT	8 R	9 R	10 L	11 LTR	12 R
v (vph)	6	241	58		290		13	
C(m) (vph)	1179	1186	252		719		161	
v/c	0.01	0.20	0.23		0.40		0.08	
95% queue length	0.02	0.76	0.87		1.96		0.26	
Control Delay	8.1	8.8	23.5		13.3		29.3	
LOS	A	A	C		B		D	
Approach Delay				15.0+			29.3	
Approach LOS				C			D	

Phone:  
 Fax:  
 E-Mail:

TWO-WAY STOP CONTROL(TWSC) ANALYSIS

Analyst: PM  
 Agency/Co.: RBF  
 Date Performed: 11-18-2009  
 Analysis Time Period: Weekday Mid-day Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 WITH PROJECT  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: MIRALESTE  
 Intersection Orientation: NS  
 Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street Movements	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	6	301	53	222	354	7
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	2	82	14	60	96	2
Hourly Flow Rate, HFR	6	327	57	241	384	7
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L		TR
Upstream Signal?	No			No		

Minor Street Movements	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	36	18	267	4	3	6
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	10	5	73	1	1	2
Hourly Flow Rate, HFR	39	19	290	4	3	6
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage				/ No /		
RT Channelized?	No					
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Pedestrian Volumes and Adjustments

Movements	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)	12.0	12.0	12.0	12.0
Walking Speed (ft/sec)	4.0	4.0	4.0	4.0
Percent Blockage	0	0	0	0

Upstream Signal Data							
	Prog. Flow vph	Sat Flow vph	Arrival Type	Green Time sec	Cycle Length sec	Prog. Speed mph	Distance to Signal feet
S2	Left-Turn						
	Through						
S5	Left-Turn						
	Through						

Worksheet 3-Data for Computing Effect of Delay to Major Street Vehicles

	Movement 2	Movement 5
Shared ln volume, major th vehicles:		327
Shared ln volume, major rt vehicles:		0
Sat flow rate, major th vehicles:		1700
Sat flow rate, major rt vehicles:		1700
Number of major street through lanes:		1

Worksheet 4-Critical Gap and Follow-up Time Calculation

Critical Gap Calculation									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
t(c,base)	4.1	4.1	7.1	6.5	6.2	7.1	6.5	6.2	
t(c,hv)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
P(hv)	0	0	0	0	0	0	0	0	
t(c,g)			0.20	0.20	0.10	0.20	0.20	0.10	
Grade/100			0.00	0.00	0.00	0.00	0.00	0.00	
t(3,lt)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
t(c,T):	1-stage	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
	2-stage	0.00	0.00	1.00	1.00	0.00	1.00	1.00	
t(c)	1-stage	4.1	4.1	7.1	6.5	7.1	6.5	6.2	
	2-stage	4.1	4.1	6.1	5.5	6.2	6.1	5.5	

Follow-Up Time Calculations									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
t(f,base)	2.20	2.20	3.50	4.00	3.30	3.50	4.00	3.30	
t(f,HV)	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	
P(HV)	0	0	0	0	0	0	0	0	
t(f)	2.2	2.2	3.5	4.0	3.3	3.5	4.0	3.3	

Worksheet 5-Effect of Upstream Signals

Computation 1-Queue Clearance Time at Upstream Signal				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
V prog				
Total Saturation Flow Rate, s (vph)				
Arrival Type				
Effective Green, g (sec)				
Cycle Length, C (sec)				
Rp (from Exhibit 16-11)				
Proportion vehicles arriving on green P				
g(q1)				
g(q2)				
g(q)				

Computation 2-Proportion of TWSC Intersection Time blocked				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
alpha				
beta				
Travel time, t(a) (sec)				
Smoothing Factor, F				
Proportion of conflicting flow, f				
Max platooned flow, V(c,max)				
Min platooned flow, V(c,min)				
Duration of blocked period, t(p)				
Proportion time blocked, p		0.000		0.000

Computation 3-Platoon Event Periods	Result
p(2)	0.000
p(5)	0.000
p(dom)	
p(subo)	
Constrained or unconstrained?	

Proportion unblocked for minor movements, p(x)	(1)	(2)	(3)
	Single-stage Process	Two-Stage Stage I	Process Stage II
p(1)			
p(4)			
p(7)			
p(8)			
p(9)			
p(10)			
p(11)			
p(12)			

Computation 4 and 5 Single-Stage Process									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
V c,x	391	384	1213	1212	327	1392	1266	388	
s									
Px									
V c,u,x									
C r,x									
C plat,x									

	7		8		10		11	
	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2
V(c,x)	339	874	339	873	870	522	870	396
s		1500		1500		1500		1500
P(x)								
V(c,u,x)								
C(r,x)								
C(plat,x)								

Worksheet 6-Impedance and Capacity Equations

Step 1: RT from Minor St.	9	12
Conflicting Flows	327	388
Potential Capacity	719	665
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	719	665
Probability of Queue free St.	0.60	0.99
Step 2: LT from Major St.	4	1
Conflicting Flows	384	391
Potential Capacity	1186	1179
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	1186	1179
Probability of Queue free St.	0.80	0.99
Maj L-Shared Prob Q free St.		0.99
Step 3: TH from Minor St.	8	11
Conflicting Flows	1212	1266
Potential Capacity	184	171
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.79	0.79
Movement Capacity	146	135
Probability of Queue free St.	0.93	0.99
Step 4: LT from Minor St.	7	10
Conflicting Flows	1213	1392
Potential Capacity	160	121
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.78	0.74
Maj. L, Min T Adj. Imp Factor.	0.83	0.80
Cap. Adj. factor due to Impeding mvmnt	0.82	0.47
Movement Capacity	132	57

Worksheet 7-Computation of the Effect of Two-stage Gap Acceptance

Step 3: TH from Minor St.	8	11
Part 1 - First Stage		
Conflicting Flows	339	870
Potential Capacity	643	372
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.99	0.80
Movement Capacity	639	296
Probability of Queue free St.	0.97	0.99
Part 2 - Second Stage		
Conflicting Flows	873	396
Potential Capacity	370	607
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.80	0.99
Movement Capacity	295	603
Part 3 - Single Stage		
Conflicting Flows	1212	1266
Potential Capacity	184	171
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.79	0.79
Movement Capacity	146	135
Result for 2 stage process:		
a	0.95	0.95

Y	3.45	0.71
C t	266	246
Probability of Queue free St.	0.93	0.99
Step 4: LT from Minor St.	7	10
Part 1 - First Stage		
Conflicting Flows	339	870
Potential Capacity	680	349
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.99	0.80
Movement Capacity	676	278
Part 2 - Second Stage		
Conflicting Flows	874	522
Potential Capacity	347	542
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.78	0.58
Movement Capacity	271	312
Part 3 - Single Stage		
Conflicting Flows	1213	1392
Potential Capacity	160	121
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.78	0.74
Maj. L, Min T Adj. Imp Factor.	0.83	0.80
Cap. Adj. factor due to Impeding mvmnt	0.82	0.47
Movement Capacity	132	57
Results for Two-stage process:		
a	0.95	0.95
Y	4.09	15.79
C t	246	67

Worksheet 8-Shared Lane Calculations

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (vph)	39	19	290	4	3	6
Movement Capacity (vph)	246	266	719	67	246	665
Shared Lane Capacity (vph)	252				161	

Worksheet 9-Computation of Effect of Flared Minor Street Approaches

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
C sep	246	266	719	67	246	665
Volume	39	19	290	4	3	6
Delay						
Q sep						
Q sep +1						
round (Qsep +1)						
n max						
C sh	252				161	
SUM C sep						
n						
C act						

Worksheet 10-Delay, Queue Length, and Level of Service

Movement	1	4	7	8	9	10	11	12
Lane Config	LT	L	LT		R		LTR	
v (vph)	6	241	58		290		13	
C(m) (vph)	1179	1186	252		719		161	
v/c	0.01	0.20	0.23		0.40		0.08	
95% queue length	0.02	0.76	0.87		1.96		0.26	
Control Delay	8.1	8.8	23.5		13.3		29.3	
LOS	A	A	C		B		D	
Approach Delay				15.0+			29.3	
Approach LOS				C			D	

Worksheet 11-Shared Major LT Impedance and Delay

	Movement 2	Movement 5
p(oj)	0.99	0.80
v(i1), Volume for stream 2 or 5	327	
v(i2), Volume for stream 3 or 6	0	
s(i1), Saturation flow rate for stream 2 or 5	1700	
s(i2), Saturation flow rate for stream 3 or 6	1700	
P*(oj)	0.99	
d(M,LT), Delay for stream 1 or 4	8.1	8.8
N, Number of major street through lanes	1	
d(rank,1) Delay for stream 2 or 5	0.1	



MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF RPV
WEEKDAY (2:00-4:00) PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

\*\*\*\*\*

Intersection #3 PALOS VERDES DRIVE EAST/PALOS VERDES DRIVE SOUTH

\*\*\*\*\*

Average Delay (sec/veh): 12.7 Worst Case Level Of Service: F[ 90.9]

\*\*\*\*\*

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, and Lanes.

Volume Module:

Table with 12 columns representing different traffic volumes and adjustments. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, and Final Volume.

Critical Gap Module:

Table with 12 columns for critical gap and follow-up time. Rows include Critical Gp and FollowUpTim.

Capacity Module:

Table with 12 columns for capacity and volume. Rows include Cnflct Vol, Potent Cap., Move Cap., and Volume/Cap.

Level Of Service Module:

Table with 12 columns for level of service. Rows include 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., Shared Queue, Shrd ConDel, Shared LOS, ApproachDel, and ApproachLOS.

Note: Queue reported is the number of cars per lane.

\*\*\*\*\*

HCS+: Unsignalized Intersections Release 5.21

TWO-WAY STOP CONTROL SUMMARY

Analyst: PM  
 Agency/Co.: RBF  
 Date Performed: 11-18-2009  
 Analysis Time Period: Afternoon Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 WITH PROJECT  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: MIRALESTE  
 Intersection Orientation: NS  
 Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	Northbound			Southbound		
	1 L	2 T	3 R	4 L	5 T	6 R
Volume	11	311	42	314	442	5
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	11	338	45	341	480	5
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L	TR	
Upstream Signal?	No			No		

Minor Street: Approach Movement	Westbound			Eastbound		
	7 L	8 T	9 R	10 L	11 T	12 R
Volume	52	20	239	1	2	14
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	56	21	259	1	2	15
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage				/ No /		
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Delay, Queue Length, and Level of Service

Approach Movement Lane Config	NB	SB	Westbound		Eastbound			
	1 LT	4 L	7 LT	8 R	9 R	10 L	11 LTR	12
v (vph)	11	341	77		259		18	
C(m) (vph)	1088	1187	148		709		0	
v/c	0.01	0.29	0.52		0.37			
95% queue length	0.03	1.20	2.53		1.68			
Control Delay	8.3	9.3	53.1		13.0			
LOS	A	A	F		B		F	
Approach Delay				22.2				
Approach LOS				C				

HCS+: Unsignalized Intersections Release 5.21

Phone:  
 Fax:  
 E-Mail:

TWO-WAY STOP CONTROL(TWSC) ANALYSIS

Analyst: PM  
 Agency/Co.: RBF  
 Date Performed: 11-18-2009  
 Analysis Time Period: Afternoon Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 WITH PROJECT  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: MIRALESTE  
 Intersection Orientation: NS  
 Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street Movements	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	11	311	42	314	442	5
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	3	85	11	85	120	1
Hourly Flow Rate, HFR	11	338	45	341	480	5
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L	TR	
Upstream Signal?	No			No		

Minor Street Movements	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	52	20	239	1	2	14
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	14	5	65	0	1	4
Hourly Flow Rate, HFR	56	21	259	1	2	15
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage				/ No /		
RT Channelized?	No					
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Pedestrian Volumes and Adjustments

Movements	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)	12.0	12.0	12.0	12.0
Walking Speed (ft/sec)	4.0	4.0	4.0	4.0
Percent Blockage	0	0	0	0

Upstream Signal Data							
	Prog. Flow vph	Sat Flow vph	Arrival Type	Green Time sec	Cycle Length sec	Prog. Speed mph	Distance to Signal feet
S2	Left-Turn						
	Through						
S5	Left-Turn						
	Through						

Worksheet 3-Data for Computing Effect of Delay to Major Street Vehicles

	Movement 2	Movement 5
Shared ln volume, major th vehicles:	338	
Shared ln volume, major rt vehicles:	0	
Sat flow rate, major th vehicles:	1700	
Sat flow rate, major rt vehicles:	1700	
Number of major street through lanes:	1	

Worksheet 4-Critical Gap and Follow-up Time Calculation

Critical Gap Calculation									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
t(c,base)	4.1	4.1	7.1	6.5	6.2	7.1	6.5	6.2	
t(c,hv)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
P(hv)	0	0	0	0	0	0	0	0	
t(c,g)			0.20	0.20	0.10	0.20	0.20	0.10	
Grade/100			0.00	0.00	0.00	0.00	0.00	0.00	
t(3,lt)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
t(c,T):	1-stage	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
	2-stage	0.00	0.00	1.00	1.00	0.00	1.00	1.00	
t(c)	1-stage	4.1	4.1	7.1	6.5	7.1	6.5	6.2	
	2-stage	4.1	4.1	6.1	5.5	6.2	6.1	5.5	

Follow-Up Time Calculations									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
t(f,base)	2.20	2.20	3.50	4.00	3.30	3.50	4.00	3.30	
t(f,HV)	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	
P(HV)	0	0	0	0	0	0	0	0	
t(f)	2.2	2.2	3.5	4.0	3.3	3.5	4.0	3.3	

Worksheet 5-Effect of Upstream Signals

Computation 1-Queue Clearance Time at Upstream Signal				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
V prog				
Total Saturation Flow Rate, s (vph)				
Arrival Type				
Effective Green, g (sec)				
Cycle Length, C (sec)				
Rp (from Exhibit 16-11)				
Proportion vehicles arriving on green P				
g(q1)				
g(q2)				
g(q)				

Computation 2-Proportion of TWSC Intersection Time blocked				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
alpha				
beta				
Travel time, t(a) (sec)				
Smoothing Factor, F				
Proportion of conflicting flow, f				
Max platooned flow, V(c,max)				
Min platooned flow, V(c,min)				
Duration of blocked period, t(p)				
Proportion time blocked, p		0.000		0.000

Computation 3-Platoon Event Periods	Result
p(2)	0.000
p(5)	0.000
p(dom)	
p(subo)	
Constrained or unconstrained?	

Proportion unblocked for minor movements, p(x)	(1)	(2)	(3)
	Single-stage Process	Two-Stage Stage I	Process Stage II
p(1)			
p(4)			
p(7)			
p(8)			
p(9)			
p(10)			
p(11)			
p(12)			

Computation 4 and 5 Single-Stage Process									
Movement	1	4	7	8	9	10	11	12	
	L	L	L	T	R	L	T	R	
V c,x	485	383	1533	1527	338	1686	1569	482	
s									
Px									
V c,u,x									
C r,x									
C plat,x									

Two-Stage Process								
	7		8		10		11	
	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2
V(c,x)	360	1173	360	1167	1164	522	1164	405
s		1500		1500		1500		1500
P(x)								
V(c,u,x)								
C(r,x)								
C(plat,x)								

Worksheet 6-Impedance and Capacity Equations

Step 1: RT from Minor St.	9	12
Conflicting Flows	338	482
Potential Capacity	709	588
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	709	588
Probability of Queue free St.	0.63	0.97
Step 2: LT from Major St.	4	1
Conflicting Flows	383	485
Potential Capacity	1187	1088
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	1187	1088
Probability of Queue free St.	0.71	0.99
Maj L-Shared Prob Q free St.		0.99
Step 3: TH from Minor St.	8	11
Conflicting Flows	1527	1569
Potential Capacity	119	112
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.70	0.70
Movement Capacity	84	79
Probability of Queue free St.	0.88	0.99
Step 4: LT from Minor St.	7	10
Conflicting Flows	1533	1686
Potential Capacity	96	75
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.69	0.62
Maj. L, Min T Adj. Imp Factor.	0.76	0.70
Cap. Adj. factor due to Impeding mvmnt	0.74	0.45
Movement Capacity	71	33

Worksheet 7-Computation of the Effect of Two-stage Gap Acceptance

Step 3: TH from Minor St.	8	11
Part 1 - First Stage		
Conflicting Flows	360	1164
Potential Capacity	630	271
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.99	0.71
Movement Capacity	622	193
Probability of Queue free St.	0.97	0.99
Part 2 - Second Stage		
Conflicting Flows	1167	405
Potential Capacity	270	602
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.71	0.99
Movement Capacity	192	594
Part 3 - Single Stage		
Conflicting Flows	1527	1569
Potential Capacity	119	112
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.70	0.70
Movement Capacity	84	79
Result for 2 stage process:		
a	0.95	0.95

Y	5.55	0.66
C t	169	161
Probability of Queue free St.	0.88	0.99
Step 4: LT from Minor St.	7	10
Part 1 - First Stage		
Conflicting Flows	360	1164
Potential Capacity	662	239
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.99	0.71
Movement Capacity	654	170
Part 2 - Second Stage		
Conflicting Flows	1173	522
Potential Capacity	236	542
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.69	0.61
Movement Capacity	162	328
Part 3 - Single Stage		
Conflicting Flows	1533	1686
Potential Capacity	96	75
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.69	0.62
Maj. L, Min T Adj. Imp Factor.	0.76	0.70
Cap. Adj. factor due to Impeding mvmnt	0.74	0.45
Movement Capacity	71	33
Results for Two-stage process:		
a	0.95	0.95
Y	7.29	-2.98
C t	142	0

Worksheet 8-Shared Lane Calculations

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (vph)	56	21	259	1	2	15
Movement Capacity (vph)	142	169	709	0	161	588
Shared Lane Capacity (vph)	148				0	

Worksheet 9-Computation of Effect of Flared Minor Street Approaches

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
C sep	142	169	709	0	161	588
Volume	56	21	259	1	2	15
Delay						
Q sep						
Q sep +1						
round (Qsep +1)						
n max						
C sh	148				0	
SUM C sep						
n						
C act						

Worksheet 10-Delay, Queue Length, and Level of Service

Movement	1	4	7	8	9	10	11	12
Lane Config	LT	L	LT		R		LTR	
v (vph)	11	341	77		259		18	
C(m) (vph)	1088	1187	148		709		0	
v/c	0.01	0.29	0.52		0.37			
95% queue length	0.03	1.20	2.53		1.68			
Control Delay	8.3	9.3	53.1		13.0			
LOS	A	A	F		B		F	
Approach Delay				22.2				
Approach LOS				C				

Worksheet 11-Shared Major LT Impedance and Delay

	Movement 2	Movement 5
p(oj)	0.99	0.71
v(i1), Volume for stream 2 or 5	338	
v(i2), Volume for stream 3 or 6	0	
s(i1), Saturation flow rate for stream 2 or 5	1700	
s(i2), Saturation flow rate for stream 3 or 6	1700	
P*(oj)	0.99	
d(M,LT), Delay for stream 1 or 4	8.3	9.3
N, Number of major street through lanes	1	
d(rank,1) Delay for stream 2 or 5	0.1	

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF REV
SATURDAY (11:00-1:00) PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #1 PALOS VERDES DRIVE EAST/MIRALESTE DRIVE
\*\*\*\*\*

Average Delay (sec/veh): 29.3 Worst Case Level Of Service: F[ 75.1]

Table with columns: Approach, North Bound, South Bound, East Bound, West Bound, Movement, Control, Rights, Lanes. Rows include Uncontrolled, Stop Sign, and lane configurations.

Volume Module:

Table with columns: Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume. Rows for North, South, East, West bounds.

Critical Gap Module:

Table with columns: Critical Gap, FollowUpTim. Rows for North, South, East, West bounds.

Capacity Module:

Table with columns: Cnflct Vol, Potent Cap., Move Cap., Volume/Cap. Rows for North, South, East, West bounds.

Level Of Service Module:

Table with columns: 2Way95thQ, Control Del, LOS by Move, Movement, Shared Cap., SharedQueue, Shrd ConDel, Shared LOS, ApproachDel, ApproachLOS. Rows for North, South, East, West bounds.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF REV
SATURDAY (11:00-1:00) PEAK HOUR

Level Of Service Computation Report

ICU l(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\*
Intersection #2 CREST DRIVE - COLLEGE ENTRANCE/PALOS VERDES DRIVE EAST
\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.329

Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 26 Level Of Service: A

Table with columns: Approach, North Bound, South Bound, East Bound, West Bound, Movement, Control, Rights, Lanes. Rows include Permitted, Include, and lane configurations.

Volume Module:

Table with columns: Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume. Rows for North, South, East, West bounds.

Saturation Flow Module:

Table with columns: Sat/Lane, Adjustment, Lanes, Final Sat. Rows for North, South, East, West bounds.

Capacity Analysis Module:

Table with columns: Vol/Sat, Crit Moves. Rows for North, South, East, West bounds.

\*\*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF REV
SATURDAY (11:00-1:00) PEAK HOUR

Level Of Service Computation Report

2000 HCM Unsignalized Method (Future Volume Alternative)

\*\*\*\*\*

Intersection #3 PALOS VERDES DRIVE EAST/PALOS VERDES DRIVE SOUTH

\*\*\*\*\*

Average Delay (sec/veh): 3.4 Worst Case Level Of Service: E[ 38.9]

\*\*\*\*\*

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled

Rights: Include Include Include Include

Lanes: 0 0 0 0 0 1 0 0 0 1 1 0 1 0 0 0 0 1 0 1

-----|-----|-----|-----|

Volume Module:

Table with 16 columns and 11 rows showing traffic volume metrics: Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Final Volume.

-----|-----|-----|-----|

Critical Gap Module:

Critical Gp:xxxxx xxxxx xxxxx 6.4 xxxxx 6.2 4.1 xxxxx xxxxxx xxxxx xxxxx xxxxxx

FollowUpTim:xxxxx xxxxx xxxxx 3.5 xxxxx 3.3 2.2 xxxxx xxxxxx xxxxx xxxxx xxxxxx

-----|-----|-----|-----|

Capacity Module:

Cnflct Vol: xxxxx xxxxx xxxxxx 1755 xxxxx 800 877 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Potent Cap.: xxxxx xxxxx xxxxxx 95 xxxxx 388 778 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Move Cap.: xxxxx xxxxx xxxxxx 85 xxxxx 388 778 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Volume/Cap: xxxxx xxxxx xxxxx 0.51 xxxxx 0.24 0.13 xxxxx xxxxx xxxxx xxxxx xxxxxx

-----|-----|-----|-----|

Level Of Service Module:

2Way95thQ: xxxxx xxxxx xxxxxx 2.2 xxxxx 0.9 0.4 xxxxx xxxxxx xxxxx xxxxx xxxxxx

Control Del:xxxxxx xxxxx xxxxxx 84.7 xxxxx 17.1 10.3 xxxxx xxxxxx xxxxxx xxxxx xxxxxx

LOS by Move: \* \* \* F \* C B \* \* \* \* \* \*

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx

SharedQueue:xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx

Shrd ConDel:xxxxxx xxxxx xxxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx xxxxx xxxxx xxxxxx

Shared LOS: \* \* \* \* \* \* \* \* \* \* \* \* \* \* \* \*

ApproachDel: xxxxxxxx 38.9 xxxxxxxx xxxxxxxx

ApproachLOS: \* E \*

\*\*\*\*\*

Note: Queue reported is the number of cars per lane.

\*\*\*\*\*

TWO-WAY STOP CONTROL SUMMARY

Analyst: PM  
 Agency/Co.: RBF  
 Date Performed: 11-18-2009  
 Analysis Time Period: Saturday Mid-day Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 WITH PROJECT  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: MIRALESTE  
 Intersection Orientation: NS Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	Northbound			Southbound		
	1 L	2 T	3 R	4 L	5 T	6 R
Volume	3	292	51	212	304	7
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	3	317	55	230	330	7
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L	TR	
Upstream Signal?	No			No		

Minor Street: Approach Movement	Westbound			Eastbound		
	7 L	8 T	9 R	10 L	11 T	12 R
Volume	0	8	162	0	4	0
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	0	8	176	0	4	0
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage				/ No /		
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Delay, Queue Length, and Level of Service

Approach Movement Lane Config	NB	SB	Westbound		Eastbound	
	1 LT	4 L	7 LT	8 R	9 R	10 LTR
v (vph)	3	230	8		176	4
C(m) (vph)	1234	1198	294		728	271
v/c	0.00	0.19	0.03		0.24	0.01
95% queue length	0.01	0.71	0.08		0.94	0.04
Control Delay	7.9	8.7	17.6		11.5	18.5
LOS	A	A	C		B	C
Approach Delay				11.8	18.5	
Approach LOS				B	C	

Phone:  
 Fax:  
 E-Mail:

TWO-WAY STOP CONTROL(TWSC) ANALYSIS

Analyst: PM  
 Agency/Co.: RBF  
 Date Performed: 11-18-2009  
 Analysis Time Period: Saturday Mid-day Peak Hour  
 Intersection: Miraleste/Via Colinita  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 WITH PROJECT  
 Project ID: Marymount  
 East/West Street: Via Colinita  
 North/South Street: MIRALESTE  
 Intersection Orientation: NS Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street Movements	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	3	292	51	212	304	7
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	1	79	14	58	83	2
Hourly Flow Rate, HFR	3	317	55	230	330	7
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type/Storage	Raised curb			/ 2		
RT Channelized?	No					
Lanes	0	1	1	1	1	0
Configuration	LT R			L	TR	
Upstream Signal?	No			No		

Minor Street Movements	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	0	8	162	0	4	0
Peak Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Peak-15 Minute Volume	0	2	44	0	1	0
Hourly Flow Rate, HFR	0	8	176	0	4	0
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage				/ No /		
RT Channelized?	No					
Lanes	0	1	1	0	1	0
Configuration	LT R			LTR		

Pedestrian Volumes and Adjustments

Movements	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)	12.0	12.0	12.0	12.0
Walking Speed (ft/sec)	4.0	4.0	4.0	4.0
Percent Blockage	0	0	0	0

Upstream Signal Data							
	Prog. Flow vph	Sat Flow vph	Arrival Type	Green Time sec	Cycle Length sec	Prog. Speed mph	Distance to Signal feet
S2	Left-Turn						
	Through						
S5	Left-Turn						
	Through						

Worksheet 3-Data for Computing Effect of Delay to Major Street Vehicles

	Movement 2	Movement 5
Shared ln volume, major th vehicles:	317	
Shared ln volume, major rt vehicles:	0	
Sat flow rate, major th vehicles:	1700	
Sat flow rate, major rt vehicles:	1700	
Number of major street through lanes:	1	

Worksheet 4-Critical Gap and Follow-up Time Calculation

Critical Gap Calculation								
Movement	1	4	7	8	9	10	11	12
	L	L	L	T	R	L	T	R
t(c,base)	4.1	4.1	7.1	6.5	6.2	7.1	6.5	6.2
t(c,hv)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
P(hv)	0	0	0	0	0	0	0	0
t(c,g)			0.20	0.20	0.10	0.20	0.20	0.10
Grade/100			0.00	0.00	0.00	0.00	0.00	0.00
t(3,lt)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
t(c,T):	1-stage 0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2-stage 0.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00
t(c)	1-stage 4.1	4.1	7.1	6.5	6.2	7.1	6.5	6.2
	2-stage 4.1	4.1	6.1	5.5	6.2	6.1	5.5	6.2

Follow-Up Time Calculations								
Movement	1	4	7	8	9	10	11	12
	L	L	L	T	R	L	T	R
t(f,base)	2.20	2.20	3.50	4.00	3.30	3.50	4.00	3.30
t(f,HV)	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
P(HV)	0	0	0	0	0	0	0	0
t(f)	2.2	2.2	3.5	4.0	3.3	3.5	4.0	3.3

Worksheet 5-Effect of Upstream Signals

Computation 1-Queue Clearance Time at Upstream Signal				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
V prog				
Total Saturation Flow Rate, s (vph)				
Arrival Type				
Effective Green, g (sec)				
Cycle Length, C (sec)				
Rp (from Exhibit 16-11)				
Proportion vehicles arriving on green P				
g(q1)				
g(q2)				
g(q)				

Computation 2-Proportion of TWSC Intersection Time blocked				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
alpha				
beta				
Travel time, t(a) (sec)				
Smoothing Factor, F				
Proportion of conflicting flow, f				
Max platooned flow, V(c,max)				
Min platooned flow, V(c,min)				
Duration of blocked period, t(p)				
Proportion time blocked, p		0.000		0.000

Computation 3-Platoon Event Periods	Result
p(2)	0.000
p(5)	0.000
p(dom)	
p(subo)	
Constrained or unconstrained?	

Proportion unblocked for minor movements, p(x)	(1)	(2)	(3)
	Single-stage Process	Two-Stage Stage I	Process Stage II
p(1)			
p(4)			
p(7)			
p(8)			
p(9)			
p(10)			
p(11)			
p(12)			

Computation 4 and 5 Single-Stage Process								
Movement	1	4	7	8	9	10	11	12
	L	L	L	T	R	L	T	R
V c,x	337	372	1119	1120	317	1236	1172	334
s								
Px								
V c,u,x								
C r,x								
C plat,x								

	7		8		10		11	
	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2
V(c,x)	323	796	323	797	794	442	794	378
s		1500		1500		1500		1500
P(x)								
V(c,u,x)								
C(r,x)								
C(plat,x)								

Worksheet 6-Impedance and Capacity Equations

Step 1: RT from Minor St.	9	12
Conflicting Flows	317	334
Potential Capacity	728	712
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	728	712
Probability of Queue free St.	0.76	1.00
Step 2: LT from Major St.	4	1
Conflicting Flows	372	337
Potential Capacity	1198	1234
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity	1198	1234
Probability of Queue free St.	0.81	1.00
Maj L-Shared Prob Q free St.		1.00
Step 3: TH from Minor St.	8	11
Conflicting Flows	1120	1172
Potential Capacity	208	194
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.81	0.81
Movement Capacity	168	156
Probability of Queue free St.	0.97	0.99
Step 4: LT from Minor St.	7	10
Conflicting Flows	1119	1236
Potential Capacity	186	154
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.79	0.78
Maj. L, Min T Adj. Imp Factor.	0.84	0.83
Cap. Adj. factor due to Impeding mvmnt	0.84	0.63
Movement Capacity	156	97

Worksheet 7-Computation of the Effect of Two-stage Gap Acceptance

Step 3: TH from Minor St.	8	11
Part 1 - First Stage		
Conflicting Flows	323	794
Potential Capacity	654	403
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	1.00	0.81
Movement Capacity	652	326
Probability of Queue free St.	0.99	0.99
Part 2 - Second Stage		
Conflicting Flows	797	378
Potential Capacity	401	619
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.81	1.00
Movement Capacity	324	617
Part 3 - Single Stage		
Conflicting Flows	1120	1172
Potential Capacity	208	194
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.81	0.81
Movement Capacity	168	156
Result for 2 stage process:		
a	0.95	0.95

Y	3.16	0.74
C t	294	271
Probability of Queue free St.	0.97	0.99
Step 4: LT from Minor St.	7	10
Part 1 - First Stage		
Conflicting Flows	323	794
Potential Capacity	693	384
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	1.00	0.81
Movement Capacity	691	310
Part 2 - Second Stage		
Conflicting Flows	796	442
Potential Capacity	383	598
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.80	0.75
Movement Capacity	306	447
Part 3 - Single Stage		
Conflicting Flows	1119	1236
Potential Capacity	186	154
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.79	0.78
Maj. L, Min T Adj. Imp Factor.	0.84	0.83
Cap. Adj. factor due to Impeding mvmnt	0.84	0.63
Movement Capacity	156	97
Results for Two-stage process:		
a	0.95	0.95
Y	3.64	1.77
C t	280	187

Worksheet 8-Shared Lane Calculations

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (vph)	0	8	176	0	4	0
Movement Capacity (vph)	280	294	728	187	271	712
Shared Lane Capacity (vph)	294				271	

Worksheet 9-Computation of Effect of Flared Minor Street Approaches

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
C sep	280	294	728	187	271	712
Volume	0	8	176	0	4	0
Delay						
Q sep						
Q sep +1						
round (Qsep +1)						
n max						
C sh	294				271	
SUM C sep						
n						
C act						

Worksheet 10-Delay, Queue Length, and Level of Service

Movement	1	4	7	8	9	10	11	12
Lane Config	LT	L	LT		R		LTR	
v (vph)	3	230	8		176		4	
C(m) (vph)	1234	1198	294		728		271	
v/c	0.00	0.19	0.03		0.24		0.01	
95% queue length	0.01	0.71	0.08		0.94		0.04	
Control Delay	7.9	8.7	17.6		11.5		18.5	
LOS	A	A	C		B		C	
Approach Delay				11.8			18.5	
Approach LOS				B			C	

Worksheet 11-Shared Major LT Impedance and Delay

	Movement 2	Movement 5
p(oj)	1.00	0.81
v(i1), Volume for stream 2 or 5	317	
v(i2), Volume for stream 3 or 6	0	
s(i1), Saturation flow rate for stream 2 or 5	1700	
s(i2), Saturation flow rate for stream 3 or 6	1700	
P*(oj)	1.00	
d(M,LT), Delay for stream 1 or 4	7.9	8.7
N, Number of major street through lanes	1	
d(rank,1) Delay for stream 2 or 5	0.0	

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF LOS ANGELES AM PEAK HOUR

Level of Service Computation Report
Circular 212 Planning Method (Future Volume Alternative)
Intersection #7 WESTERN AVENUE/CAPITOL DRIVE
Cycle (sec): 100 Critical Vol./Cap.(X): 1.097
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Protected Protected Permitted Permitted
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Lanes: 1 0 2 0 1 1 0 2 0 1 0 1 0 0 1 1 0 1 0 1
Volume Module:
Base Vol: 13 1332 208 132 907 85 132 74 44 190 53 165
Growth Adj: 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04
Initial Bse: 14 1391 217 138 947 89 138 77 46 198 55 172
Added Vol: 0 5 3 0 16 49 16 5 0 8 16 0
Cumulative: 1 96 39 52 113 2 3 9 0 48 7 63
Initial Fut: 15 1492 259 190 1076 140 157 91 46 254 78 235
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 16 1621 282 206 1169 152 170 99 50 276 85 256
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 16 1621 282 206 1169 152 170 99 50 276 85 256
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 16 1621 282 206 1169 152 170 99 50 276 85 256
Saturation Flow Module:
Sat/Lane: 1425 1425 1425 1425 1425 1425 1425 1425 1425 1425 1425 1425
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 1.00 2.00 1.00 1.00 2.00 1.00 0.63 0.37 1.00 1.00 1.00 1.00
Final Sat.: 1425 2850 1425 1425 2850 1425 901 524 1425 1425 1425 1425
Capacity Analysis Module:
Vol/Sat: 0.01 0.57 0.20 0.14 0.41 0.11 0.19 0.19 0.04 0.19 0.06 0.18
Crit Volume: 811 206 270 276
Crit Moves: \*\*\*\* \*\*\*\* \*\*\*\* \*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF LOS ANGELES AM PEAK HOUR

Level of Service Computation Report
Circular 212 Planning Method (Future Volume Alternative)
Intersection #8 WESTERN AVENUE/CRESTWOOD STREET
Cycle (sec): 100 Critical Vol./Cap.(X): 0.861
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 143 Level Of Service: D
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Permitted Permitted Split Phase Split Phase
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Lanes: 1 0 2 0 1 1 0 2 0 1 1 0 0 1 0 1 0 0 1 0
Volume Module:
Base Vol: 93 1394 149 23 1107 128 179 31 104 83 10 25
Growth Adj: 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04
Initial Bse: 97 1455 156 24 1156 134 187 32 109 87 10 26
Added Vol: 0 8 0 0 24 0 0 0 0 0 0 0
Cumulative: 23 86 15 15 98 25 26 8 23 15 7 15
Initial Fut: 120 1549 171 39 1278 159 213 40 132 102 17 41
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 131 1684 185 42 1389 172 231 44 143 110 19 45
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 131 1684 185 42 1389 172 231 44 143 110 19 45
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 131 1684 185 42 1389 172 231 44 143 110 19 45
Saturation Flow Module:
Sat/Lane: 1425 1425 1425 1425 1425 1425 1425 1425 1425 1425 1425 1425
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 1.00 2.00 1.00 1.00 2.00 1.00 1.00 0.23 0.77 1.00 0.30 0.70
Final Sat.: 1425 2850 1425 1425 2850 1425 1425 335 1090 1425 425 1000
Capacity Analysis Module:
Vol/Sat: 0.09 0.59 0.13 0.03 0.49 0.12 0.16 0.13 0.13 0.08 0.04 0.04
Crit Volume: 842 42 231 110
Crit Moves: \*\*\*\* \*\*\*\* \*\*\*\* \*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF LOS ANGELES
AM PEAK HOUR

Level Of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

Intersection #9 WESTERN AVENUE/1ST STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 1.465
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module:

Table with 12 columns and 14 rows showing traffic volume data for various approaches and movements.

Saturation Flow Module:

Table with 12 columns and 4 rows showing saturation flow and adjustment factors.

Capacity Analysis Module:

Table with 12 columns and 4 rows showing capacity analysis results.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF LOS ANGELES
AM PEAK HOUR

Level Of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

Intersection #10 WESTERN AVENUE/9TH STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.661
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 59 Level Of Service: B

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 4 rows: Movement, Control, Rights, Min. Green, Lanes.

Volume Module:

Table with 12 columns and 14 rows showing traffic volume data for various approaches and movements.

Saturation Flow Module:

Table with 12 columns and 4 rows showing saturation flow and adjustment factors.

Capacity Analysis Module:

Table with 12 columns and 4 rows showing capacity analysis results.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF LOS ANGELES
AM PEAK HOUR

Level Of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

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Intersection #11 WESTERN AVENUE/WEST 25TH STREET

\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.833
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 120 Level Of Service: D

\*\*\*\*\*

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 3 rows: Movement, Control, Rights, Min. Green, Lanes.

-----|-----|-----|-----|

Volume Module: Table with 12 columns and 14 rows showing traffic volume data for various scenarios.

-----|-----|-----|-----|

Saturation Flow Module: Table with 12 columns and 4 rows showing saturation flow data.

-----|-----|-----|-----|

Capacity Analysis Module: Table with 12 columns and 4 rows showing capacity analysis data.

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MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF LOS ANGELES PM PEAK HOUR

Level Of Service Computation Report Circular 212 Planning Method (Future Volume Alternative) Intersection #7 WESTERN AVENUE/CAPITOL DRIVE. Table with columns for Approach, Movement, Control, Rights, Min. Green, Lanes, Volume Module, Saturation Flow Module, Capacity Analysis Module.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF LOS ANGELES PM PEAK HOUR

Level Of Service Computation Report Circular 212 Planning Method (Future Volume Alternative) Intersection #8 WESTERN AVENUE/CRESTWOOD STREET. Table with columns for Approach, Movement, Control, Rights, Min. Green, Lanes, Volume Module, Saturation Flow Module, Capacity Analysis Module.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF LOS ANGELES PM PEAK HOUR

Level Of Service Computation Report Circular 212 Planning Method (Future Volume Alternative) Intersection #9 WESTERN AVENUE/1ST STREET. Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Cycle, Loss Time, Optimal Cycle, Control, Rights, Min. Green, Lanes, Volume Module, Saturation Flow Module, and Capacity Analysis Module.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF LOS ANGELES PM PEAK HOUR

Level Of Service Computation Report Circular 212 Planning Method (Future Volume Alternative) Intersection #10 WESTERN AVENUE/9TH STREET. Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Cycle, Loss Time, Optimal Cycle, Control, Rights, Min. Green, Lanes, Volume Module, Saturation Flow Module, and Capacity Analysis Module.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF LOS ANGELES
PM PEAK HOUR

Level Of Service Computation Report

Circular 212 Planning Method (Future Volume Alternative)

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Intersection #11 WESTERN AVENUE/WEST 25TH STREET

\*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.818
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 110 Level Of Service: D

\*\*\*\*\*

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and 3 rows: Movement, Control, Rights, Min. Green, Lanes.

-----|-----|-----|-----|

Volume Module: Table with 12 columns and 14 rows showing traffic volume metrics like Base Vol, Growth Adj, Initial Bse, etc.

-----|-----|-----|-----|

Saturation Flow Module: Table with 12 columns and 4 rows showing saturation flow metrics like Sat/Lane, Adjustment, Lanes, Final Sat.

-----|-----|-----|-----|

Capacity Analysis Module: Table with 12 columns and 4 rows showing capacity metrics like Vol/Sat, Crit Volume, Crit Moves.

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MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CALTRANS
AM PEAK HOUR

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #6 WESTERN AVENUE/TOSCANINI DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 0.761
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 18.3
Optimal Cycle: 63 Level Of Service: B

Table with 4 columns: Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Table with 12 columns for Volume Module. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Table with 12 columns for Saturation Flow Module. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Table with 12 columns for Capacity Analysis Module. Rows include Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CALTRANS
AM PEAK HOUR

Level Of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #7 WESTERN AVENUE/CAPITOL DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 0.994
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 38.2
Optimal Cycle: 180 Level Of Service: D

Table with 4 columns: Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Table with 12 columns for Volume Module. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Table with 12 columns for Saturation Flow Module. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Table with 12 columns for Capacity Analysis Module. Rows include Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CALTRANS AM PEAK HOUR

Level Of Service Computation Report 2000 HCM Operations Method (Future Volume Alternative) Intersection #8 WESTERN AVENUE/CRESTWOOD STREET. Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Cycle, Loss Time, Optimal Cycle, Control, Rights, Min. Green, Lanes, Volume Module, Saturation Flow Module, and Capacity Analysis Module.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CALTRANS AM PEAK HOUR

Level Of Service Computation Report 2000 HCM Operations Method (Future Volume Alternative) Intersection #9 WESTERN AVENUE/1ST STREET. Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Cycle, Loss Time, Optimal Cycle, Control, Rights, Min. Green, Lanes, Volume Module, Saturation Flow Module, and Capacity Analysis Module.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CALTRANS AM PEAK HOUR

Level Of Service Computation Report 2000 HCM Operations Method (Future Volume Alternative)

Intersection #10 WESTERN AVENUE/9TH STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.553
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 22.3
Optimal Cycle: 40 Level Of Service: C

Table with 4 columns: Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module table with 12 columns for traffic flows and 12 rows for metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module table with 12 columns for traffic flows and 4 rows for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module table with 12 columns for traffic flows and 12 rows for Vol/Sat, Crit Moves, Green/Cycle, etc.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CALTRANS AM PEAK HOUR

Level Of Service Computation Report 2000 HCM Operations Method (Future Volume Alternative)

Intersection #11 WESTERN AVENUE/WEST 25TH STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.742
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 27.7
Optimal Cycle: 60 Level Of Service: C

Table with 4 columns: Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module table with 12 columns for traffic flows and 12 rows for metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module table with 12 columns for traffic flows and 4 rows for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module table with 12 columns for traffic flows and 12 rows for Vol/Sat, Crit Moves, Green/Cycle, etc.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CALTRANS PM PEAK HOUR

Level Of Service Computation Report 2000 HCM Operations Method (Future Volume Alternative)

\*\*\*\*\* Intersection #6 WESTERN AVENUE/TOSCANINI DRIVE \*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.658
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 10.3
Optimal Cycle: 49 Level Of Service: B

Table with 4 columns: Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module table with 12 columns for traffic volume and 12 rows for various metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module table with 12 columns for saturation flow and 4 rows for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module table with 12 columns for capacity and 12 rows for Vol/Sat, Crit Moves, Green/Cycle, etc.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CALTRANS PM PEAK HOUR

Level Of Service Computation Report 2000 HCM Operations Method (Future Volume Alternative)

\*\*\*\*\* Intersection #7 WESTERN AVENUE/CAPITOL DRIVE \*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.978
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 40.2
Optimal Cycle: 168 Level Of Service: D

Table with 4 columns: Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Control, Rights, Min. Green, and Lanes.

Volume Module table with 12 columns for traffic volume and 12 rows for various metrics like Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module table with 12 columns for saturation flow and 4 rows for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module table with 12 columns for capacity and 12 rows for Vol/Sat, Crit Moves, Green/Cycle, etc.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CALTRANS PM PEAK HOUR

Level Of Service Computation Report 2000 HCM Operations Method (Future Volume Alternative) Intersection #8 WESTERN AVENUE/CRESTWOOD STREET. Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Cycle, Loss Time, Optimal Cycle, Control, Rights, Min. Green, Lanes, Volume Module, Saturation Flow Module, and Capacity Analysis Module.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CALTRANS PM PEAK HOUR

Level Of Service Computation Report 2000 HCM Operations Method (Future Volume Alternative) Intersection #9 WESTERN AVENUE/1ST STREET. Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Cycle, Loss Time, Optimal Cycle, Control, Rights, Min. Green, Lanes, Volume Module, Saturation Flow Module, and Capacity Analysis Module.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CALTRANS PM PEAK HOUR

Level Of Service Computation Report 2000 HCM Operations Method (Future Volume Alternative) Intersection #10 WESTERN AVENUE/9TH STREET. Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Cycle, Loss Time, Optimal Cycle, Control, Rights, Min. Green, Lanes, Volume Module, Sat/Lane, Adjustment, Lanes, Final Sat., Capacity Analysis Module, Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CALTRANS PM PEAK HOUR

Level Of Service Computation Report 2000 HCM Operations Method (Future Volume Alternative) Intersection #11 WESTERN AVENUE/WEST 25TH STREET. Table with columns for Approach (North, South, East, West Bound) and Movement (L, T, R). Rows include Cycle, Loss Time, Optimal Cycle, Control, Rights, Min. Green, Lanes, Volume Module, Sat/Lane, Adjustment, Lanes, Final Sat., Capacity Analysis Module, Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CMP
AM PEAK HOUR

Level Of Service Computation Report

ICU l(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #6 WESTERN AVENUE/TOSCANINI DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 0.908
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 104 Level Of Service: E

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Includes Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns for traffic volume metrics: Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume.

Saturation Flow Module:

Table with 12 columns for saturation flow metrics: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with 12 columns for capacity analysis metrics: Vol/Sat, Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CMP
AM PEAK HOUR

Level Of Service Computation Report

ICU l(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #10 WESTERN AVENUE/9TH STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.689
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 49 Level Of Service: B

Table with 4 columns: Approach (North Bound, South Bound, East Bound, West Bound) and Movement (L, T, R). Includes Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns for traffic volume metrics: Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume.

Saturation Flow Module:

Table with 12 columns for saturation flow metrics: Sat/Lane, Adjustment, Lanes, Final Sat.

Capacity Analysis Module:

Table with 12 columns for capacity analysis metrics: Vol/Sat, Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CMP PM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #6 WESTERN AVENUE/TOSCANINI DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 0.799
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 66 Level Of Service: C

Table with 4 columns: Approach (North, South, East, West Bound), Movement (L, T, R), Control (Protected, Permitted), and Lanes.

Volume Module:

Table with 12 columns for traffic volume and 12 columns for adjustment factors (Growth, Initial, Added, Cumulative, User, PHF, PCE, MLF, Final).

Saturation Flow Module:

Table with 12 columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns for Vol/Sat and Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CMP PM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #10 WESTERN AVENUE/9TH STREET

Cycle (sec): 100 Critical Vol./Cap.(X): 0.873
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 88 Level Of Service: D

Table with 4 columns: Approach (North, South, East, West Bound), Movement (L, T, R), Control (Protected, Permitted), and Lanes.

Volume Module:

Table with 12 columns for traffic volume and 12 columns for adjustment factors (Growth, Initial, Added, Cumulative, User, PHF, PCE, MLF, Final).

Saturation Flow Module:

Table with 12 columns for Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns for Vol/Sat and Crit Moves.

**Mitigated Forecast Year 2012  
With Project Conditions**

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) MITIGATED FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF RPV AM PEAK HOUR

Level of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #1 PALOS VERDES DRIVE EAST/MIRALESTE DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 0.963
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 146 Level Of Service: E

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns representing traffic volumes and adjustments. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume, and OvlAdjVol.

Saturation Flow Module:

Table with 12 columns representing saturation flow values. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns representing capacity analysis values. Rows include Vol/Sat, OvlAdjV/S, and Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) MITIGATED FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF RPV AM PEAK HOUR

Level of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #7 WESTERN AVENUE/CAPITOL DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 1.002
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns representing traffic volumes and adjustments. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume, and OvlAdjVol.

Saturation Flow Module:

Table with 12 columns representing saturation flow values. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns representing capacity analysis values. Rows include Vol/Sat, Crit Moves.

HCS+: Unsignalized Intersections Release 5.21

TWO-WAY STOP CONTROL SUMMARY

Analyst: PM  
 Agency/Co.: RBF  
 Date Performed: 11-18-2009  
 Analysis Time Period: AM Peak Hour  
 Intersection: PV Dr East/PV Dr South  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 With Project  
 Project ID: Marymount  
 East/West Street: Palos Verdes Dr South  
 North/South Street: Palos Verdes Drive East  
 Intersection Orientation: EW Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	Eastbound			Westbound		
	1 L	2 T	3 R	4 L	5 T	6 R
Volume	206	520			593	85
Peak-Hour Factor, PHF	0.92	0.92			0.92	0.92
Hourly Flow Rate, HFR	223	565			644	92
Percent Heavy Vehicles	0	--	--		--	--
Median Type/Storage	Raised curb			/ 1		
RT Channelized?				No		
Lanes	1	1			1	1
Configuration	L	T			T	R
Upstream Signal?	No			No		

Minor Street: Approach Movement	Northbound			Southbound		
	7 L	8 T	9 R	10 L	11 T	12 R
Volume				41		89
Peak Hour Factor, PHF				0.92		0.92
Hourly Flow Rate, HFR				44		96
Percent Heavy Vehicles				0		0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage				/		
Lanes				1		1
Configuration				L		R

Delay, Queue Length, and Level of Service

Approach Movement Lane Config	EB	WB	Northbound			Southbound			
	1 L	4 	7 	8	9 	10 	11 	12 R	
v (vph)	223					44		96	
C(m) (vph)	879					193		476	
v/c	0.25					0.23		0.20	
95% queue length	1.01					0.85		0.75	
Control Delay	10.5					29.1		14.5	
LOS	B					D		B	
Approach Delay							19.1		
Approach LOS							C		

HCS+: Unsignalized Intersections Release 5.21

Phone:  
 E-Mail:  
 Fax:

TWO-WAY STOP CONTROL(TWSC) ANALYSIS

Analyst: PM  
 Agency/Co.: RBF  
 Date Performed: 11-18-2009  
 Analysis Time Period: AM Peak Hour  
 Intersection: PV Dr East/PV Dr South  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 With Project  
 Project ID: Marymount  
 East/West Street: Palos Verdes Dr South  
 North/South Street: Palos Verdes Drive East  
 Intersection Orientation: EW Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street Movements	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	206	520			593	85
Peak-Hour Factor, PHF	0.92	0.92			0.92	0.92
Peak-15 Minute Volume	56	141			161	23
Hourly Flow Rate, HFR	223	565			644	92
Percent Heavy Vehicles	0	--	--		--	--
Median Type/Storage	Raised curb			/ 1		
RT Channelized?				No		
Lanes	1	1			1	1
Configuration	L	T			T	R
Upstream Signal?	No			No		

Minor Street Movements	7	8	9	10	11	12
	L	T	R	L	T	R
Volume				41		89
Peak Hour Factor, PHF				0.92		0.92
Peak-15 Minute Volume				11		24
Hourly Flow Rate, HFR				44		96
Percent Heavy Vehicles				0		0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage				/		
RT Channelized?				No		
Lanes				1		1
Configuration				L		R

Minor Street Movements	7	8	9	10	11	12
	L	T	R	L	T	R
Volume				41		89
Peak Hour Factor, PHF				0.92		0.92
Peak-15 Minute Volume				11		24
Hourly Flow Rate, HFR				44		96
Percent Heavy Vehicles				0		0
Percent Grade (%)	0			0		
Flared Approach: Exists?/Storage				/		
RT Channelized?				No		
Lanes				1		1
Configuration				L		R

Pedestrian Volumes and Adjustments

Movements	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)	12.0	12.0	12.0	12.0
Walking Speed (ft/sec)	4.0	4.0	4.0	4.0
Percent Blockage	0	0	0	0

Upstream Signal Data							
	Prog. Flow vph	Sat Flow vph	Arrival Type	Green Time sec	Cycle Length sec	Prog. Speed mph	Distance to Signal feet
S2	Left-Turn						
	Through						
S5	Left-Turn						
	Through						

Worksheet 3-Data for Computing Effect of Delay to Major Street Vehicles

	Movement 2	Movement 5
Shared ln volume, major th vehicles:		
Shared ln volume, major rt vehicles:		
Sat flow rate, major th vehicles:		
Sat flow rate, major rt vehicles:		
Number of major street through lanes:		

Worksheet 4-Critical Gap and Follow-up Time Calculation

Critical Gap Calculation								
Movement	1	4	7	8	9	10	11	12
	L	L	L	T	R	L	T	R
t(c,base)	4.1					7.1		6.2
t(c,hv)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
P(hv)	0					0		0
t(c,g)			0.20	0.20	0.10	0.20	0.20	0.10
Grade/100			0.00	0.00	0.00	0.00	0.00	0.00
t(3,lt)	0.00					0.70		0.00
t(c,T): 1-stage	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2-stage	0.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00
t(c) 1-stage	4.1					6.4		6.2
2-stage	4.1					5.4		6.2

Follow-Up Time Calculations								
Movement	1	4	7	8	9	10	11	12
	L	L	L	T	R	L	T	R
t(f,base)	2.20					3.50		3.30
t(f,HV)	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
P(HV)	0					0		0
t(f)	2.2					3.5		3.3

Worksheet 5-Effect of Upstream Signals

Computation 1-Queue Clearance Time at Upstream Signal				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
V prog				
Total Saturation Flow Rate, s (vph)				
Arrival Type				
Effective Green, g (sec)				
Cycle Length, C (sec)				
Rp (from Exhibit 16-11)				
Proportion vehicles arriving on green P				
g(q1)				
g(q2)				
g(q)				

Computation 2-Proportion of TWSC Intersection Time blocked				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
alpha				
beta				
Travel time, t(a) (sec)				
Smoothing Factor, F				
Proportion of conflicting flow, f				
Max platooned flow, V(c,max)				
Min platooned flow, V(c,min)				
Duration of blocked period, t(p)				
Proportion time blocked, p		0.000		0.000

Computation 3-Platoon Event Periods	Result
p(2)	0.000
p(5)	0.000
p(dom)	
p(subo)	
Constrained or unconstrained?	

Proportion unblocked for minor movements, p(x)	(1) Single-stage Process	(2) Two-Stage Stage I	(3) Process Stage II
p(1)			
p(4)			
p(7)			
p(8)			
p(9)			
p(10)			
p(11)			
p(12)			

Computation 4 and 5 Single-Stage Process								
Movement	1	4	7	8	9	10	11	12
	L	L	L	T	R	L	T	R
V c,x	736					1655		644
s								
Px								
V c,u,x								
C r,x								
C plat,x								

Two-Stage Process								
	7		8		10		11	
	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2
V(c,x)					644	1011		
s						1500		
P(x)								
V(c,u,x)								
C(r,x)								
C(plat,x)								

Worksheet 6-Impedance and Capacity Equations

Step 1: RT from Minor St.	9	12
Conflicting Flows		644
Potential Capacity		476
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity		476
Probability of Queue free St.	1.00	0.80
Step 2: LT from Major St.	4	1
Conflicting Flows		736
Potential Capacity		879
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity		879
Probability of Queue free St.	1.00	0.75
Maj L-Shared Prob Q free St.		
Step 3: TH from Minor St.	8	11
Conflicting Flows		
Potential Capacity		
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.75	0.75
Movement Capacity		
Probability of Queue free St.	1.00	1.00
Step 4: LT from Minor St.	7	10
Conflicting Flows		1655
Potential Capacity		109
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.75	
Maj. L, Min T Adj. Imp Factor.	0.80	
Cap. Adj. factor due to Impeding mvmnt	0.64	0.75
Movement Capacity		81

Worksheet 7-Computation of the Effect of Two-stage Gap Acceptance

Step 3: TH from Minor St.	8	11
Part 1 - First Stage		
Conflicting Flows		
Potential Capacity	320	471
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.75	1.00
Movement Capacity	239	471
Probability of Queue free St.	1.00	1.00
Part 2 - Second Stage		
Conflicting Flows		
Potential Capacity	428	320
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	1.00	0.75
Movement Capacity	428	239
Part 3 - Single Stage		
Conflicting Flows		
Potential Capacity		
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.75	0.75
Movement Capacity		
Result for 2 stage process:		
a	0.91	0.91

Y		
C t		
Probability of Queue free St.	1.00	1.00
Step 4: LT from Minor St.	7	10
Part 1 - First Stage		
Conflicting Flows		644
Potential Capacity	355	527
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.75	1.00
Movement Capacity	265	527
Part 2 - Second Stage		
Conflicting Flows		1011
Potential Capacity	476	355
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.80	0.75
Movement Capacity	380	265
Part 3 - Single Stage		
Conflicting Flows		1655
Potential Capacity		109
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.75	
Maj. L, Min T Adj. Imp Factor.	0.80	
Cap. Adj. factor due to Impeding mvmnt	0.64	0.75
Movement Capacity		81
Results for Two-stage process:		
a	0.91	0.91
Y		2.42
C t		193

Worksheet 8-Shared Lane Calculations

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (vph)				44		96
Movement Capacity (vph)				193		476
Shared Lane Capacity (vph)						

Worksheet 9-Computation of Effect of Flared Minor Street Approaches

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
C sep				193		476
Volume				44		96
Delay						
Q sep						
Q sep +1						
round (Qsep +1)						
n max						
C sh						
SUM C sep						
n						
C act						

Worksheet 10-Delay, Queue Length, and Level of Service

Movement	1	4	7	8	9	10	11	12
Lane Config	L					L		R
v (vph)	223					44		96
C(m) (vph)	879					193		476
v/c	0.25					0.23		0.20
95% queue length	1.01					0.85		0.75
Control Delay	10.5					29.1		14.5
LOS	B					D		B
Approach Delay							19.1	
Approach LOS							C	

Worksheet 11-Shared Major LT Impedance and Delay

	Movement 2	Movement 5
p(oj)	0.75	1.00
v(i1), Volume for stream 2 or 5		
v(i2), Volume for stream 3 or 6		
s(i1), Saturation flow rate for stream 2 or 5		
s(i2), Saturation flow rate for stream 3 or 6		
P*(oj)		
d(M,LT), Delay for stream 1 or 4	10.5	
N, Number of major street through lanes		
d(rank,1) Delay for stream 2 or 5		

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) MITIGATED FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF RPV PM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\* Intersection #1 PALOS VERDES DRIVE EAST/MIRALESTE DRIVE \*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 0.896
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 98 Level Of Service: D

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Protected Protected Protected Protected
Rights: Include Include Include Ovl

Volume Module:
Base Vol: 0 138 157 419 200 0 0 0 0 293 0 351
Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 0 138 157 419 200 0 0 0 0 293 0 351
Added Vol: 0 10 63 0 9 0 0 0 0 61 0 0
Cumulative: 0 0 10 10 1 0 0 0 0 10 0 10
Initial Fut: 0 148 230 429 210 0 0 0 0 364 0 361
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 0 161 250 466 228 0 0 0 0 396 0 392
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 0 161 250 466 228 0 0 0 0 396 0 392
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 0 161 250 466 228 0 0 0 0 396 0 392
OvlAdjVol: 0

Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 0.00 0.39 0.61 1.00 1.00 0.00 0.00 0.00 0.00 1.00 0.00 1.00
Final Sat.: 0 626 974 1600 1600 0 0 0 0 1600 0 1600

Capacity Analysis Module:
Vol/Sat: 0.00 0.26 0.26 0.29 0.14 0.00 0.00 0.00 0.00 0.25 0.00 0.25
OvlAdjV/S: 0.00
Crit Moves: \*\*\*\*

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089) MITIGATED FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF RPV PM PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

\*\*\*\*\* Intersection #7 WESTERN AVENUE/CAPITOL DRIVE \*\*\*\*\*

Cycle (sec): 100 Critical Vol./Cap.(X): 1.011
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F

Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R L - T - R
Control: Protected Protected Permitted Permitted
Rights: Include Include Include Include
Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0
Lanes: 1 0 2 0 1 1 0 2 0 1 1 0 0 1 0 1 0 1 0 1

Volume Module:
Base Vol: 75 1098 86 185 1294 50 76 65 30 153 104 162
Growth Adj: 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04 1.04
Initial Bse: 78 1146 90 193 1351 52 79 68 31 160 109 169
Added Vol: 0 10 5 0 9 28 29 10 0 5 9 0
Cumulative: 4 155 75 97 82 1 1 22 0 99 20 63
Initial Fut: 82 1311 170 290 1442 81 109 100 31 264 138 232
User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92
PHF Volume: 89 1425 185 315 1567 88 119 109 34 287 150 252
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0
Reduced Vol: 89 1425 185 315 1567 88 119 109 34 287 150 252
PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
FinalVolume: 89 1425 185 315 1567 88 119 109 34 287 150 252

Saturation Flow Module:
Sat/Lane: 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600 1600
Adjustment: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00
Lanes: 1.00 2.00 1.00 1.00 2.00 1.00 1.00 0.76 0.24 1.00 1.00 1.00
Final Sat.: 1600 3200 1600 1600 3200 1600 1600 1218 382 1600 1600 1600

Capacity Analysis Module:
Vol/Sat: 0.06 0.45 0.12 0.20 0.49 0.06 0.07 0.09 0.09 0.18 0.09 0.16
Crit Moves: \*\*\*\*

HCS+: Unsignalized Intersections Release 5.21

TWO-WAY STOP CONTROL SUMMARY

Analyst: PM  
 Agency/Co.: RBF  
 Date Performed: 11-18-2009  
 Analysis Time Period: PM PEAK HOUR  
 Intersection: PV Dr East/PV Dr South  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 With Project  
 Project ID:  
 East/West Street: Palos Verdes Dr South  
 North/South Street: Palos Verdes Drive East  
 Intersection Orientation: EW Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	Eastbound			Westbound		
	1 L	2 T	3 R	4 L	5 T	6 R
Volume	123	722			595	68
Peak-Hour Factor, PHF	0.92	0.92			0.92	0.92
Hourly Flow Rate, HFR	133	784			646	73
Percent Heavy Vehicles	0	--	--		--	--
Median Type/Storage	Raised curb			/ 1		
RT Channelized?				No		
Lanes	1	1			1	1
Configuration	L	T			T	R
Upstream Signal?	No			No		

Minor Street: Approach Movement	Northbound			Southbound		
	7 L	8 T	9 R	10 L	11 T	12 R
Volume				60		119
Peak Hour Factor, PHF				0.92		0.92
Hourly Flow Rate, HFR				65		129
Percent Heavy Vehicles				0		0
Percent Grade (%)	0				0	
Flared Approach: Exists?/Storage				/		/
Lanes				1		1
Configuration				L		R

Delay, Queue Length, and Level of Service

Approach Movement Lane Config	EB	WB	Northbound			Southbound		
	1 L	4 	7 	8	9	10 	11 	12 R
v (vph)	133					65		129
C(m) (vph)	892					206		475
v/c	0.15					0.32		0.27
95% queue length	0.52					1.29		1.09
Control Delay	9.7					30.3		15.4
LOS	A					D		C
Approach Delay								20.4
Approach LOS								C

HCS+: Unsignalized Intersections Release 5.21

Phone:  
 E-Mail:  
 Fax:

TWO-WAY STOP CONTROL(TWSC) ANALYSIS

Analyst: PM  
 Agency/Co.: RBF  
 Date Performed: 11-18-2009  
 Analysis Time Period: PM PEAK HOUR  
 Intersection: PV Dr East/PV Dr South  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 With Project  
 Project ID:  
 East/West Street: Palos Verdes Dr South  
 North/South Street: Palos Verdes Drive East  
 Intersection Orientation: EW Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street Movements	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	123	722			595	68
Peak-Hour Factor, PHF	0.92	0.92			0.92	0.92
Peak-15 Minute Volume	33	196			162	18
Hourly Flow Rate, HFR	133	784			646	73
Percent Heavy Vehicles	0	--	--		--	--
Median Type/Storage	Raised curb			/ 1		
RT Channelized?				No		
Lanes	1	1			1	1
Configuration	L	T			T	R
Upstream Signal?	No			No		

Minor Street Movements	7	8	9	10	11	12
	L	T	R	L	T	R
Volume				60		119
Peak Hour Factor, PHF				0.92		0.92
Peak-15 Minute Volume				16		32
Hourly Flow Rate, HFR				65		129
Percent Heavy Vehicles				0		0
Percent Grade (%)		0			0	
Flared Approach: Exists?/Storage				/		/
RT Channelized?				No		
Lanes				1		1
Configuration				L		R

Pedestrian Volumes and Adjustments

Movements	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)	12.0	12.0	12.0	12.0
Walking Speed (ft/sec)	4.0	4.0	4.0	4.0
Percent Blockage	0	0	0	0

Upstream Signal Data

Prog. Flow vph	Sat Flow vph	Arrival Type	Green Time sec	Cycle Length sec	Prog. Speed mph	Distance to Signal feet
----------------	--------------	--------------	----------------	------------------	-----------------	-------------------------

S2 Left-Turn Through

S5 Left-Turn  
Through

Worksheet 3-Data for Computing Effect of Delay to Major Street Vehicles

	Movement 2	Movement 5
Shared ln volume, major th vehicles:		
Shared ln volume, major rt vehicles:		
Sat flow rate, major th vehicles:		
Sat flow rate, major rt vehicles:		
Number of major street through lanes:		

Worksheet 4-Critical Gap and Follow-up Time Calculation

Critical Gap Calculation

Movement	1	4	7	8	9	10	11	12
	L	L	L	T	R	L	T	R
t(c,base)	4.1					7.1		6.2
t(c,hv)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
P(hv)	0					0		0
t(c,g)			0.20	0.20	0.10	0.20	0.20	0.10
Grade/100			0.00	0.00	0.00	0.00	0.00	0.00
t(3,lt)	0.00					0.70		0.00
t(c,T): 1-stage	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2-stage	0.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00
t(c) 1-stage	4.1					6.4		6.2
2-stage	4.1					5.4		6.2

Follow-Up Time Calculations

Movement	1	4	7	8	9	10	11	12
	L	L	L	T	R	L	T	R
t(f,base)	2.20					3.50		3.30
t(f,HV)	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
P(HV)	0					0		0
t(f)	2.2					3.5		3.3

Worksheet 5-Effect of Upstream Signals

Computation 1-Queue Clearance Time at Upstream Signal

	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
V prog				
Total Saturation Flow Rate, s (vph)				
Arrival Type				
Effective Green, g (sec)				
Cycle Length, C (sec)				
Rp (from Exhibit 16-11)				
Proportion vehicles arriving on green P				
g(q1)				
g(q2)				
g(q)				

V prog  
Total Saturation Flow Rate, s (vph)  
Arrival Type  
Effective Green, g (sec)  
Cycle Length, C (sec)  
Rp (from Exhibit 16-11)  
Proportion vehicles arriving on green P  
g(q1)  
g(q2)  
g(q)

Computation 2-Proportion of TWSC Intersection Time blocked

	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
alpha				
beta				

Travel time, t(a) (sec)

Smoothing Factor, F

Proportion of conflicting flow, f

Max platooned flow, V(c,max)

Min platooned flow, V(c,min)

Duration of blocked period, t(p)

Proportion time blocked, p 0.000 0.000

Computation 3-Platoon Event Periods Result

p(2) 0.000  
p(5) 0.000

p(dom)

p(subo)

Constrained or unconstrained?

Proportion

unblocked (1) (2) (3)  
for minor Single-stage Two-Stage Process  
movements, p(x) Process Stage I Stage II

p(1)

p(4)

p(7)

p(8)

p(9)

p(10)

p(11)

p(12)

Computation 4 and 5

Single-Stage Process

Movement	1	4	7	8	9	10	11	12
	L	L	L	T	R	L	T	R
V c,x	719					1696		646

s

Px

V c,u,x

C r,x

C plat,x

Two-Stage Process

	7		8		10		11	
	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2
V(c,x)					646	1050		
s						1500		
P(x)								
V(c,u,x)								

C(r,x)

C(plat,x)

Worksheet 6-Impedance and Capacity Equations

Step 1: RT from Minor St. 9 12

Conflicting Flows 646  
Potential Capacity 475  
Pedestrian Impedance Factor 1.00 1.00  
Movement Capacity 475  
Probability of Queue free St. 1.00 0.73

Step 2: LT from Major St.	4	1
Conflicting Flows		719
Potential Capacity		892
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity		892
Probability of Queue free St.	1.00	0.85
Maj L-Shared Prob Q free St.		

Step 3: TH from Minor St.	8	11
Conflicting Flows		
Potential Capacity		
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.85	0.85
Movement Capacity		
Probability of Queue free St.	1.00	1.00

Step 4: LT from Minor St.	7	10
Conflicting Flows		1696
Potential Capacity		103
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.85	
Maj. L, Min T Adj. Imp Factor.	0.89	
Cap. Adj. factor due to Impeding mvmnt	0.65	0.85
Movement Capacity		88

Worksheet 7-Computation of the Effect of Two-stage Gap Acceptance

Step 3: TH from Minor St.	8	11
Part 1 - First Stage		
Conflicting Flows		
Potential Capacity	307	470
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.85	1.00
Movement Capacity	261	470
Probability of Queue free St.	1.00	1.00

Part 2 - Second Stage		
Conflicting Flows		
Potential Capacity	436	307
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	1.00	0.85
Movement Capacity	436	261

Part 3 - Single Stage		
Conflicting Flows		
Potential Capacity		
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.85	0.85
Movement Capacity		

Result for 2 stage process:		
a	0.91	0.91
Y		
C t		
Probability of Queue free St.	1.00	1.00

Step 4: LT from Minor St.	7	10
---------------------------	---	----

Part 1 - First Stage

Conflicting Flows		646
Potential Capacity	340	526
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.85	1.00
Movement Capacity	289	526

Part 2 - Second Stage		
Conflicting Flows		1050
Potential Capacity	472	340
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.73	0.85
Movement Capacity	344	289

Part 3 - Single Stage		
Conflicting Flows		1696
Potential Capacity		103
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.85	
Maj. L, Min T Adj. Imp Factor.	0.89	
Cap. Adj. factor due to Impeding mvmnt	0.65	0.85
Movement Capacity		88

Results for Two-stage process:		
a	0.91	0.91
Y		2.18
C t		206

Worksheet 8-Shared Lane Calculations

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (vph)				65		129
Movement Capacity (vph)				206		475
Shared Lane Capacity (vph)						

Worksheet 9-Computation of Effect of Flared Minor Street Approaches

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
C sep				206		475
Volume				65		129
Delay						
Q sep						
Q sep +1						
round (Qsep +1)						
n max						
C sh						
SUM C sep						
n						
C act						

Worksheet 10-Delay, Queue Length, and Level of Service

Movement	1	4	7	8	9	10	11	12
Lane Config	L					L		R
v (vph)	133					65		129
C(m) (vph)	892					206		475
v/c	0.15					0.32		0.27

95% queue length	0.52	1.29	1.09
Control Delay	9.7	30.3	15.4
LOS	A	D	C
Approach Delay			20.4
Approach LOS			C

---

Worksheet 11-Shared Major LT Impedance and Delay

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	Movement 2	Movement 5
P(oj)	0.85	1.00
v(i1), Volume for stream 2 or 5		
v(i2), Volume for stream 3 or 6		
s(i1), Saturation flow rate for stream 2 or 5		
s(i2), Saturation flow rate for stream 3 or 6		
P*(oj)		
d(M,LT), Delay for stream 1 or 4	9.7	
N, Number of major street through lanes		
d(rank,1) Delay for stream 2 or 5		

---

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
MITIGATED FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF RPV
WEEKDAY (11:00-1:00) PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #1 PALOS VERDES DRIVE EAST/MIRALESTE DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 0.798
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 66 Level Of Service: C

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns representing different traffic movements. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume, and OvlAdjVol.

Saturation Flow Module:

Table with 12 columns representing different traffic movements. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns representing different traffic movements. Rows include Vol/Sat, OvlAdjV/S, and Crit Moves.

HCS+: Unsignalized Intersections Release 5.21

TWO-WAY STOP CONTROL SUMMARY

Analyst: PM  
 Agency/Co.: RBF  
 Date Performed: 11-18-2009  
 Analysis Time Period: Weekday Mid-day Peak Hour  
 Intersection: PV Dr East/PV Dr South  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 With Project  
 Project ID: Marymount  
 East/West Street: Palos Verdes Dr South  
 North/South Street: Palos Verdes Drive East  
 Intersection Orientation: EW Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	Eastbound			Westbound		
	1 L	2 T	3 R	4 L	5 T	6 R
Volume	68	536			467	48
Peak-Hour Factor, PHF	0.92	0.92			0.92	0.92
Hourly Flow Rate, HFR	73	582			507	52
Percent Heavy Vehicles	0	--	--		--	--
Median Type/Storage	Raised curb			/ 1		
RT Channelized?				No		
Lanes	1	1			1	1
Configuration	L	T			T	R
Upstream Signal?	No			No		

Minor Street: Approach Movement	Northbound			Southbound		
	7 L	8 T	9 R	10 L	11 T	12 R
Volume				70		91
Peak Hour Factor, PHF				0.92		0.92
Hourly Flow Rate, HFR				76		98
Percent Heavy Vehicles				0		0
Percent Grade (%)	0				0	
Flared Approach: Exists?/Storage				/		
Lanes				1		1
Configuration				L		R

Delay, Queue Length, and Level of Service

Approach Movement Lane Config	EB	WB	Northbound			Southbound		
	1 L	4 	7 	8	9	10 	11 	12 R
v (vph)	73					76		98
C(m) (vph)	1022					316		570
v/c	0.07					0.24		0.17
95% queue length	0.23					0.92		0.62
Control Delay	8.8					20.0		12.6
LOS	A					C		B
Approach Delay								15.8
Approach LOS								C

HCS+: Unsignalized Intersections Release 5.21

Phone:  
 Fax:  
 E-Mail:

TWO-WAY STOP CONTROL(TWSC) ANALYSIS

Analyst: PM  
 Agency/Co.: RBF  
 Date Performed: 11-18-2009  
 Analysis Time Period: Weekday Mid-day Peak Hour  
 Intersection: PV Dr East/PV Dr South  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 With Project  
 Project ID: Marymount  
 East/West Street: Palos Verdes Dr South  
 North/South Street: Palos Verdes Drive East  
 Intersection Orientation: EW Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street Movements	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	68	536			467	48
Peak-Hour Factor, PHF	0.92	0.92			0.92	0.92
Peak-15 Minute Volume	18	146			127	13
Hourly Flow Rate, HFR	73	582			507	52
Percent Heavy Vehicles	0	--	--		--	--
Median Type/Storage	Raised curb			/ 1		
RT Channelized?				No		
Lanes	1	1			1	1
Configuration	L	T			T	R
Upstream Signal?	No			No		

Minor Street Movements	7	8	9	10	11	12
	L	T	R	L	T	R
Volume				70		91
Peak Hour Factor, PHF				0.92		0.92
Peak-15 Minute Volume				19		25
Hourly Flow Rate, HFR				76		98
Percent Heavy Vehicles				0		0
Percent Grade (%)		0			0	
Flared Approach: Exists?/Storage				/		
RT Channelized?				No		
Lanes				1		1
Configuration				L		R

Pedestrian Volumes and Adjustments

Movements	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)	12.0	12.0	12.0	12.0
Walking Speed (ft/sec)	4.0	4.0	4.0	4.0
Percent Blockage	0	0	0	0

Upstream Signal Data							
	Prog. Flow vph	Sat Flow vph	Arrival Type	Green Time sec	Cycle Length sec	Prog. Speed mph	Distance to Signal feet
S2	Left-Turn						
	Through						
S5	Left-Turn						
	Through						

Worksheet 3-Data for Computing Effect of Delay to Major Street Vehicles

	Movement 2	Movement 5
Shared ln volume, major th vehicles:		
Shared ln volume, major rt vehicles:		
Sat flow rate, major th vehicles:		
Sat flow rate, major rt vehicles:		
Number of major street through lanes:		

Worksheet 4-Critical Gap and Follow-up Time Calculation

Critical Gap Calculation								
Movement	1	4	7	8	9	10	11	12
	L	L	L	T	R	L	T	R
t(c,base)	4.1					7.1		6.2
t(c,hv)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
P(hv)	0					0		0
t(c,g)			0.20	0.20	0.10	0.20	0.20	0.10
Grade/100			0.00	0.00	0.00	0.00	0.00	0.00
t(3,lt)	0.00					0.70		0.00
t(c,T): 1-stage	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2-stage	0.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00
t(c) 1-stage	4.1					6.4		6.2
2-stage	4.1					5.4		6.2

Follow-Up Time Calculations								
Movement	1	4	7	8	9	10	11	12
	L	L	L	T	R	L	T	R
t(f,base)	2.20					3.50		3.30
t(f,HV)	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
P(HV)	0					0		0
t(f)	2.2					3.5		3.3

Worksheet 5-Effect of Upstream Signals

Computation 1-Queue Clearance Time at Upstream Signal				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)

V prog  
 Total Saturation Flow Rate, s (vph)  
 Arrival Type  
 Effective Green, g (sec)  
 Cycle Length, C (sec)  
 Rp (from Exhibit 16-11)  
 Proportion vehicles arriving on green P  
 g(q1)  
 g(q2)  
 g(q)

Computation 2-Proportion of TWSC Intersection Time blocked				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)

alpha  
 beta  
 Travel time, t(a) (sec)  
 Smoothing Factor, F  
 Proportion of conflicting flow, f  
 Max platooned flow, V(c,max)  
 Min platooned flow, V(c,min)  
 Duration of blocked period, t(p)  
 Proportion time blocked, p

Computation 3-Platoon Event Periods	Result
p(2)	0.000
p(5)	0.000
p(dom)	
p(subo)	
Constrained or unconstrained?	

Proportion unblocked for minor movements, p(x)	(1) Single-stage Process	(2) Two-Stage Process Stage I	(3) Process Stage II
p(1)			
p(4)			
p(7)			
p(8)			
p(9)			
p(10)			
p(11)			
p(12)			

Computation 4 and 5 Single-Stage Process								
Movement	1	4	7	8	9	10	11	12
	L	L	L	T	R	L	T	R

V c,x  
 s  
 Px  
 V c,u,x  
 C r,x  
 C plat,x

Two-Stage Process								
	7		8		10		11	
	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2

V(c,x)  
 s  
 P(x)  
 V(c,u,x)  
 C(r,x)  
 C(plat,x)

Worksheet 6-Impedance and Capacity Equations

Step 1: RT from Minor St.	9	12
Conflicting Flows		507
Potential Capacity		570
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity		570
Probability of Queue free St.	1.00	0.83
Step 2: LT from Major St.	4	1
Conflicting Flows		559
Potential Capacity		1022
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity		1022
Probability of Queue free St.	1.00	0.93
Maj L-Shared Prob Q free St.		
Step 3: TH from Minor St.	8	11
Conflicting Flows		
Potential Capacity		
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.93	0.93
Movement Capacity		
Probability of Queue free St.	1.00	1.00
Step 4: LT from Minor St.	7	10
Conflicting Flows		1235
Potential Capacity		197
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.93	
Maj. L, Min T Adj. Imp Factor.	0.95	
Cap. Adj. factor due to Impeding mvmnt	0.78	0.93
Movement Capacity		183

Worksheet 7-Computation of the Effect of Two-stage Gap Acceptance

Step 3: TH from Minor St.	8	11
Part 1 - First Stage		
Conflicting Flows		
Potential Capacity	432	543
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.93	1.00
Movement Capacity	401	543
Probability of Queue free St.	1.00	1.00
Part 2 - Second Stage		
Conflicting Flows		
Potential Capacity	514	432
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	1.00	0.93
Movement Capacity	514	401
Part 3 - Single Stage		
Conflicting Flows		
Potential Capacity		
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.93	0.93
Movement Capacity		
Result for 2 stage process:		
a	0.91	0.91

Y		
C t		
Probability of Queue free St.	1.00	1.00
Step 4: LT from Minor St.	7	10
Part 1 - First Stage		
Conflicting Flows		507
Potential Capacity	482	609
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.93	1.00
Movement Capacity	448	609
Part 2 - Second Stage		
Conflicting Flows		728
Potential Capacity	563	482
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.83	0.93
Movement Capacity	466	448
Part 3 - Single Stage		
Conflicting Flows		1235
Potential Capacity		197
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.93	
Maj. L, Min T Adj. Imp Factor.	0.95	
Cap. Adj. factor due to Impeding mvmnt	0.78	0.93
Movement Capacity		183
Results for Two-stage process:		
a	0.91	0.91
Y		1.61
C t		316

Worksheet 8-Shared Lane Calculations

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (vph)				76		98
Movement Capacity (vph)				316		570
Shared Lane Capacity (vph)						

Worksheet 9-Computation of Effect of Flared Minor Street Approaches

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
C sep				316		570
Volume				76		98
Delay						
Q sep						
Q sep +1						
round (Qsep +1)						
n max						
C sh						
SUM C sep						
n						
C act						

Worksheet 10-Delay, Queue Length, and Level of Service

Movement	1	4	7	8	9	10	11	12
Lane Config	L					L		R
v (vph)	73					76		98
C(m) (vph)	1022					316		570
v/c	0.07					0.24		0.17
95% queue length	0.23					0.92		0.62
Control Delay	8.8					20.0		12.6
LOS	A					C		B
Approach Delay							15.8	
Approach LOS							C	

Worksheet 11-Shared Major LT Impedance and Delay

	Movement 2	Movement 5
p(oj)	0.93	1.00
v(i1), Volume for stream 2 or 5		
v(i2), Volume for stream 3 or 6		
s(i1), Saturation flow rate for stream 2 or 5		
s(i2), Saturation flow rate for stream 3 or 6		
P*(oj)		
d(M,LT), Delay for stream 1 or 4	8.8	
N, Number of major street through lanes		
d(rank,1) Delay for stream 2 or 5		

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
MITIGATED FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF RPV
WEEKDAY (2:00-4:00) PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #1 PALOS VERDES DRIVE EAST/MIRALESTE DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 0.899
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 99 Level Of Service: D

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns representing different volume categories. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume, and OvlAdjVol.

Saturation Flow Module:

Table with 12 columns representing saturation flow factors. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns representing capacity analysis metrics. Rows include Vol/Sat, OvlAdjV/S, and Crit Moves.

HCS+: Unsignalized Intersections Release 5.21

TWO-WAY STOP CONTROL SUMMARY

Analyst: PM  
 Agency/Co.: RBF  
 Date Performed: 11-18-2009  
 Analysis Time Period: AFTERNOON 2-4PM PEAK HOUR  
 Intersection: PV Dr East/PV Dr South  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 With Project  
 Project ID: Marymount  
 East/West Street: Palos Verdes Dr South  
 North/South Street: Palos Verdes Drive East  
 Intersection Orientation: EW Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	Eastbound			Westbound		
	1 L	2 T	3 R	4 L	5 T	6 R
Volume	210	683			557	64
Peak-Hour Factor, PHF	0.92	0.92			0.92	0.92
Hourly Flow Rate, HFR	228	742			605	69
Percent Heavy Vehicles	0	--	--		--	--
Median Type/Storage	Raised curb			/ 1		
RT Channelized?				No		
Lanes	1	1			1	1
Configuration	L	T			T	R
Upstream Signal?	No			No		

Minor Street: Approach Movement	Northbound			Southbound		
	7 L	8 T	9 R	10 L	11 T	12 R
Volume				73		145
Peak Hour Factor, PHF				0.92		0.92
Hourly Flow Rate, HFR				79		157
Percent Heavy Vehicles				0		0
Percent Grade (%)	0				0	
Flared Approach: Exists?/Storage				/		
Lanes				1		1
Configuration				L		R

Delay, Queue Length, and Level of Service

Approach Movement Lane Config	EB	WB	Northbound			Southbound		
	1 L	4 	7 	8	9	10 	11 	12 R
v (vph)	228					79		157
C(m) (vph)	927					166		501
v/c	0.25					0.48		0.31
95% queue length	0.97					2.26		1.33
Control Delay	10.1					45.0		15.4
LOS	B					E		C
Approach Delay								25.3
Approach LOS								D

HCS+: Unsignalized Intersections Release 5.21

Phone:  
 Fax:  
 E-Mail:

TWO-WAY STOP CONTROL(TWSC) ANALYSIS

Analyst: PM  
 Agency/Co.: RBF  
 Date Performed: 11-18-2009  
 Analysis Time Period: AFTERNOON 2-4PM PEAK HOUR  
 Intersection: PV Dr East/PV Dr South  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 With Project  
 Project ID: Marymount  
 East/West Street: Palos Verdes Dr South  
 North/South Street: Palos Verdes Drive East  
 Intersection Orientation: EW Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street Movements	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	210	683			557	64
Peak-Hour Factor, PHF	0.92	0.92			0.92	0.92
Peak-15 Minute Volume	57	186			151	17
Hourly Flow Rate, HFR	228	742			605	69
Percent Heavy Vehicles	0	--	--		--	--
Median Type/Storage	Raised curb			/ 1		
RT Channelized?				No		
Lanes	1	1			1	1
Configuration	L	T			T	R
Upstream Signal?	No			No		

Minor Street Movements	7	8	9	10	11	12
	L	T	R	L	T	R
Volume				73		145
Peak Hour Factor, PHF				0.92		0.92
Peak-15 Minute Volume				20		39
Hourly Flow Rate, HFR				79		157
Percent Heavy Vehicles				0		0
Percent Grade (%)		0			0	
Flared Approach: Exists?/Storage				/		
RT Channelized?				No		
Lanes				1		1
Configuration				L		R

Pedestrian Volumes and Adjustments

Movements	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)	12.0	12.0	12.0	12.0
Walking Speed (ft/sec)	4.0	4.0	4.0	4.0
Percent Blockage	0	0	0	0

Upstream Signal Data							
	Prog. Flow vph	Sat Flow vph	Arrival Type	Green Time sec	Cycle Length sec	Prog. Speed mph	Distance to Signal feet
S2	Left-Turn						
	Through						
S5	Left-Turn						
	Through						

Worksheet 3-Data for Computing Effect of Delay to Major Street Vehicles

	Movement 2	Movement 5
Shared ln volume, major th vehicles:		
Shared ln volume, major rt vehicles:		
Sat flow rate, major th vehicles:		
Sat flow rate, major rt vehicles:		
Number of major street through lanes:		

Worksheet 4-Critical Gap and Follow-up Time Calculation

Critical Gap Calculation								
Movement	1	4	7	8	9	10	11	12
	L	L	L	T	R	L	T	R
t(c,base)	4.1					7.1		6.2
t(c,hv)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
P(hv)	0					0		0
t(c,g)			0.20	0.20	0.10	0.20	0.20	0.10
Grade/100			0.00	0.00	0.00	0.00	0.00	0.00
t(3,lt)	0.00					0.70		0.00
t(c,T): 1-stage	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2-stage	0.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00
t(c) 1-stage	4.1					6.4		6.2
2-stage	4.1					5.4		6.2

Follow-Up Time Calculations								
Movement	1	4	7	8	9	10	11	12
	L	L	L	T	R	L	T	R
t(f,base)	2.20					3.50		3.30
t(f,HV)	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
P(HV)	0					0		0
t(f)	2.2					3.5		3.3

Worksheet 5-Effect of Upstream Signals

Computation 1-Queue Clearance Time at Upstream Signal				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
V prog				
Total Saturation Flow Rate, s (vph)				
Arrival Type				
Effective Green, g (sec)				
Cycle Length, C (sec)				
Rp (from Exhibit 16-11)				
Proportion vehicles arriving on green P				
g(q1)				
g(q2)				
g(q)				

Computation 2-Proportion of TWSC Intersection Time blocked				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
alpha				
beta				
Travel time, t(a) (sec)				
Smoothing Factor, F				
Proportion of conflicting flow, f				
Max platooned flow, V(c,max)				
Min platooned flow, V(c,min)				
Duration of blocked period, t(p)				
Proportion time blocked, p		0.000		0.000

Computation 3-Platoon Event Periods	Result
p(2)	0.000
p(5)	0.000
p(dom)	
p(subo)	
Constrained or unconstrained?	

Proportion unblocked for minor movements, p(x)	(1) Single-stage Process	(2) Two-Stage Process Stage I	(3) Process Stage II
p(1)			
p(4)			
p(7)			
p(8)			
p(9)			
p(10)			
p(11)			
p(12)			

Computation 4 and 5 Single-Stage Process								
Movement	1	4	7	8	9	10	11	12
	L	L	L	T	R	L	T	R
V c,x	674					1803		605
s								
Px								
V c,u,x								
C r,x								
C plat,x								

Two-Stage Process								
	7		8		10		11	
	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2
V(c,x)					605	1198		
s						1500		
P(x)								
V(c,u,x)								
C(r,x)								
C(plat,x)								

Worksheet 6-Impedance and Capacity Equations

Step 1: RT from Minor St.	9	12
Conflicting Flows		605
Potential Capacity		501
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity		501
Probability of Queue free St.	1.00	0.69
Step 2: LT from Major St.	4	1
Conflicting Flows		674
Potential Capacity		927
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity		927
Probability of Queue free St.	1.00	0.75
Maj L-Shared Prob Q free St.		
Step 3: TH from Minor St.	8	11
Conflicting Flows		
Potential Capacity		
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.75	0.75
Movement Capacity		
Probability of Queue free St.	1.00	1.00
Step 4: LT from Minor St.	7	10
Conflicting Flows		1803
Potential Capacity		88
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.75	
Maj. L, Min T Adj. Imp Factor.	0.81	
Cap. Adj. factor due to Impeding mvmnt	0.56	0.75
Movement Capacity		66

Worksheet 7-Computation of the Effect of Two-stage Gap Acceptance

Step 3: TH from Minor St.	8	11
Part 1 - First Stage		
Conflicting Flows		
Potential Capacity	261	491
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.75	1.00
Movement Capacity	197	491
Probability of Queue free St.	1.00	1.00
Part 2 - Second Stage		
Conflicting Flows		
Potential Capacity	457	261
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	1.00	0.75
Movement Capacity	457	197
Part 3 - Single Stage		
Conflicting Flows		
Potential Capacity		
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.75	0.75
Movement Capacity		
Result for 2 stage process:		
a	0.91	0.91

Y		
C t		
Probability of Queue free St.	1.00	1.00
Step 4: LT from Minor St.	7	10
Part 1 - First Stage		
Conflicting Flows		605
Potential Capacity	289	549
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.75	1.00
Movement Capacity	218	549
Part 2 - Second Stage		
Conflicting Flows		1198
Potential Capacity	487	289
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.69	0.75
Movement Capacity	334	218
Part 3 - Single Stage		
Conflicting Flows		1803
Potential Capacity		88
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.75	
Maj. L, Min T Adj. Imp Factor.	0.81	
Cap. Adj. factor due to Impeding mvmnt	0.56	0.75
Movement Capacity		66
Results for Two-stage process:		
a	0.91	0.91
Y		3.18
C t		166

Worksheet 8-Shared Lane Calculations

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (vph)				79		157
Movement Capacity (vph)				166		501
Shared Lane Capacity (vph)						

Worksheet 9-Computation of Effect of Flared Minor Street Approaches

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
C sep				166		501
Volume				79		157
Delay						
Q sep						
Q sep +1						
round (Qsep +1)						
n max						
C sh						
SUM C sep						
n						
C act						

Worksheet 10-Delay, Queue Length, and Level of Service

Movement	1	4	7	8	9	10	11	12
Lane Config	L					L		R
v (vph)	228					79		157
C(m) (vph)	927					166		501
v/c	0.25					0.48		0.31
95% queue length	0.97					2.26		1.33
Control Delay	10.1					45.0		15.4
LOS	B					E		C
Approach Delay							25.3	
Approach LOS							D	

Worksheet 11-Shared Major LT Impedance and Delay

	Movement 2	Movement 5
p(oj)	0.75	1.00
v(i1), Volume for stream 2 or 5		
v(i2), Volume for stream 3 or 6		
s(i1), Saturation flow rate for stream 2 or 5		
s(i2), Saturation flow rate for stream 3 or 6		
P*(oj)		
d(M,LT), Delay for stream 1 or 4	10.1	
N, Number of major street through lanes		
d(rank,1) Delay for stream 2 or 5		

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
MITIGATED FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF RPV
SATURDAY (11:00-1:00) PEAK HOUR

Level Of Service Computation Report

ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #1 PALOS VERDES DRIVE EAST/MIRALESTE DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 0.670
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 47 Level Of Service: B

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns representing different volume categories. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, Final Volume, and OvlAdjVol.

Saturation Flow Module:

Table with 12 columns representing saturation flow metrics. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns representing capacity analysis metrics. Rows include Vol/Sat, OvlAdjV/S, and Crit Moves.

HCS+: Unsignalized Intersections Release 5.21

TWO-WAY STOP CONTROL SUMMARY

Analyst: PM  
 Agency/Co.: RBF  
 Date Performed: 11-18-2009  
 Analysis Time Period: Saturday Peak Hour  
 Intersection: PV Dr East/PV Dr South  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 With Project  
 Project ID: Marymount  
 East/West Street: Palos Verdes Dr South  
 North/South Street: Palos Verdes Drive East  
 Intersection Orientation: EW Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	Eastbound			Westbound		
	1 L	2 T	3 R	4 L	5 T	6 R
Volume	92	695			736	71
Peak-Hour Factor, PHF	0.92	0.92			0.92	0.92
Hourly Flow Rate, HFR	99	755			799	77
Percent Heavy Vehicles	0	--	--		--	--
Median Type/Storage	Raised curb			/ 1		
RT Channelized?						No
Lanes	1	1			1	1
Configuration	L	T			T	R
Upstream Signal?	No			No		

Minor Street: Approach Movement	Northbound			Southbound		
	7 L	8 T	9 R	10 L	11 T	12 R
Volume				40		84
Peak Hour Factor, PHF				0.92		0.92
Hourly Flow Rate, HFR				43		91
Percent Heavy Vehicles				0		0
Percent Grade (%)	0				0	
Flared Approach: Exists?/Storage				/		/
Lanes				1		1
Configuration				L		R

Delay, Queue Length, and Level of Service

Approach Movement Lane Config	EB	WB	Northbound			Southbound		
	1 L	4 	7 	8	9	10 	11 	12 R
v (vph)	99					43		91
C(m) (vph)	779					210		389
v/c	0.13					0.20		0.23
95% queue length	0.43					0.75		0.89
Control Delay	10.3					26.5		17.1
LOS	B					D		C
Approach Delay								20.1
Approach LOS								C

HCS+: Unsignalized Intersections Release 5.21

Phone:  
 E-Mail: Fax:

TWO-WAY STOP CONTROL(TWSC) ANALYSIS

Analyst: PM  
 Agency/Co.: RBF  
 Date Performed: 11-18-2009  
 Analysis Time Period: Saturday Peak Hour  
 Intersection: PV Dr East/PV Dr South  
 Jurisdiction: RPV  
 Units: U. S. Customary  
 Analysis Year: 2012 With Project  
 Project ID: Marymount  
 East/West Street: Palos Verdes Dr South  
 North/South Street: Palos Verdes Drive East  
 Intersection Orientation: EW Study period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street Movements	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	92	695			736	71
Peak-Hour Factor, PHF	0.92	0.92			0.92	0.92
Peak-15 Minute Volume	25	189			200	19
Hourly Flow Rate, HFR	99	755			799	77
Percent Heavy Vehicles	0	--	--		--	--
Median Type/Storage	Raised curb			/ 1		
RT Channelized?						No
Lanes	1	1			1	1
Configuration	L	T			T	R
Upstream Signal?	No			No		

Minor Street Movements	7	8	9	10	11	12
	L	T	R	L	T	R
Volume				40		84
Peak Hour Factor, PHF				0.92		0.92
Peak-15 Minute Volume				11		23
Hourly Flow Rate, HFR				43		91
Percent Heavy Vehicles				0		0
Percent Grade (%)		0			0	
Flared Approach: Exists?/Storage				/		/
RT Channelized?						No
Lanes				1		1
Configuration				L		R

Pedestrian Volumes and Adjustments

Movements	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)	12.0	12.0	12.0	12.0
Walking Speed (ft/sec)	4.0	4.0	4.0	4.0
Percent Blockage	0	0	0	0

Upstream Signal Data							
	Prog. Flow vph	Sat Flow vph	Arrival Type	Green Time sec	Cycle Length sec	Prog. Speed mph	Distance to Signal feet
S2	Left-Turn						
	Through						
S5	Left-Turn						
	Through						

Worksheet 3-Data for Computing Effect of Delay to Major Street Vehicles

	Movement 2	Movement 5
Shared ln volume, major th vehicles:		
Shared ln volume, major rt vehicles:		
Sat flow rate, major th vehicles:		
Sat flow rate, major rt vehicles:		
Number of major street through lanes:		

Worksheet 4-Critical Gap and Follow-up Time Calculation

Critical Gap Calculation								
Movement	1	4	7	8	9	10	11	12
	L	L	L	T	R	L	T	R
t(c,base)	4.1					7.1		6.2
t(c,hv)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
P(hv)	0					0		0
t(c,g)			0.20	0.20	0.10	0.20	0.20	0.10
Grade/100			0.00	0.00	0.00	0.00	0.00	0.00
t(3,lt)	0.00					0.70		0.00
t(c,T): 1-stage	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2-stage	0.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00
t(c) 1-stage	4.1					6.4		6.2
2-stage	4.1					5.4		6.2

Follow-Up Time Calculations								
Movement	1	4	7	8	9	10	11	12
	L	L	L	T	R	L	T	R
t(f,base)	2.20					3.50		3.30
t(f,HV)	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
P(HV)	0					0		0
t(f)	2.2					3.5		3.3

Worksheet 5-Effect of Upstream Signals

Computation 1-Queue Clearance Time at Upstream Signal				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
V prog				
Total Saturation Flow Rate, s (vph)				
Arrival Type				
Effective Green, g (sec)				
Cycle Length, C (sec)				
Rp (from Exhibit 16-11)				
Proportion vehicles arriving on green P				
g(q1)				
g(q2)				
g(q)				

Computation 2-Proportion of TWSC Intersection Time blocked				
	Movement 2		Movement 5	
	V(t)	V(l,prot)	V(t)	V(l,prot)
alpha				
beta				
Travel time, t(a) (sec)				
Smoothing Factor, F				
Proportion of conflicting flow, f				
Max platooned flow, V(c,max)				
Min platooned flow, V(c,min)				
Duration of blocked period, t(p)				
Proportion time blocked, p		0.000		0.000

Computation 3-Platoon Event Periods	Result
p(2)	0.000
p(5)	0.000
p(dom)	
p(subo)	
Constrained or unconstrained?	

Proportion unblocked for minor movements, p(x)	(1) Single-stage Process	(2) Two-Stage Process Stage I	(3) Process Stage II
p(1)			
p(4)			
p(7)			
p(8)			
p(9)			
p(10)			
p(11)			
p(12)			

Computation 4 and 5 Single-Stage Process								
Movement	1	4	7	8	9	10	11	12
	L	L	L	T	R	L	T	R
V c,x	876					1752		799
s								
Px								
V c,u,x								
C r,x								
C plat,x								

Two-Stage Process								
	7		8		10		11	
	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2	Stage1	Stage2
V(c,x)					799	953		
s						1500		
P(x)								
V(c,u,x)								
C(r,x)								
C(plat,x)								

Worksheet 6-Impedance and Capacity Equations

Step 1: RT from Minor St.	9	12
Conflicting Flows		799
Potential Capacity		389
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity		389
Probability of Queue free St.	1.00	0.77
Step 2: LT from Major St.	4	1
Conflicting Flows		876
Potential Capacity		779
Pedestrian Impedance Factor	1.00	1.00
Movement Capacity		779
Probability of Queue free St.	1.00	0.87
Maj L-Shared Prob Q free St.		
Step 3: TH from Minor St.	8	11
Conflicting Flows		
Potential Capacity		
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.87	0.87
Movement Capacity		
Probability of Queue free St.	1.00	1.00
Step 4: LT from Minor St.	7	10
Conflicting Flows		1752
Potential Capacity		95
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.87	
Maj. L, Min T Adj. Imp Factor.	0.90	
Cap. Adj. factor due to Impeding mvmnt	0.69	0.87
Movement Capacity		83

Worksheet 7-Computation of the Effect of Two-stage Gap Acceptance

Step 3: TH from Minor St.	8	11
Part 1 - First Stage		
Conflicting Flows		
Potential Capacity	340	401
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.87	1.00
Movement Capacity	297	401
Probability of Queue free St.	1.00	1.00
Part 2 - Second Stage		
Conflicting Flows		
Potential Capacity	369	340
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	1.00	0.87
Movement Capacity	369	297
Part 3 - Single Stage		
Conflicting Flows		
Potential Capacity		
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.87	0.87
Movement Capacity		
Result for 2 stage process:		
a	0.91	0.91

Y		
C t		
Probability of Queue free St.	1.00	1.00
Step 4: LT from Minor St.	7	10
Part 1 - First Stage		
Conflicting Flows		799
Potential Capacity	378	446
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.87	1.00
Movement Capacity	330	446
Part 2 - Second Stage		
Conflicting Flows		953
Potential Capacity	408	378
Pedestrian Impedance Factor	1.00	1.00
Cap. Adj. factor due to Impeding mvmnt	0.77	0.87
Movement Capacity	313	330
Part 3 - Single Stage		
Conflicting Flows		1752
Potential Capacity		95
Pedestrian Impedance Factor	1.00	1.00
Maj. L, Min T Impedance factor	0.87	
Maj. L, Min T Adj. Imp Factor.	0.90	
Cap. Adj. factor due to Impeding mvmnt	0.69	0.87
Movement Capacity		83
Results for Two-stage process:		
a	0.91	0.91
Y		1.47
C t		210

Worksheet 8-Shared Lane Calculations

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (vph)				43		91
Movement Capacity (vph)				210		389
Shared Lane Capacity (vph)						

Worksheet 9-Computation of Effect of Flared Minor Street Approaches

Movement	7	8	9	10	11	12
	L	T	R	L	T	R
C sep				210		389
Volume				43		91
Delay						
Q sep						
Q sep +1						
round (Qsep +1)						
n max						
C sh						
SUM C sep						
n						
C act						

Worksheet 10-Delay, Queue Length, and Level of Service

Movement	1	4	7	8	9	10	11	12
Lane Config	L					L		R
v (vph)	99					43		91
C(m) (vph)	779					210		389
v/c	0.13					0.20		0.23
95% queue length	0.43					0.75		0.89
Control Delay	10.3					26.5		17.1
LOS	B					D		C
Approach Delay							20.1	
Approach LOS							C	

Worksheet 11-Shared Major LT Impedance and Delay

	Movement 2	Movement 5
p(oj)	0.87	1.00
v(i1), Volume for stream 2 or 5		
v(i2), Volume for stream 3 or 6		
s(i1), Saturation flow rate for stream 2 or 5		
s(i2), Saturation flow rate for stream 3 or 6		
P*(oj)		
d(M,LT), Delay for stream 1 or 4	10.3	
N, Number of major street through lanes		
d(rank,1) Delay for stream 2 or 5		

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
MITIGATED FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF LOS ANGELES
AM PEAK HOUR

Level Of Service Computation Report
Circular 212 Planning Method (Future Volume Alternative)

Intersection #7 WESTERN AVENUE/CAPITOL DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 1.013
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module table with 12 columns representing different traffic movements and 12 rows of volume-related metrics like Base Vol, Growth Adj, etc.

Saturation Flow Module table with 12 columns and 4 rows showing Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module table with 12 columns and 4 rows showing Vol/Sat, Crit Volume, and Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
MITIGATED FORECAST YEAR 2012 WITH PROJECT CONDITIONS - CITY OF LOS ANGELES
PM PEAK HOUR

Level Of Service Computation Report
Circular 212 Planning Method (Future Volume Alternative)

Intersection #7 WESTERN AVENUE/CAPITOL DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 1.023
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): xxxxxx
Optimal Cycle: 180 Level Of Service: F

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns representing different volume metrics and 12 rows of data including Base Vol, Growth Adj, Initial Bse, etc.

Saturation Flow Module:

Table with 12 columns representing saturation flow metrics and 4 rows of data including Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns representing capacity analysis metrics and 3 rows of data including Vol/Sat, Crit Volume, and Crit Moves.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
MITIGATED FORECAST YEAR 2012 WITH PROJECT CONDITIONS - Caltrans
AM PEAK HOUR

Level of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #7 WESTERN AVENUE/CAPITOL DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 0.904
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 28.0
Optimal Cycle: 107 Level of Service: C

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module:

Table with 12 columns representing different traffic movements. Rows include Base Vol, Growth Adj, Initial Bse, Added Vol, Cumulative, Initial Fut, User Adj, PHF Adj, PHF Volume, Reduct Vol, Reduced Vol, PCE Adj, MLF Adj, and Final Volume.

Saturation Flow Module:

Table with 12 columns. Rows include Sat/Lane, Adjustment, Lanes, and Final Sat.

Capacity Analysis Module:

Table with 12 columns. Rows include Vol/Sat, Crit Moves, Green/Cycle, Volume/Cap, Delay/Veh, User DelAdj, AdjDel/Veh, LOS by Move, and HCM2kAvgQ.

Note: Queue reported is the number of cars per lane.

MARYMOUNT COLLEGE FACILITIES EXPANSION TRAFFIC IMPACT ANALYSIS (JN 10104089)
MITIGATED FORECAST YEAR 2012 WITH PROJECT CONDITIONS - Caltrans
PM PEAK HOUR

Level of Service Computation Report
2000 HCM Operations Method (Future Volume Alternative)

Intersection #7 WESTERN AVENUE/CAPITOL DRIVE

Cycle (sec): 100 Critical Vol./Cap.(X): 0.916
Loss Time (sec): 10 (Y+R=4.0 sec) Average Delay (sec/veh): 32.9
Optimal Cycle: 114 Level of Service: C

Table with 4 columns: North Bound, South Bound, East Bound, West Bound. Rows include Movement, Control, Rights, Min. Green, and Lanes.

Volume Module table with 12 columns representing different traffic movements and 12 rows of volume-related metrics like Base Vol, Growth Adj, etc.

Saturation Flow Module table with 12 columns and 5 rows of saturation flow data.

Capacity Analysis Module table with 12 columns and 10 rows of capacity analysis metrics.

Note: Queue reported is the number of cars per lane.

## **Appendix D-2 Air Quality Data**

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**Parenthetical URBEMIS 2007 Assumptions  
For: Marymount College Facilities Expansion EIR  
Date: December 2009**

**LAND USES**

**BA Program**

Amount	Land Use Type	Unit Type	Trip Rate
59.482	Junior/Community College	Thousand Square Feet	27.49
250	Junior College	Students	1.18
Note: The Junior College amount was adjusted to reflect net trips generated by the project. The total daily trip rate is consistent with the Marymount College Project Traffic & Parking Analysis, dated December 10, 2009.			

**YEAR 2007 AREA SOURCES**

**Natural Gas Fuel Combustion:**

(URBEMIS 2007 default all phases)

**Wood Stoves Fuel Combustion:**

Off

**Fireplaces:**

Off

**Landscape Maintenance Equipment:**

Year of Completion	Summer Days
2012	180

**Consumer Products:**

(URBEMIS 2007 default all phases)

**Area Source Mitigation:**

Refer to URBEMIS 2007 file output.

**YEAR 2010 OPERATIONAL SOURCES**

**Vehicle Fleet %:**

(URBEMIS 2007 default all phases)

**Year:**

Year of Completion – 2012\*

(\*Year 2012 was used for area source completion year to be consistent with the Traffic Impact Analysis. It should be noted that this represents a conservative analysis as emissions rates decline in future years.)

**Trip Characteristics:**

(URBEMIS 2007 Default all phases)

**Temperature Data:**

50 to 95 degrees Fahrenheit

**Variable Starts:**

(URBEMIS 2007 default all phases)

**Road Dust:**

Paved – 100%  
Unpaved – 0%

**Pass By Trips (On/Off):**

On

**Double-Counting(On/Off):**

Off

**Operational Mitigation Measures:**

Refer to URBEMIS 2007 file output.

Urbemis 2007 Version 9.2.4

Combined Summer Emissions Reports (Pounds/Day)

File Name: I:\pdata\00000100\10P\WPWIN\EddieT\Programs\Air\URBEMIS\URBEMIS2007\Marymount Scenario1.urb924

Project Name: Marymount Scenario 1

Project Location: South Coast AQMD

On-Road Vehicle Emissions Based on: Version : Emfac2007 V2.3 Nov 1 2006

Off-Road Vehicle Emissions Based on: OFFROAD2007

Summary Report:

AREA SOURCE EMISSION ESTIMATES

	<u>ROG</u>	<u>NOx</u>	<u>CO</u>	<u>SO2</u>	<u>PM10</u>	<u>PM2.5</u>	<u>CO2</u>
TOTALS (lbs/day, unmitigated)	0.78	0.66	3.73	0.00	0.01	0.01	748.22

OPERATIONAL (VEHICLE) EMISSION ESTIMATES

	<u>ROG</u>	<u>NOx</u>	<u>CO</u>	<u>SO2</u>	<u>PM10</u>	<u>PM2.5</u>	<u>CO2</u>
TOTALS (lbs/day, unmitigated)	11.71	14.71	131.24	0.18	29.99	5.84	17,890.14

SUM OF AREA SOURCE AND OPERATIONAL EMISSION ESTIMATES

	<u>ROG</u>	<u>NOx</u>	<u>CO</u>	<u>SO2</u>	<u>PM10</u>	<u>PM2.5</u>	<u>CO2</u>
TOTALS (lbs/day, unmitigated)	12.49	15.37	134.97	0.18	30.00	5.85	18,638.36

Operational Unmitigated Detail Report:

OPERATIONAL EMISSION ESTIMATES Summer Pounds Per Day, Unmitigated

<u>Source</u>	ROG	NOX	CO	SO2	PM10	PM25	CO2
Junior college (2 yrs)	3.05	2.27	20.30	0.03	4.63	0.90	2,763.17
Junior College	8.66	12.44	110.94	0.15	25.36	4.94	15,126.97
TOTALS (lbs/day, unmitigated)	11.71	14.71	131.24	0.18	29.99	5.84	17,890.14

Operational Settings:

Does not include correction for passby trips

Does not include double counting adjustment for internal trips

Analysis Year: 2014 Temperature (F): 80 Season: Summer

Emfac: Version : Emfac2007 V2.3 Nov 1 2006

Summary of Land Uses

Land Use Type	Acreage	Trip Rate	Unit Type	No. Units	Total Trips	Total VMT
Junior college (2 yrs)		1.18	students	250.00	295.00	2,679.34
Junior College		27.49	1000 sq ft	59.49	1,635.38	14,674.27
					1,930.38	17,353.61

Vehicle Fleet Mix

Vehicle Type	Percent Type	Non-Catalyst	Catalyst	Diesel
Light Auto	51.1	0.4	99.4	0.2
Light Truck < 3750 lbs	7.3	1.4	95.9	2.7
Light Truck 3751-5750 lbs	23.1	0.4	99.6	0.0
Med Truck 5751-8500 lbs	10.8	0.9	99.1	0.0

Vehicle Fleet Mix

Vehicle Type	Percent Type	Non-Catalyst	Catalyst	Diesel
Lite-Heavy Truck 8501-10,000 lbs	1.7	0.0	82.4	17.6
Lite-Heavy Truck 10,001-14,000 lbs	0.5	0.0	60.0	40.0
Med-Heavy Truck 14,001-33,000 lbs	0.9	0.0	22.2	77.8
Heavy-Heavy Truck 33,001-60,000 lbs	0.6	0.0	0.0	100.0
Other Bus	0.1	0.0	0.0	100.0
Urban Bus	0.1	0.0	0.0	100.0
Motorcycle	2.8	50.0	50.0	0.0
School Bus	0.1	0.0	0.0	100.0
Motor Home	0.9	0.0	88.9	11.1

Travel Conditions

	Residential			Commercial		
	Home-Work	Home-Shop	Home-Other	Commuter	Non-Work	Customer
Urban Trip Length (miles)	12.7	7.0	9.5	13.3	7.4	8.9
Rural Trip Length (miles)	17.6	12.1	14.9	15.4	9.6	12.6
Trip speeds (mph)	30.0	30.0	30.0	30.0	30.0	30.0
% of Trips - Residential	32.9	18.0	49.1			
% of Trips - Commercial (by land use)						
Junior college (2 yrs)				5.0	2.5	92.5
Junior College				2.0	1.0	97.0

Urbemis 2007 Version 9.2.4

Combined Winter Emissions Reports (Pounds/Day)

File Name: I:\pdata\00000100\10P\WPWIN\EddieT\Programs\Air\URBEMIS\URBEMIS2007\Marymount Scenario1.urb924

Project Name: Marymount Scenario 1

Project Location: South Coast AQMD

On-Road Vehicle Emissions Based on: Version : Emfac2007 V2.3 Nov 1 2006

Off-Road Vehicle Emissions Based on: OFFROAD2007

Summary Report:

AREA SOURCE EMISSION ESTIMATES

	<u>ROG</u>	<u>NOx</u>	<u>CO</u>	<u>SO2</u>	<u>PM10</u>	<u>PM2.5</u>	<u>CO2</u>
TOTALS (lbs/day, unmitigated)	0.52	0.62	0.52	0.00	0.00	0.00	742.72

OPERATIONAL (VEHICLE) EMISSION ESTIMATES

	<u>ROG</u>	<u>NOx</u>	<u>CO</u>	<u>SO2</u>	<u>PM10</u>	<u>PM2.5</u>	<u>CO2</u>
TOTALS (lbs/day, unmitigated)	12.11	17.69	125.69	0.15	29.99	5.84	16,190.40

SUM OF AREA SOURCE AND OPERATIONAL EMISSION ESTIMATES

	<u>ROG</u>	<u>NOx</u>	<u>CO</u>	<u>SO2</u>	<u>PM10</u>	<u>PM2.5</u>	<u>CO2</u>
TOTALS (lbs/day, unmitigated)	12.63	18.31	126.21	0.15	29.99	5.84	16,933.12

Operational Unmitigated Detail Report:

OPERATIONAL EMISSION ESTIMATES Winter Pounds Per Day, Unmitigated

<u>Source</u>	ROG	NOX	CO	SO2	PM10	PM25	CO2
Junior college (2 yrs)	2.45	2.73	19.42	0.02	4.63	0.90	2,500.74
Junior College	9.66	14.96	106.27	0.13	25.36	4.94	13,689.66
TOTALS (lbs/day, unmitigated)	12.11	17.69	125.69	0.15	29.99	5.84	16,190.40

Operational Settings:

Does not include correction for passby trips

Does not include double counting adjustment for internal trips

Analysis Year: 2014 Temperature (F): 60 Season: Winter

Emfac: Version : Emfac2007 V2.3 Nov 1 2006

Summary of Land Uses

Land Use Type	Acreage	Trip Rate	Unit Type	No. Units	Total Trips	Total VMT
Junior college (2 yrs)		1.18	students	250.00	295.00	2,679.34
Junior College		27.49	1000 sq ft	59.49	1,635.38	14,674.27
					1,930.38	17,353.61

Vehicle Fleet Mix

Vehicle Type	Percent Type	Non-Catalyst	Catalyst	Diesel
Light Auto	51.1	0.4	99.4	0.2
Light Truck < 3750 lbs	7.3	1.4	95.9	2.7
Light Truck 3751-5750 lbs	23.1	0.4	99.6	0.0
Med Truck 5751-8500 lbs	10.8	0.9	99.1	0.0

Vehicle Fleet Mix

Vehicle Type	Percent Type	Non-Catalyst	Catalyst	Diesel
Lite-Heavy Truck 8501-10,000 lbs	1.7	0.0	82.4	17.6
Lite-Heavy Truck 10,001-14,000 lbs	0.5	0.0	60.0	40.0
Med-Heavy Truck 14,001-33,000 lbs	0.9	0.0	22.2	77.8
Heavy-Heavy Truck 33,001-60,000 lbs	0.6	0.0	0.0	100.0
Other Bus	0.1	0.0	0.0	100.0
Urban Bus	0.1	0.0	0.0	100.0
Motorcycle	2.8	50.0	50.0	0.0
School Bus	0.1	0.0	0.0	100.0
Motor Home	0.9	0.0	88.9	11.1

Travel Conditions

	Residential			Commercial		
	Home-Work	Home-Shop	Home-Other	Commuter	Non-Work	Customer
Urban Trip Length (miles)	12.7	7.0	9.5	13.3	7.4	8.9
Rural Trip Length (miles)	17.6	12.1	14.9	15.4	9.6	12.6
Trip speeds (mph)	30.0	30.0	30.0	30.0	30.0	30.0
% of Trips - Residential	32.9	18.0	49.1			
% of Trips - Commercial (by land use)						
Junior college (2 yrs)				5.0	2.5	92.5
Junior College				2.0	1.0	97.0

Urbemis 2007 Version 9.2.4

Combined Annual Emissions Reports (Tons/Year)

File Name: I:\pdata\00000100\10P\WPWIN\EddieT\Programs\Air\URBEMIS\URBEMIS2007\Marymount Scenario1.urb924

Project Name: Marymount Scenario 1

Project Location: South Coast AQMD

On-Road Vehicle Emissions Based on: Version : Emfac2007 V2.3 Nov 1 2006

Off-Road Vehicle Emissions Based on: OFFROAD2007

Summary Report:

AREA SOURCE EMISSION ESTIMATES

	<u>ROG</u>	<u>NOx</u>	<u>CO</u>	<u>SO2</u>	<u>PM10</u>	<u>PM2.5</u>	<u>CO2</u>
TOTALS (tons/year, unmitigated)	0.15	0.12	0.68	0.00	0.00	0.00	136.55

OPERATIONAL (VEHICLE) EMISSION ESTIMATES

	<u>ROG</u>	<u>NOx</u>	<u>CO</u>	<u>SO2</u>	<u>PM10</u>	<u>PM2.5</u>	<u>CO2</u>
TOTALS (tons/year, unmitigated)	2.16	2.86	23.61	0.03	5.48	1.06	3,161.55

SUM OF AREA SOURCE AND OPERATIONAL EMISSION ESTIMATES

	<u>ROG</u>	<u>NOx</u>	<u>CO</u>	<u>SO2</u>	<u>PM10</u>	<u>PM2.5</u>	<u>CO2</u>
TOTALS (tons/year, unmitigated)	2.31	2.98	24.29	0.03	5.48	1.06	3,298.10

Operational Unmitigated Detail Report:

OPERATIONAL EMISSION ESTIMATES Annual Tons Per Year, Unmitigated

<u>Source</u>	ROG	NOX	CO	SO2	PM10	PM25	CO2
Junior college (2 yrs)	0.52	0.44	3.65	0.00	0.85	0.16	488.31
Junior College	1.64	2.42	19.96	0.03	4.63	0.90	2,673.24
TOTALS (tons/year, unmitigated)	2.16	2.86	23.61	0.03	5.48	1.06	3,161.55

Operational Settings:

Does not include correction for passby trips

Does not include double counting adjustment for internal trips

Analysis Year: 2014 Season: Annual

Emfac: Version : Emfac2007 V2.3 Nov 1 2006

Summary of Land Uses

Land Use Type	Acreage	Trip Rate	Unit Type	No. Units	Total Trips	Total VMT
Junior college (2 yrs)		1.18	students	250.00	295.00	2,679.34
Junior College		27.49	1000 sq ft	59.49	1,635.38	14,674.27
					1,930.38	17,353.61

Vehicle Fleet Mix

Vehicle Type	Percent Type	Non-Catalyst	Catalyst	Diesel
Light Auto	51.1	0.4	99.4	0.2
Light Truck < 3750 lbs	7.3	1.4	95.9	2.7
Light Truck 3751-5750 lbs	23.1	0.4	99.6	0.0
Med Truck 5751-8500 lbs	10.8	0.9	99.1	0.0

Vehicle Fleet Mix

Vehicle Type	Percent Type	Non-Catalyst	Catalyst	Diesel
Lite-Heavy Truck 8501-10,000 lbs	1.7	0.0	82.4	17.6
Lite-Heavy Truck 10,001-14,000 lbs	0.5	0.0	60.0	40.0
Med-Heavy Truck 14,001-33,000 lbs	0.9	0.0	22.2	77.8
Heavy-Heavy Truck 33,001-60,000 lbs	0.6	0.0	0.0	100.0
Other Bus	0.1	0.0	0.0	100.0
Urban Bus	0.1	0.0	0.0	100.0
Motorcycle	2.8	50.0	50.0	0.0
School Bus	0.1	0.0	0.0	100.0
Motor Home	0.9	0.0	88.9	11.1

Travel Conditions

	Residential			Commercial		
	Home-Work	Home-Shop	Home-Other	Commute	Non-Work	Customer
Urban Trip Length (miles)	12.7	7.0	9.5	13.3	7.4	8.9
Rural Trip Length (miles)	17.6	12.1	14.9	15.4	9.6	12.6
Trip speeds (mph)	30.0	30.0	30.0	30.0	30.0	30.0
% of Trips - Residential	32.9	18.0	49.1			
% of Trips - Commercial (by land use)						
Junior college (2 yrs)				5.0	2.5	92.5
Junior College				2.0	1.0	97.0

## Construction Emissions

### Demolition

Duration (days): 66

Equipment	Emission Factors (pounds/hour)			Hours/day	Quantity	Emissions (pounds/hour)			Emissions (tons/year)		
	CO <sub>2</sub>	CH <sub>4</sub>	N <sub>2</sub> O			CO <sub>2</sub>	CH <sub>4</sub>	N <sub>2</sub> O	CO <sub>2</sub>	CH <sub>4</sub>	N <sub>2</sub> O
Concrete/Industrial Saws	58.5	0.0114	0.0015	8	1	58.5	0.0114	0.0015	15.4440	0.0030	0.0004
Rubber Tired Dozers	239.1	0.0305	0.0062	8	1	239.1	0.0305	0.0062	63.1224	0.0081	0.0016
Tractors/Loaders/Backhoes	66.8	0.0092	0.0017	8	1	66.8	0.0092	0.0017	17.6352	0.0024	0.0004
Crushing/Proc. Equipment	132.3	0.0194	0.0035	8	1	132.3	0.0194	0.0035	34.9272	0.0051	0.0009
Off-Highway Tractors	151.4	0.0213	0.0040	8	1	151.4	0.0213	0.0040	39.9696	0.0056	0.0011
<b>Total Emissions for Mass Grading</b>									171.0984	0.0242	0.0045

### Mass Grading

Duration (days): 66

Equipment	Emission Factors (pounds/hour)			Hours/day	Quantity	Emissions (pounds/hour)			Emissions (tons/year)		
	CO <sub>2</sub>	CH <sub>4</sub>	N <sub>2</sub> O			CO <sub>2</sub>	CH <sub>4</sub>	N <sub>2</sub> O	CO <sub>2</sub>	CH <sub>4</sub>	N <sub>2</sub> O
Graders	132.7	0.0155	0.0035	8	1	132.7	0.0155	0.0035	35.0328	0.0041	0.0009
Rubber Tired Dozers	239.1	0.0305	0.0062	8	1	239.1	0.0305	0.0062	63.1224	0.0081	0.0016
Tractors/Loaders/Backhoes	66.8	0.0092	0.0017	8	1	66.8	0.0092	0.0017	17.6352	0.0024	0.0004
Scrapers	262.5	0.0289	0.0068	8	1	262.5	0.0289	0.0068	69.3000	0.0076	0.0018
Other Construction Equipment	122.8	0.0095	0.0032	8	1	122.8	0.0095	0.0032	32.4192	0.0025	0.0008
Off-Highway Trucks	260.1	0.0224	0.0067	8	1	260.1	0.0224	0.0067	68.6664	0.0059	0.0018
<b>Total Emissions for Mass Grading</b>									286.1760	0.0306	0.0074

### Paving

Duration (days): 22

Equipment	Emission Factors			Hours/day	Quantity	Emissions (pounds/hour)			Emissions (tons/year)		
	CO <sub>2</sub>	CH <sub>4</sub>	N <sub>2</sub> O			CO <sub>2</sub>	CH <sub>4</sub>	N <sub>2</sub> O	CO <sub>2</sub>	CH <sub>4</sub>	N <sub>2</sub> O
Cement and Mortar Mixers	7.2	0.0009	0.0002	8	4	28.8	0.0036	0.0008	2.5344	0.0003	0.0001
Pavers	77.9	0.016	0.002	8	1	77.9	0.0160	0.0020	6.8552	0.0014	0.0002
Paving Equipment	68.9	0.012	0.0018	8	1	68.9	0.0120	0.0018	6.0632	0.0011	0.0002
Rollers	67.1	0.0106	0.0018	8	1	67.1	0.0106	0.0018	5.9048	0.0009	0.0002
Tractors/Loaders/Backhoes	66.8	0.0092	0.0017	8	1	66.8	0.0092	0.0017	5.8784	0.0008	0.0001
<b>Total Emissions for Paving</b>									27.2360	0.0045	0.0007

### Building Construction

Duration (days): 572

Equipment	Emission Factors			Hours/day	Quantity	Emissions (pounds/hour)			Emissions (tons/year)		
	CO <sub>2</sub>	CH <sub>4</sub>	N <sub>2</sub> O			CO <sub>2</sub>	CH <sub>4</sub>	N <sub>2</sub> O	CO <sub>2</sub>	CH <sub>4</sub>	N <sub>2</sub> O
Welders	25.6	0.0073	0.0007	6	1	25.6	0.0073	0.0007	43.9296	0.0125	0.0012
Forklifts	54.4	0.0062	0.0014	6	1	54.4	0.0062	0.0014	93.3504	0.0106	0.0024
Cranes	128.7	0.0144	0.0033	6	1	128.7	0.0144	0.0033	220.8492	0.0247	0.0057
Generator Sets	61	0.0087	0.0016	8	1	61	0.0087	0.0016	139.5680	0.0199	0.0037
Trenchers	58.7	0.0151	0.0015	8	1	58.7	0.0151	0.0015	134.3056	0.0345	0.0034
Off-Highway Trucks	260.1	0.0224	0.0067	8	1	260.1	0.0224	0.0067	595.1088	0.0513	0.0153
Tractors/Loaders/Backhoes	66.8	0.0092	0.0017	8	1	66.8	0.0092	0.0017	152.8384	0.0210	0.0039
<b>Total Emissions for Building Construction</b>									1379.9500	0.1621	0.0344

### Total Construction Emissions

<b>tons/year</b>	1864.46	0.22	0.05
<b>metric tons/year</b>	1,691.41	0.20	0.04
<b>metric tons CO<sub>2</sub> eq/year</b>	1,691.41	62.29	0.89

Notes:

Construction Equipment Emission Factor Source: Provided by SCAQMD.  
 Refer to the URBEMIS 2007 assumptions and model output for construction equipment assumptions

### Emissions From Natural Gas Consumed By Land Uses

Land Use	Amount	Cubic feet per unit/square feet/customer per month	CO	ROG	NO <sub>x</sub>	NO <sub>x</sub>	SO <sub>x</sub>	PM <sub>10</sub>	CO <sub>2</sub>	N <sub>2</sub> O	CH <sub>4</sub>
			2.00E+01	5.30E+00	Residential 8.00E+01	Non-Residential 1.20E+02	negligible	2.00E-01	0.12	2.20E-06	2.30E-06
<b>Residential</b>											
Single Family Units		6,665	0.00	0.00	0.00	0.00	--	0.00	0.00	0.00	0.00
Multi-Family Units		4,011.5	0.00	0.00	0.00	0.00	--	0.00	0.00	0.00	0.00
<b>NonResidential</b>											
Industrial		241,611	0.00	0.00	0.00	0.00	--	0.00	0.00	0.00	0.00
Hotel/Motel		4.8	0.00	0.00	0.00	0.00	--	0.00	0.00	0.00	0.00
Retail/Shopping Center		2.9	0.00	0.00	0.00	0.00	--	0.00	0.00	0.00	0.00
Office		2.0	0.00	0.00	0.00	0.00	--	0.00	0.00	0.00	0.00
Blank	77,504	5.0	21,233.97	5,627.00	84,935.89		--	212.34	1,529.68	0.03	0.03
<b>TOTAL - pounds per day</b>	--	--	<b>2.12E+04</b>	<b>5.63E+03</b>	<b>8.49E+04</b>	<b>0.00E+00</b>	--	<b>2.12E+02</b>	<b>1,529.68</b>	<b>0.03</b>	<b>0.03</b>
<b>TOTAL - tons per year</b>	--	--	<b>3.88E+03</b>	<b>1.03E+03</b>	<b>1.55E+04</b>	<b>0.00E+00</b>	--	<b>3.88E+01</b>	<b>279.1674</b>	<b>0.0051</b>	<b>0.0054</b>
<b>TOTAL - metric tons per year</b>	--	--	<b>3.52E+03</b>	<b>9.32E+02</b>	<b>1.41E+04</b>	<b>0.00E+00</b>	--	<b>3.52E+01</b>	<b>2.53E+02</b>	<b>4.64E-03</b>	<b>4.85E-03</b>

	CO <sub>2</sub>	N <sub>2</sub> O	CH <sub>4</sub>
<b>metric tons per year</b>	253.26	0.00	0.00
<b>metric tons CO<sub>2</sub>eq per year</b>	253.26	1.44	0.10

Notes:

1. Usage rate; average for SCE and LADWP.

Source:

South Coast Air Quality Management District, *CEQA Air Quality Handbook*, November 1993, Table A9-12.

### Emissions From Electricity Consumed By Land Uses

Land Use	Amount	kilowatt-hours per year <sup>1</sup>	CO 2.00E-04	ROG 1.00E-05	NO <sub>x</sub> 1.15E-03	SO <sub>x</sub> 1.20E-04	PM <sub>10</sub> 4.00E-05	CO <sub>2</sub> 0.772	N <sub>2</sub> O 6.59E-06	CH <sub>4</sub> 4.04E-05
Residential (Dwelling Units)		5626.5	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Food Store (SF)		53.3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Restaurant (SF)		47.45	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Hospitals (SF)		21.7	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Retail (SF)		13.55	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
College/University (SF)	77,504	11.55	0.49	0.02	2.82	0.29	0.10	1,893.35	0.02	0.10
High School (SF)		10.5	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Elementary School (SF)		5.9	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Office (SF)		12.95	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Hotel/Motel (SF)		9.95	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Warehouse (SF)		4.35	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Miscellaneous (SF)		10.5	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Blank			0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
<b>TOTAL - pounds per day</b>	--	--	<b>4.91E-04</b>	<b>2.45E-02</b>	<b>2.82E+00</b>	<b>2.94E-01</b>	<b>9.81E-02</b>	<b>1,893.35</b>	<b>0.02</b>	<b>0.10</b>
<b>TOTAL - tons per year</b>	--	--	<b>8.95E-05</b>	<b>4.48E-03</b>	<b>5.15E-01</b>	<b>5.37E-02</b>	<b>1.79E-02</b>	<b>345.54</b>	<b>0.00</b>	<b>0.02</b>
<b>TOTAL - metric tons per year</b>	--	--	<b>8.12E-05</b>	<b>4.06E-03</b>	<b>4.67E-01</b>	<b>4.87E-02</b>	<b>1.62E-02</b>	<b>313.47</b>	<b>0.00</b>	<b>0.02</b>

	CO <sub>2</sub>	N <sub>2</sub> O	CH <sub>4</sub>
<b>metric tons per year</b>	313.47	0.00	0.02
<b>metric tons CO<sub>2</sub>eq per year</b>	313.47	0.83	0.34

Notes:

1. Usage rate; average for SCE and LADWP.

Source:

South Coast Air Quality Management District, *CEQA Air Quality Handbook*, November 1993, Table A9-11.

Source for greenhouse gas emissions rates:

U.S. Energy Information Administration, *Domestic Electricity Emissions Factors 1999-2002*, October 2007. <http://www.eia.doe.gov/oiat/1605/techassist.html>

### Mobile Source Emissions Calculations

	Total	Breakdown		Emission Factor		Total Emis Passenger	Total Emis Delivery	Passnger	Delivery	Total Emissions	
	VMT	Passnger	Delivery	Passnger	Delivery	pounds/day		tons/year	tons/year	tons/year	metric tons/year
CO	17,353	16485.35	867.65	0.00709228	0.01407778	116.92	12.21	21.34	2.23	23.57	21.38
NO <sub>x</sub>	17,353	16485.35	867.65	0.00071158	0.01577311	11.73	13.69	2.14	2.50	4.64	4.21
N <sub>2</sub> O <sup>1</sup>	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	0.23	0.21
ROG	17,353	16485.35	867.65	0.00074567	0.00206295	12.29	1.79	2.24	0.33	2.57	2.33
SO <sub>x</sub>	17,353	16485.35	867.65	0.00001072	0.00002682	0.18	0.02	0.03	0.00	0.04	0.03
PM <sub>10</sub>	17,353	16485.35	867.65	0.00009067	0.00059956	1.49	0.52	0.27	0.09	0.37	0.33
PM <sub>2.5</sub>	17,353	16485.35	867.65	0.00005834	0.00050174	0.96	0.44	0.18	0.08	0.25	0.23
CH <sub>4</sub>	17,353	16485.35	867.65	0.00006707	0.00009703	1.11	0.08	0.20	0.02	0.22	0.20
CO <sub>2</sub>	17,353	16485.35	867.65	1.10087435	2.78163459	18148.30	2413.49	3312.06	440.46	3752.53	3,404.23

	CO <sub>2</sub>	N <sub>2</sub> O	CH <sub>4</sub>
metric tons per year	3,404.23	0.21	0.20
metric tons CO <sub>2</sub> eq per year	3,404.23	63.57	4.14

Notes:

1. VMT based upon URBEMIS 2007 model output.
2. Emission Factor based upon EMFAC 2007 (version 2.3), *Highest (Most Conservative) Emission Factors fo On-Road Passenger Vehicles and Delivery Trucks* .
3. Breakdown of Passenger and Delivery Trucks assumes 95% auto and 5% truck.
4. Emission Factor for N<sub>2</sub>O based upon a conversion ratio of 0.04873 from NO<sub>x</sub> to N<sub>2</sub>O. Based upon California Air Resources Board: *Estimates of Nitrous Oxide*

### Water Consumption Indirect Emissions

	Acre Feet per year	Electricity Usage kWh/year	Units	CO	ROG	NO <sub>x</sub>	SO <sub>x</sub>	PM <sub>10</sub>	CO <sub>2</sub>	N <sub>2</sub> O	CH <sub>4</sub>
				2.00E-04	1.00E-05	1.15E-03	1.20E-04	4.00E-05	0.772	6.59E-06	4.04E-05
Project Demand	39.9	66,473	pounds/yr	1.33E+01	6.65E-01	7.64E+01	7.98E+00	2.66E+00	5.13E+04	4.38E-01	2.68E+00
			tons/yr	6.65E-03	3.32E-04	3.82E-02	3.99E-03	1.33E-03	2.57E+01	2.19E-04	1.34E-03
			mt/yr	6.03E-03	3.02E-04	3.47E-02	3.62E-03	1.21E-03	2.33E+01	1.99E-04	1.22E-03
<b>MTCO<sub>2</sub>EQ</b>									23.28	0.07	0.03

Energy Factor            1,666 kWh/acre-foot

Based on energy usage factors for water conveyance from the California Energy Commission, Water Energy Use in California, Accessed May 2009.  
<http://www.energy.ca.gov/research/iaw/industry/water.html>

marymount2012.rts

Title : South Coast Air Basin Avg Winter CYr 2012 Default Title  
Version : Emfac2007 V2.3 Nov 1 2006  
Run Date : 2007/06/07 10:28:55  
Scen Year: 2012 -- All model years in the range 1968 to 2012 selected  
Season : Winter  
Area : South Coast

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Year: 2012 -- Model Years 1968 to 2012 Inclusive -- Winter  
Emfac2007 Emission Factors: V2.3 Nov 1 2006

Average South Coast Basin Average Basin

Table 1: Running Exhaust Emissions (grams/mile)

0% Pollutant Name: Reactive Org Gases Temperature: 50F Relative Humidity:

Speed MPH	LDA	LDT	MDT	HDT	UBUS	MCY	ALL
5	0.271	0.373	0.636	5.918	3.731	5.450	0.644
35	0.048	0.069	0.108	0.542	0.690	2.343	0.100

0% Pollutant Name: Carbon Monoxide Temperature: 50F Relative Humidity:

Speed MPH	LDA	LDT	MDT	HDT	UBUS	MCY	ALL
5	3.701	5.191	7.264	19.121	30.284	35.087	5.599
35	1.938	2.633	2.915	4.157	5.455	23.034	2.532

0% Pollutant Name: Oxides of Nitrogen Temperature: 50F Relative Humidity:

Speed MPH	LDA	LDT	MDT	HDT	UBUS	MCY	ALL
5	0.387	0.717	1.199	22.409	33.003	1.438	1.649
35	0.227	0.406	0.761	9.475	14.966	1.520	0.804

0% Pollutant Name: Carbon Dioxide Temperature: 50F Relative Humidity:

Speed MPH	LDA	LDT	MDT	HDT	UBUS	MCY	ALL
5	951.901	1183.288	1702.692	2861.803	2684.771	250.084	1212.968
35	309.937	386.449	522.956	1522.170	2101.808	128.635	419.713

0% Pollutant Name: Sulfur Dioxide Temperature: 50F Relative Humidity:

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Speed MPH	LDA	LDT	MDT	HDT	UBUS	MCY	ALL
5	0.009	0.011	0.016	0.027	0.026	0.003	0.012
35	0.003	0.004	0.005	0.015	0.020	0.002	0.004

0% Pollutant Name: PM2.5 Temperature: 50F Relative Humidity:

Speed MPH	LDA	LDT	MDT	HDT	UBUS	MCY	ALL
5	0.049	0.101	0.102	1.144	0.666	0.035	0.123
35	0.008	0.017	0.018	0.260	0.162	0.016	0.024

0% Pollutant Name: PM2.5 - Tire Wear Temperature: 50F Relative Humidity:

Speed MPH	LDA	LDT	MDT	HDT	UBUS	MCY	ALL
5	0.002	0.002	0.002	0.006	0.002	0.001	0.002
35	0.002	0.002	0.002	0.006	0.002	0.001	0.002

0% Pollutant Name: PM2.5 - Break Wear Temperature: 50F Relative Humidity:

Speed MPH	LDA	LDT	MDT	HDT	UBUS	MCY	ALL
5	0.005	0.005	0.005	0.009	0.005	0.003	0.006
35	0.005	0.005	0.005	0.009	0.005	0.003	0.006

0% Pollutant Name: Gasoline - mi/gal Temperature: 50F Relative Humidity:

Speed MPH	LDA	LDT	MDT	HDT	UBUS	MCY	ALL
5	9.245	7.399	5.110	3.403	3.371	27.495	8.140
35	28.325	22.666	16.782	17.214	17.072	51.438	24.928

0% Pollutant Name: Diesel - mi/gal Temperature: 50F Relative Humidity:

Speed MPH	LDA	LDT	MDT	HDT	UBUS	MCY	ALL
5	27.949	29.002	19.645	4.229	3.659	0.000	7.793
35	27.949	29.002	19.645	5.981	3.659	0.000	9.148



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180	6.560	7.981	16.833	29.027	21.870	11.077	10.108
240	6.994	8.514	17.742	30.713	22.713	12.913	10.736
300	7.388	8.999	18.591	32.315	23.550	14.558	11.315
360	7.744	9.435	19.380	33.833	24.380	16.012	11.845
420	8.062	9.823	20.109	35.267	25.204	17.275	12.326
480	8.340	10.162	20.778	36.617	26.022	18.346	12.758
540	8.580	10.453	21.388	37.883	26.833	19.227	13.141
600	8.781	10.696	21.937	39.065	27.638	19.917	13.475
660	8.943	10.890	22.426	40.162	28.436	20.415	13.760
720	9.067	11.037	22.855	41.176	29.228	20.723	13.996

ALL Pollutant Name: Oxides of Nitrogen Temperature: 50F Relative Humidity:

Time min	LDA	LDT	MDT	HDT	UBUS	MCY	ALL
5	0.219	0.359	0.939	0.670	0.725	0.224	0.416
10	0.248	0.401	1.115	0.997	1.090	0.259	0.492
20	0.301	0.476	1.426	1.570	1.729	0.322	0.627
30	0.345	0.538	1.682	2.038	2.250	0.376	0.739
40	0.379	0.589	1.884	2.399	2.653	0.418	0.827
50	0.405	0.626	2.032	2.655	2.937	0.451	0.891
60	0.422	0.651	2.125	2.805	3.103	0.474	0.931
120	0.447	0.697	2.238	2.870	3.175	0.486	0.981
180	0.479	0.744	2.277	2.858	3.163	0.481	1.016
240	0.476	0.739	2.261	2.841	3.144	0.470	1.008
300	0.471	0.730	2.239	2.817	3.120	0.456	0.998
360	0.463	0.719	2.208	2.788	3.089	0.439	0.984
420	0.455	0.705	2.170	2.753	3.052	0.420	0.967
480	0.444	0.687	2.125	2.712	3.009	0.397	0.946
540	0.431	0.667	2.072	2.665	2.960	0.372	0.922
600	0.417	0.644	2.012	2.612	2.904	0.344	0.894
660	0.400	0.618	1.944	2.553	2.842	0.313	0.863
720	0.382	0.588	1.868	2.489	2.774	0.280	0.828

ALL Pollutant Name: Carbon Dioxide Temperature: 50F Relative Humidity:

Time min	LDA	LDT	MDT	HDT	UBUS	MCY	ALL
5	11.327	14.221	18.469	6.576	3.875	21.496	13.292
10	13.402	16.684	22.867	9.879	6.635	23.984	15.976
20	17.948	22.122	32.238	16.418	12.105	28.845	21.779
30	23.025	28.243	42.379	22.866	17.509	33.549	28.160
40	28.633	35.045	53.289	29.224	22.845	38.099	35.119
50	34.772	42.529	64.968	35.490	28.113	42.493	42.656
60	41.441	50.696	77.416	41.667	33.315	46.731	50.772
120	89.902	111.069	160.786	68.586	55.892	66.163	107.659
180	102.641	126.687	184.512	79.425	65.491	68.802	123.060
240	115.193	142.105	207.688	89.624	74.523	71.287	138.180
300	127.559	157.325	230.316	99.184	82.989	73.619	153.019
360	139.740	172.345	252.394	108.105	90.888	75.798	167.576
420	151.734	187.165	273.923	116.387	98.221	77.823	181.852
480	163.542	201.787	294.903	124.029	104.987	79.695	195.847
540	175.163	216.209	315.333	131.032	111.188	81.414	209.560
600	186.599	230.432	335.214	137.395	116.821	82.979	222.992
660	197.848	244.456	354.547	143.120	121.889	84.391	236.142

720 208.911 258.281 373.329 148.205 126.390 85.650 249.011

Pol l utant Name: Sul fur Di oxi de Temperature: 50F Rel ati ve Humi di ty:

ALL

Time mi n	LDA	LDT	MDT	HDT	UBUS	MCY	ALL
5	0.000	0.000	0.000	0.000	0.000	0.000	0.000
10	0.000	0.000	0.000	0.000	0.000	0.000	0.000
20	0.000	0.000	0.000	0.000	0.000	0.000	0.000
30	0.000	0.000	0.001	0.001	0.001	0.001	0.000
40	0.000	0.000	0.001	0.001	0.001	0.001	0.001
50	0.000	0.001	0.001	0.001	0.001	0.001	0.001
60	0.001	0.001	0.001	0.001	0.001	0.001	0.001
120	0.001	0.001	0.002	0.001	0.001	0.001	0.001
180	0.001	0.001	0.002	0.001	0.001	0.001	0.001
240	0.001	0.002	0.002	0.001	0.001	0.001	0.002
300	0.001	0.002	0.003	0.002	0.001	0.001	0.002
360	0.001	0.002	0.003	0.002	0.001	0.001	0.002
420	0.002	0.002	0.003	0.002	0.001	0.001	0.002
480	0.002	0.002	0.003	0.002	0.001	0.001	0.002
540	0.002	0.002	0.003	0.002	0.002	0.001	0.002
600	0.002	0.002	0.004	0.002	0.002	0.001	0.002
660	0.002	0.003	0.004	0.002	0.002	0.001	0.003
720	0.002	0.003	0.004	0.002	0.002	0.001	0.003

Pol l utant Name: PM2. 5 Temperature: 50F Rel ati ve Humi di ty:

ALL

Time mi n	LDA	LDT	MDT	HDT	UBUS	MCY	ALL
5	0.001	0.001	0.001	0.001	0.000	0.009	0.001
10	0.001	0.002	0.002	0.001	0.001	0.008	0.002
20	0.002	0.004	0.004	0.002	0.002	0.006	0.003
30	0.003	0.006	0.006	0.002	0.002	0.005	0.005
40	0.004	0.008	0.008	0.003	0.003	0.004	0.006
50	0.005	0.010	0.009	0.003	0.003	0.003	0.007
60	0.006	0.011	0.010	0.004	0.004	0.003	0.008
120	0.009	0.018	0.016	0.005	0.005	0.007	0.012
180	0.010	0.019	0.017	0.005	0.005	0.010	0.013
240	0.010	0.020	0.018	0.005	0.005	0.013	0.014
300	0.011	0.022	0.019	0.006	0.006	0.015	0.015
360	0.011	0.023	0.020	0.006	0.006	0.018	0.016
420	0.012	0.024	0.020	0.006	0.006	0.020	0.016
480	0.012	0.024	0.021	0.006	0.006	0.021	0.017
540	0.013	0.025	0.022	0.006	0.006	0.022	0.017
600	0.013	0.026	0.022	0.007	0.007	0.023	0.018
660	0.013	0.026	0.022	0.007	0.007	0.024	0.018
720	0.013	0.026	0.023	0.007	0.007	0.024	0.018

Version : Emfac2007 V2.3 Nov 1 2006  
 Run Date : 2007/06/07 10:28:55  
 Scen Year: 2012 -- All model years in the range 1968 to 2012 selected  
 Season : Winter  
 Area : South Coast

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Year: 2012 -- Model Years 1968 to 2012 Inclusive -- Winter  
 Emfac2007 Emission Factors: V2.3 Nov 1 2006

Average South Coast Basin Average Basin

Table 4: Hot Soak Emissions (grams/trip)

ALL Pollutant Name: Reactive Org Gases Temperature: 50F Relative Humidity:

Time min	LDA	LDT	MDT	HDT	UBUS	MCY	ALL
5	0.050	0.048	0.033	0.009	0.045	0.093	0.044
10	0.092	0.089	0.061	0.017	0.082	0.172	0.082
20	0.157	0.152	0.105	0.029	0.140	0.296	0.140
30	0.202	0.195	0.136	0.037	0.180	0.384	0.180
40	0.219	0.212	0.148	0.040	0.195	0.418	0.196

Hot soak results are scaled to reflect zero emissions for trip lengths of less than 5 minutes (about 25% of in-use trips).

Title : South Coast Air Basin Avg Winter CYr 2012 Default Title  
 Version : Emfac2007 V2.3 Nov 1 2006  
 Run Date : 2007/06/07 10:28:55  
 Scen Year: 2012 -- All model years in the range 1968 to 2012 selected  
 Season : Winter  
 Area : South Coast

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Year: 2012 -- Model Years 1968 to 2012 Inclusive -- Winter  
 Emfac2007 Emission Factors: V2.3 Nov 1 2006

Average South Coast Basin Average Basin

Table 5a: Partial Day Diurnal Loss Emissions

(grams/hour)

ALL Pollutant Name: Reactive Org Gases Temperature: ALL Relative Humidity:

Temp degF	LDA	LDT	MDT	HDT	UBUS	MCY	ALL
50	0.012	0.012	0.010	0.001	0.000	0.012	0.011

Title : South Coast Air Basin Avg Winter CYr 2012 Default Title  
 Version : Emfac2007 V2.3 Nov 1 2006  
 Run Date : 2007/06/07 10:28:55  
 Scen Year: 2012 -- All model years in the range 1968 to 2012 selected  
 Season : Winter  
 Area : South Coast

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Year: 2012 -- Model Years 1968 to 2012 Inclusive -- Winter  
 Emfac2007 Emission Factors: V2.3 Nov 1 2006

South Coast Basin Average Basin  
 Average

Table 5b: Multi-Day Diurnal Loss Emissions

(grams/hour)

Temp degF	LDA	LDT	MDT	HDT	UBUS	MCY	ALL
50	0.001	0.001	0.001	0.000	0.000	0.001	0.001

Title : South Coast Air Basin Avg Winter CYr 2012 Default Title  
 Version : Emfac2007 V2.3 Nov 1 2006  
 Run Date : 2007/06/07 10:28:55  
 Scen Year: 2012 -- All model years in the range 1968 to 2012 selected  
 Season : Winter  
 Area : South Coast

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Year: 2012 -- Model Years 1968 to 2012 Inclusive -- Winter  
 Emfac2007 Emission Factors: V2.3 Nov 1 2006

South Coast Basin Average Basin  
 Average

Table 6a: Partial Day Resting Loss Emissions

(grams/hour)

Temp degF	LDA	LDT	MDT	HDT	UBUS	MCY	ALL
50	0.009	0.009	0.008	0.001	0.000	0.009	0.009

Title : South Coast Air Basin Avg Winter CYr 2012 Default Title  
 Version : Emfac2007 V2.3 Nov 1 2006  
 Run Date : 2007/06/07 10:28:55  
 Scen Year: 2012 -- All model years in the range 1968 to 2012 selected  
 Season : Winter  
 Area : South Coast

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Year: 2012 -- Model Years 1968 to 2012 Inclusive -- Winter  
 Emfac2007 Emission Factors: V2.3 Nov 1 2006

Average South Coast Basin Average Basin

Table 6b: Multi-Day Resting Loss Emissions  
 (grams/hour)

Temp degF	LDA	LDT	MDT	HDT	UBUS	MCY	ALL
50	0.001	0.001	0.001	0.000	0.000	0.001	0.001

Title : South Coast Air Basin Avg Winter CYr 2012 Default Title  
 Version : Emfac2007 V2.3 Nov 1 2006  
 Run Date : 2007/06/07 10:28:55  
 Scen Year: 2012 -- All model years in the range 1968 to 2012 selected  
 Season : Winter  
 Area : South Coast

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Year: 2012 -- Model Years 1968 to 2012 Inclusive -- Winter  
 Emfac2007 Emission Factors: V2.3 Nov 1 2006

Average South Coast Basin Average Basin

Table 7: Estimated Travel Fractions

LDA	LDT	MDT	HDT	UBUS	MCY	ALL	
%VMT	0.493	0.316	0.139	0.044	0.002	0.006	1.000
%TRIP	0.473	0.279	0.186	0.053	0.000	0.008	1.000
%VEH	0.515	0.304	0.128	0.024	0.001	0.028	1.000

Title : South Coast Air Basin Avg Winter CYr 2012 Default Title  
 Version : Emfac2007 V2.3 Nov 1 2006  
 Run Date : 2007/06/07 10:28:55  
 Scen Year: 2012 -- All model years in the range 1968 to 2012 selected  
 Season : Winter  
 Area : South Coast

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Year: 2012 -- Model Years 1968 to 2012 Inclusive -- Winter  
 Emfac2007 Emission Factors: V2.3 Nov 1 2006

South Coast Basin Average Basin  
 Average

Table 8: Evaporative Running Loss Emissions

(grams/minute)

ALL Pollutant Name: Reactive Org Gases Temperature: 50F Relative Humidity:

Time min	LDA	LDT	MDT	HDT	UBUS	MCY	ALL
1	0.019	0.353	0.321	0.154	0.452	0.036	0.173
2	0.021	0.185	0.169	0.083	0.240	0.075	0.097
3	0.024	0.131	0.120	0.059	0.170	0.095	0.074
4	0.027	0.106	0.097	0.048	0.136	0.108	0.063
5	0.029	0.091	0.084	0.041	0.116	0.117	0.057
10	0.035	0.064	0.060	0.028	0.076	0.141	0.048
15	0.037	0.059	0.055	0.024	0.064	0.155	0.047
20	0.040	0.059	0.055	0.023	0.060	0.166	0.048
25	0.041	0.061	0.057	0.023	0.058	0.176	0.050
30	0.042	0.063	0.059	0.023	0.059	0.182	0.051
35	0.043	0.065	0.061	0.024	0.061	0.188	0.053
40	0.044	0.067	0.062	0.024	0.062	0.193	0.054
45	0.045	0.068	0.064	0.025	0.064	0.198	0.055
50	0.046	0.070	0.065	0.025	0.065	0.201	0.056
55	0.046	0.072	0.067	0.026	0.066	0.204	0.057
60	0.047	0.073	0.068	0.026	0.067	0.207	0.058



Western-Crestwood.1st

RECEPTOR	* * * *	BRG (DEG)	* * * *	PRED CONC (PPM)	* * * *	1	2	3	CONC/LI NK (PPM)				7	8
1. Recpt 1	*	303.	*	3.2	*	0.0	0.2	0.1	0.0	0.0	0.3	0.1	0.0	
2. Recpt 2	*	110.	*	3.1	*	0.0	0.2	0.1	0.0	0.0	0.2	0.0	0.0	
3. Recpt 3	*	8.	*	3.0	*	0.0	0.2	0.0	0.0	0.0	0.0	0.1	0.0	
4. Recpt 4	*	180.	*	3.1	*	0.0	0.0	0.1	0.0	0.0	0.3	0.0	0.0	

RECEPTOR	* * * *	9	10	11	CONC/LI NK (PPM)					16
1. Recpt 1	*	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
2. Recpt 2	*	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3. Recpt 3	*	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4. Recpt 4	*	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

RECEPTOR	* * * *	*CONC/LI NK (PPM)			
1. Recpt 1	*	17	18	19	20
2. Recpt 2	*	0.0	0.0	0.0	0.0
3. Recpt 3	*	0.0	0.0	0.0	0.0
4. Recpt 4	*	0.0	0.0	0.0	0.0

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Run Ended on 12/08/2009 at 11:54:17



Western-Toscani ni . I st

RECEPTOR	* * * *	BRG (DEG)	* * * *	PRED CONC (PPM)	* * * *	1	2	3	CONC/LI NK (PPM)				8
									4	5	6	7	
1. Recpt 1	*	302.	*	3.1	*	0.0	0.2	0.1	0.0	0.0	0.3	0.0	0.0
2. Recpt 2	*	118.	*	3.0	*	0.0	0.1	0.1	0.0	0.0	0.2	0.0	0.0
3. Recpt 3	*	5.	*	2.9	*	0.0	0.3	0.0	0.0	0.0	0.0	0.1	0.0
4. Recpt 4	*	180.	*	3.0	*	0.0	0.0	0.1	0.0	0.0	0.3	0.0	0.0

RECEPTOR	* * * *	9	10	11	CONC/LI NK (PPM)				
					12	13	14	15	16
1. Recpt 1	*	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
2. Recpt 2	*	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3. Recpt 3	*	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4. Recpt 4	*	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

RECEPTOR	* * * *	*CONC/LI NK (PPM)			
		17	18	19	20
1. Recpt 1	*	0.0	0.0	0.0	0.0
2. Recpt 2	*	0.0	0.0	0.0	0.0
3. Recpt 3	*	0.0	0.0	0.0	0.0
4. Recpt 4	*	0.0	0.0	0.0	0.0

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Run Ended on 12/08/2009 at 11:51:43

3.0.0 PC (32 BIT) VERSI ON  
 (C) COPYRI GHT 2000, TRI NI TY CONSUL TANTS

Run Began on 12/08/2009 at 11:42:31

CALINE4: CALI FORNIA LI NE SOURCE DI SPERSI ON MODEL  
 JUNE 1989 VERSI ON  
 PAGE 1

JOB: Miral este and Vi a Col i na  
 RUN: Hour 1 (WORST CASE ANGLE)  
 POLLUTANT: Carbon Monoxi de

I. SI TE VI ABL ES

U= 0.5 M/S                      ZO= 100. CM                      ALT= 0. (M)  
 BRG= WORST CASE              VD= 0.0 CM/S  
 CLAS= 7 (G)                      VS= 0.0 CM/S  
 MI XH= 1000. M                  AMB= 2.5 PPM  
 SI GTH= 20. DEGREE S            TEMP= 15.0 DEGREE (C)

II. LI NK VI ABL ES

LINK DESCRIPTI ON	* *	LI NK COOR DI NATES (M)	* *	* *	* *	* *	* *	EF (G/MI )	H (M)	W (M)
		X1	Y1	X2	Y2	TYPE	VPH			
1. Y SB1	*	684	1256	711	1201	AG	761	2.5	0.0	16.7
2. Y SB2	*	711	1201	745	1133	AG	447	5.6	0.0	16.7
3. Y SB3	*	745	1133	800	1025	AG	499	2.5	0.0	16.7
4. Y SB4	*	800	1025	858	906	AG	499	2.5	0.0	16.7
5. Y NB1	*	868	911	813	1024	AG	364	2.5	0.0	16.7
6. Y NB2	*	814	1024	761	1138	AG	353	5.6	0.0	16.7
7. Y NB3	*	761	1138	728	1205	AG	354	2.5	0.0	16.7
8. Y NB4	*	728	1205	700	1263	AG	354	2.5	0.0	16.7
9. Y LT1	*	720	1198	753	1134	AG	314	5.6	0.0	16.7
10. Y LT2	*	753	1134	802	1038	AG	11	5.6	0.0	16.7
11. X EB1	*	596	1103	682	1125	AG	17	2.5	0.0	23.9
12. X EB2	*	682	1125	750	1141	AG	16	5.6	0.0	23.9
13. X EB3	*	750	1141	826	1158	AG	330	2.5	0.0	23.9
14. X EB4	*	826	1158	923	1182	AG	330	2.5	0.0	23.9
15. X WB1	*	924	1170	832	1147	AG	311	2.5	0.0	23.9
16. X WB2	*	832	1147	757	1128	AG	259	5.6	0.0	23.9
17. X WB3	*	757	1128	687	1111	AG	270	2.5	0.0	23.9
18. X WB4	*	687	1111	599	1090	AG	270	2.5	0.0	23.9
19. X LT1	*	674	1115	753	1134	AG	1	5.6	0.0	23.9
20. X LT2	*	753	1134	841	1157	AG	52	5.6	0.0	23.9

III. RECEPTOR LOCATI ONS

RECEPTOR	* *	COOR DI NATES (M)	* *	* *
		X	Y	Z
1. Recpt 1	*	805	1099	1.8
2. Recpt 2	*	691	1157	1.8
3. Recpt 3	*	729	1093	1.8
4. Recpt 4	*	778	1179	1.8

IV. MODEL RESULTS (WORST CASE WIND ANGLE )

4 Miral este-Vi aCol ina. Ist

RECEPTOR	* * * *	BRG (DEG)	* * * *	PRED CONC (PPM)	* * * *	1	2	3	CONC/LI NK (PPM)				8
									4	5	6	7	
1. Recpt 1	*	309.	*	2.8	*	0.0	0.1	0.0	0.0	0.0	0.1	0.0	0.0
2. Recpt 2	*	97.	*	2.7	*	0.0	0.1	0.0	0.0	0.0	0.0	0.0	0.0
3. Recpt 3	*	8.	*	2.7	*	0.0	0.1	0.0	0.0	0.0	0.0	0.0	0.0
4. Recpt 4	*	179.	*	2.7	*	0.0	0.0	0.0	0.0	0.0	0.1	0.0	0.0

RECEPTOR	* * * *	9	10	11	CONC/LI NK (PPM)				
					12	13	14	15	16
1. Recpt 1	*	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
2. Recpt 2	*	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3. Recpt 3	*	0.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4. Recpt 4	*	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

RECEPTOR	* * * *	*CONC/LI NK (PPM)			
		17	18	19	20
1. Recpt 1	*	0.0	0.0	0.0	0.0
2. Recpt 2	*	0.0	0.0	0.0	0.0
3. Recpt 3	*	0.0	0.0	0.0	0.0
4. Recpt 4	*	0.0	0.0	0.0	0.0

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Run Ended on 12/08/2009 at 11:42:31

## **Appendix D-3 Noise Data**



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

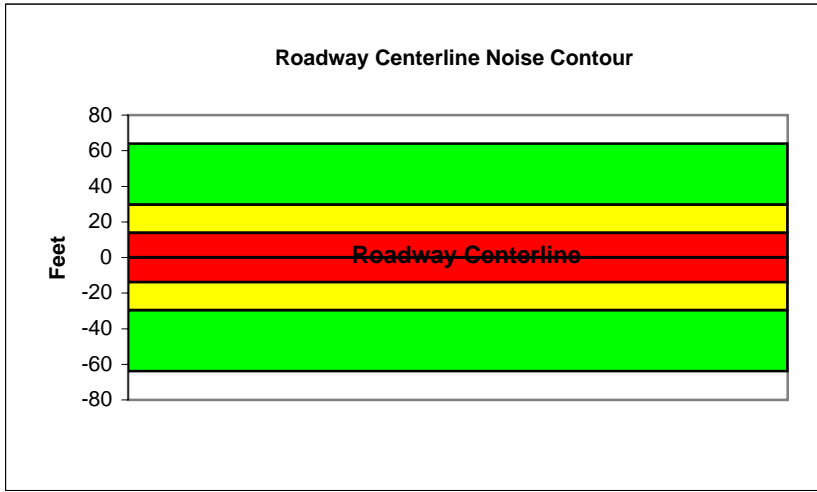
Project Name: Marymount College Facilities Expansion Project Scenario: Existing  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Palos Verdes Drive East  
 Road Segment: Between Via Colinita And Crest Drive

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	7699			
Receiver Barrier Dist:	0	Peak Hour Traffic:	769.9			
Centerline Dist. To Observer:	100	Vehicle Speed:	30			
Barrier Near Lane CL Dist:	0	Centerline Separation:	12.5			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	44.3	53.1	51.3	45.2	53.9	54.5
Medium Trucks:	54.9	46.9	40.5	38.9	47.4	47.6
Heavy Trucks:	60.6	48.6	39.6	40.8	50.9	51.1
<b>Vehicle Noise:</b>	<b>63.1</b>	<b>55.6</b>	<b>52.1</b>	<b>47.7</b>	<b>56.3</b>	<b>56.7</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	64
65 dBA	30
70 dBA	14
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

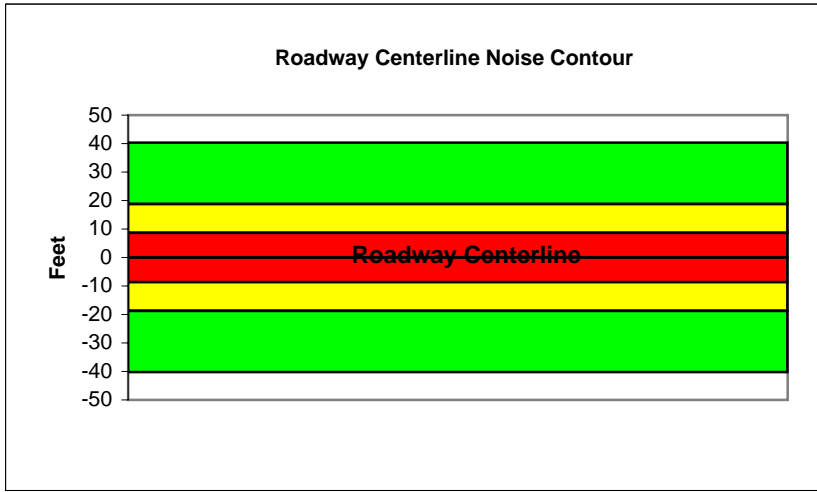
Project Name: Marymount College Facilities Expansion Project Scenario: Existing  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Palos Verdes Drive East  
 Road Segment: Between Crest Drive and Palos Verdes Drive South

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	3873			
Receiver Barrier Dist:	0	Peak Hour Traffic:	387.3			
Centerline Dist. To Observer:	100	Vehicle Speed:	30			
Barrier Near Lane CL Dist:	0	Centerline Separation:	12.5			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	41.4	50.1	48.3	42.3	50.9	51.5
Medium Trucks:	52.0	43.9	37.5	35.9	44.4	44.7
Heavy Trucks:	57.6	45.7	36.6	37.8	48.0	48.1
<b>Vehicle Noise:</b>	<b>60.1</b>	<b>52.6</b>	<b>49.1</b>	<b>44.7</b>	<b>53.3</b>	<b>53.7</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	40
65 dBA	19
70 dBA	9
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

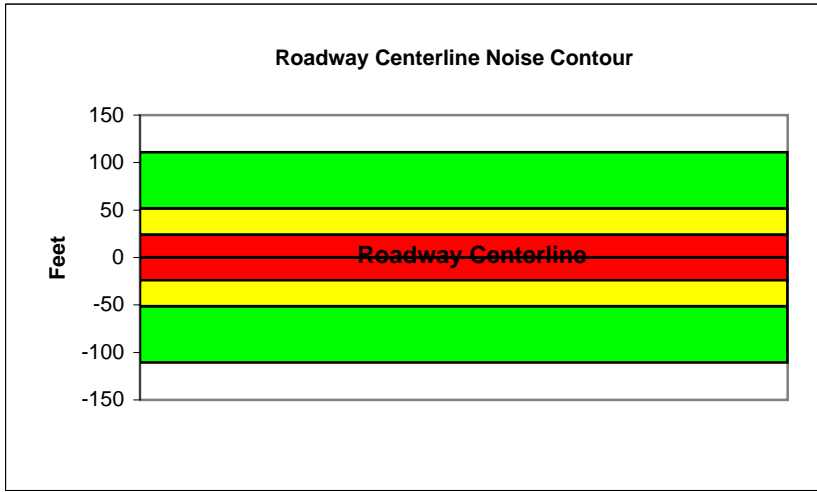
Project Name: Marymount College Facilities Expansion Project Scenario: Existing  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Miraleste Drive  
 Road Segment: Between Palos Verdes Drive East and Via Colinita

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	12635			
Receiver Barrier Dist:	0	Peak Hour Traffic:	1263.5			
Centerline Dist. To Observer:	100	Vehicle Speed:	35			
Barrier Near Lane CL Dist:	0	Centerline Separation:	94			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	46.6	55.4	53.6	47.6	56.2	56.8
Medium Trucks:	56.4	48.3	41.9	40.3	48.8	49.1
Heavy Trucks:	61.6	49.6	40.6	41.8	51.7	51.8
<b>Vehicle Noise:</b>	<b>64.0</b>	<b>57.4</b>	<b>54.2</b>	<b>49.5</b>	<b>58.1</b>	<b>58.5</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	111
65 dBA	52
70 dBA	24
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

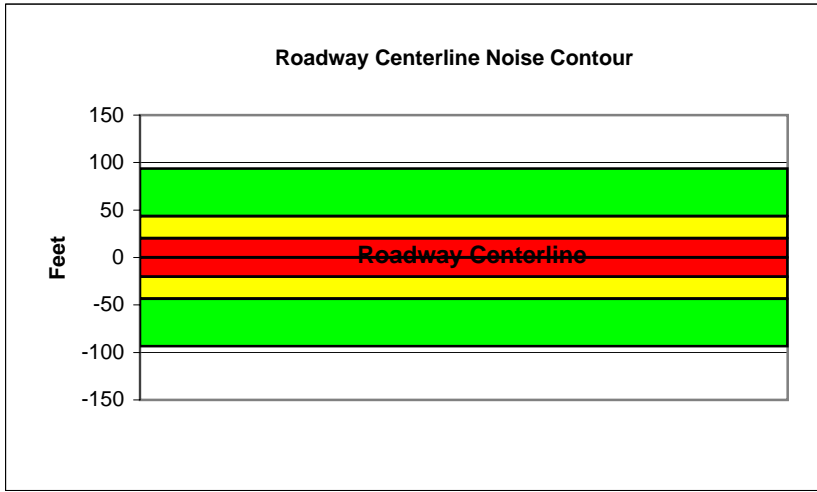
Project Name: Marymount College Facilities Expansion Project Scenario: Existing  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Miraleste Drive  
 Road Segment: Between Via Colinita and First Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	9805			
Receiver Barrier Dist:	0	Peak Hour Traffic:	980.5			
Centerline Dist. To Observer:	100	Vehicle Speed:	35			
Barrier Near Lane CL Dist:	0	Centerline Separation:	94			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	45.5	54.3	52.5	46.4	55.1	55.7
Medium Trucks:	55.3	47.2	40.8	39.2	47.7	48.0
Heavy Trucks:	60.5	48.5	39.5	40.7	50.6	50.7
<b>Vehicle Noise:</b>	<b>62.9</b>	<b>56.3</b>	<b>53.1</b>	<b>48.4</b>	<b>57.0</b>	<b>57.4</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	94
65 dBA	44
70 dBA	20
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

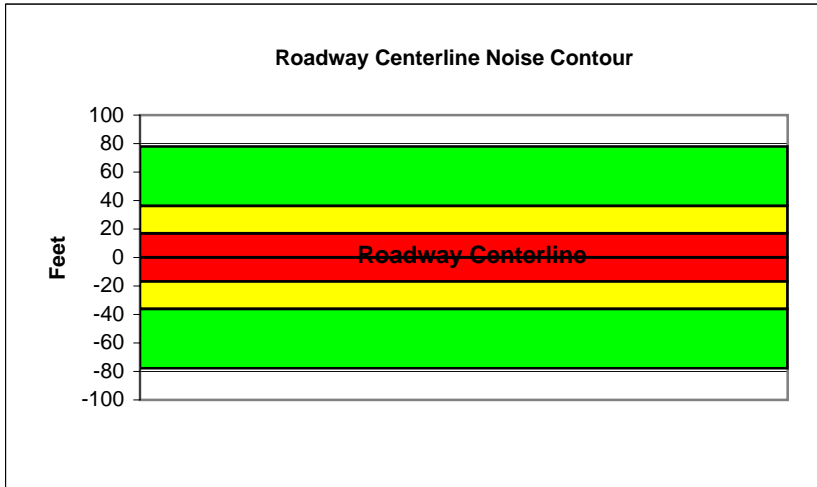
Project Name: Marymount College Facilities Expansion Project Scenario: Existing  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Miraleste Drive  
 Road Segment: Between First Street and Western Avenue

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	7446			
Receiver Barrier Dist:	0	Peak Hour Traffic:	744.6			
Centerline Dist. To Observer:	100	Vehicle Speed:	35			
Barrier Near Lane CL Dist:	0	Centerline Separation:	28			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	45.7	54.5	52.7	46.6	55.3	55.9
Medium Trucks:	55.4	47.4	41.0	39.4	47.9	48.1
Heavy Trucks:	60.6	48.7	39.6	40.9	50.8	50.9
<b>Vehicle Noise:</b>	<b>63.1</b>	<b>56.4</b>	<b>53.3</b>	<b>48.6</b>	<b>57.1</b>	<b>57.6</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	78
65 dBA	36
70 dBA	17
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

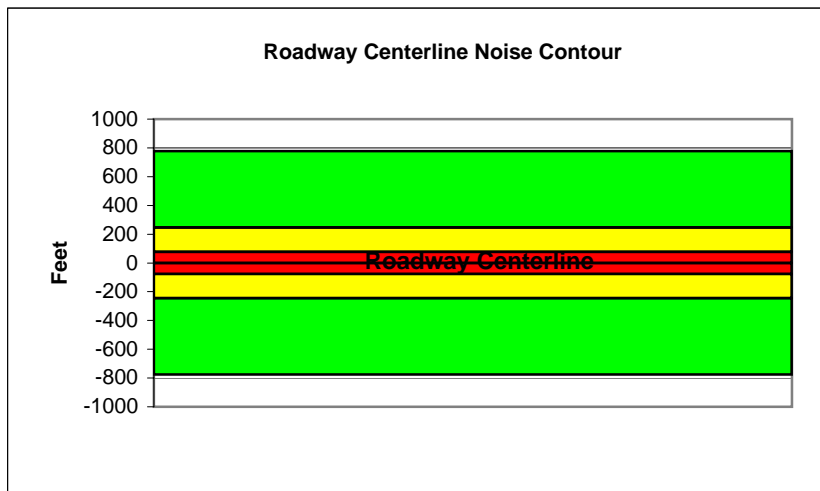
Project Name: Marymount College Facilities Expansion Project      Scenario: Existing  
 Analyst: Michelle Dunn      Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between Toscanini Drive and Capitol Drive

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	34,768			
Receiver Barrier Dist:	0	Peak Hour Traffic:	3476.8			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	48			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions <b>HARD SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	0	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.695	0.129	0.096	0.92
Rt View:      90      Lft View:      -90		Med. Truck	0.0144	0.0006	0.015	0.03
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.24	0.001	0.025	0.05
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	56.7	64.8	63.5	57.4	65.8	66.5
Medium Trucks:	65.7	42.0	34.2	43.4	49.6	49.6
Heavy Trucks:	70.5	61.3	43.5	52.7	62.7	62.7
<b>Vehicle Noise:</b>	<b>72.9</b>	<b>66.9</b>	<b>63.5</b>	<b>59.1</b>	<b>67.6</b>	<b>68.1</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	778
65 dBA	246
70 dBA	78
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

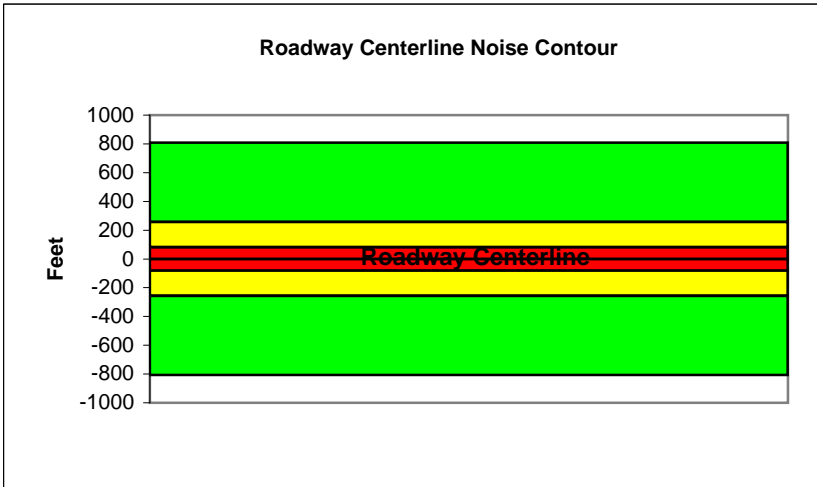
Project Name: Marymount College Facilities Expansion Project      Scenario: Existing  
 Analyst: Michelle Dunn      Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between Capitol Drive and Crestwood Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	36,211			
Receiver Barrier Dist:	0	Peak Hour Traffic:	3621.1			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	48			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions <b>HARD SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	0	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.695	0.129	0.096	0.92
Rt View:      90      Lft View:      -90		Med. Truck	0.0144	0.0006	0.015	0.03
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.24	0.001	0.025	0.05
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	56.9	64.9	63.6	57.6	66.0	66.6
Medium Trucks:	65.8	42.2	34.4	43.6	49.7	49.8
Heavy Trucks:	70.7	61.5	43.7	52.9	62.9	62.9
<b>Vehicle Noise:</b>	<b>73.1</b>	<b>67.0</b>	<b>63.7</b>	<b>59.3</b>	<b>67.8</b>	<b>68.2</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	809
65 dBA	256
70 dBA	81
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

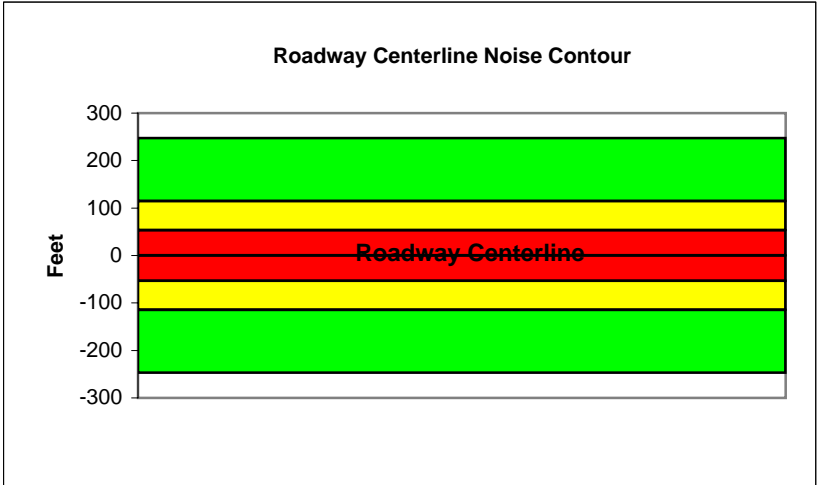
Project Name: Marymount College Facilities Expansion Project Scenario: Existing  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between Crestwood Street and First Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	30913			
Receiver Barrier Dist:	0	Peak Hour Traffic:	3091.3			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	48			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	53.1	61.9	60.1	54.0	62.6	63.2
Medium Trucks:	62.0	54.0	47.6	46.0	54.5	54.7
Heavy Trucks:	66.9	54.9	45.9	47.1	56.8	56.9
<b>Vehicle Noise:</b>	<b>69.3</b>	<b>63.4</b>	<b>60.5</b>	<b>55.6</b>	<b>64.1</b>	<b>64.6</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	247
65 dBA	115
70 dBA	53
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

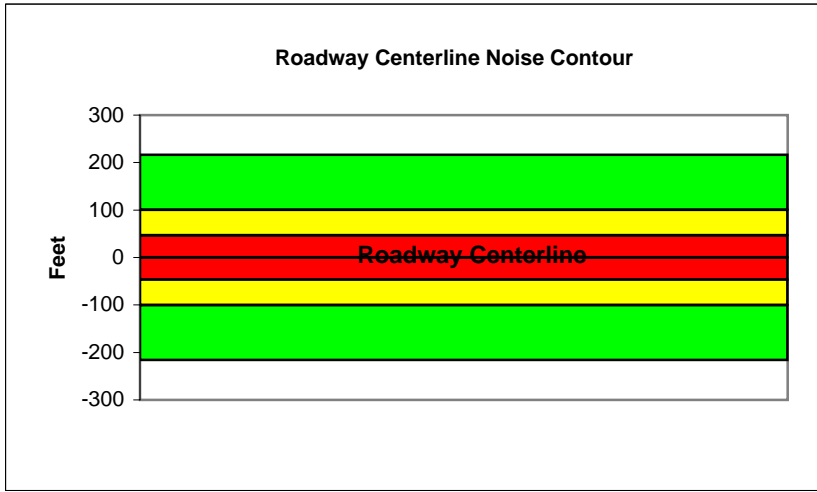
Project Name: Marymount College Facilities Expansion Project Scenario: Existing  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between First Street and Ninth Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	25362			
Receiver Barrier Dist:	0	Peak Hour Traffic:	2536.2			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	48			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	52.2	61.0	59.2	53.1	61.8	62.4
Medium Trucks:	61.2	53.1	46.7	45.1	53.6	53.9
Heavy Trucks:	66.0	54.1	45.0	46.2	56.0	56.1
<b>Vehicle Noise:</b>	<b>68.4</b>	<b>62.6</b>	<b>59.7</b>	<b>54.7</b>	<b>63.3</b>	<b>63.8</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	217
65 dBA	101
70 dBA	47
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

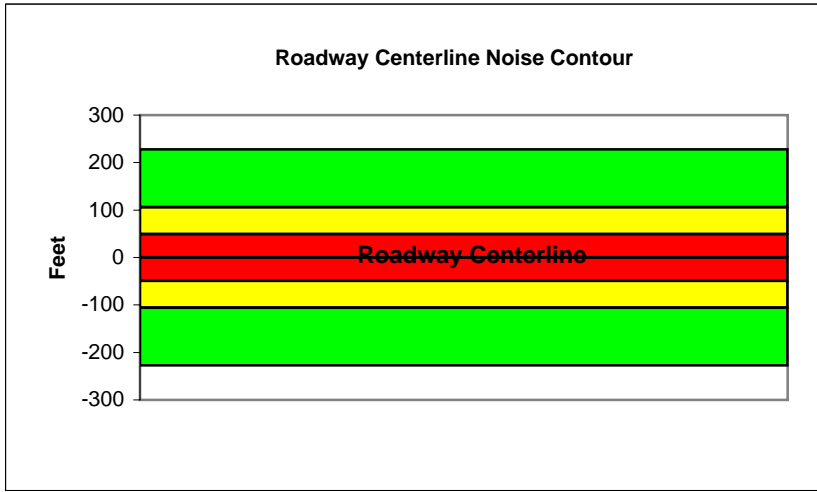
Project Name: Marymount College Facilities Expansion Project Scenario: Existing  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between Ninth Street and West 25th Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	27331			
Receiver Barrier Dist:	0	Peak Hour Traffic:	2733.1			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	43			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	52.7	61.4	59.6	53.6	62.2	62.8
Medium Trucks:	61.6	53.5	47.2	45.6	54.1	54.3
Heavy Trucks:	66.5	54.5	45.5	46.7	56.4	56.5
<b>Vehicle Noise:</b>	<b>68.8</b>	<b>63.0</b>	<b>60.1</b>	<b>55.1</b>	<b>63.7</b>	<b>64.2</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	228
65 dBA	106
70 dBA	49
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

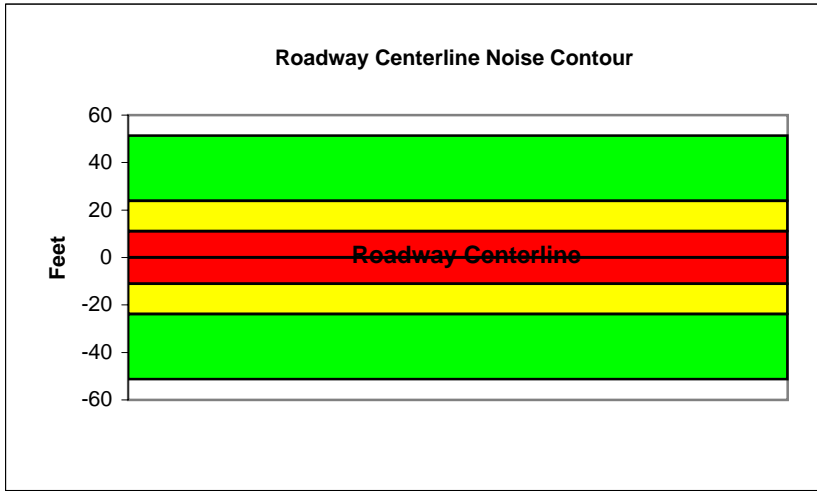
Project Name: Marymount College Facilities Expansion Project Scenario: Existing  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: First Street  
 Road Segment: Between Miraleste Drive and Western Avenue

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	5547			
Receiver Barrier Dist:	0	Peak Hour Traffic:	554.7			
Centerline Dist. To Observer:	100	Vehicle Speed:	30			
Barrier Near Lane CL Dist:	0	Centerline Separation:	14			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	42.9	51.6	49.9	43.8	52.4	53.0
Medium Trucks:	53.5	45.4	39.0	37.4	45.9	46.2
Heavy Trucks:	59.1	47.2	38.1	39.3	49.5	49.6
<b>Vehicle Noise:</b>	<b>61.6</b>	<b>54.1</b>	<b>50.6</b>	<b>46.3</b>	<b>54.8</b>	<b>55.2</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	51
65 dBA	24
70 dBA	11
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

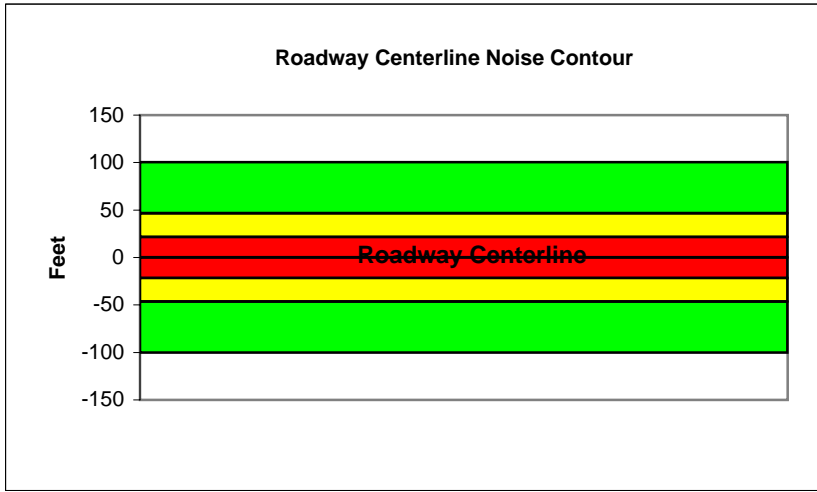
Project Name: Marymount College Facilities Expansion Project Scenario: Existing  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Palos Verdes Drive South  
 Road Segment: Between Palos Verdes Drive East and Western Avenue

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	10895			
Receiver Barrier Dist:	0	Peak Hour Traffic:	1089.5			
Centerline Dist. To Observer:	100	Vehicle Speed:	35			
Barrier Near Lane CL Dist:	0	Centerline Separation:	32			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	47.3	56.0	54.2	48.2	56.8	57.4
Medium Trucks:	57.0	48.9	42.5	40.9	49.4	49.7
Heavy Trucks:	62.2	50.3	41.2	42.4	52.3	52.4
<b>Vehicle Noise:</b>	<b>64.6</b>	<b>58.0</b>	<b>54.8</b>	<b>50.1</b>	<b>58.7</b>	<b>59.1</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	101
65 dBA	47
70 dBA	22
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

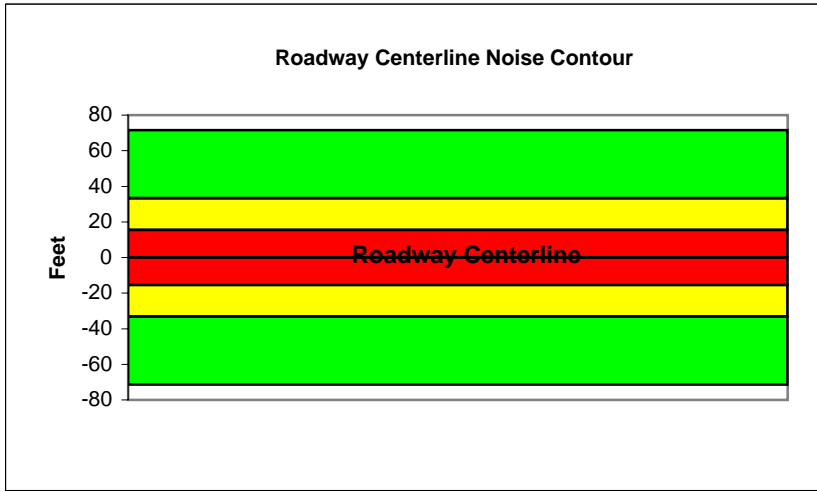
Project Name: Marymount College Facilities Expansion Project Scenario: Existing Plus Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Palos Verdes Drive East  
 Road Segment: Between Via Colinita And Crest Drive

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	9147			
Receiver Barrier Dist:	0	Peak Hour Traffic:	914.7			
Centerline Dist. To Observer:	100	Vehicle Speed:	30			
Barrier Near Lane CL Dist:	0	Centerline Separation:	12.5			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	45.1	53.9	52.1	46.0	54.6	55.3
Medium Trucks:	55.7	47.6	41.2	39.7	48.2	48.4
Heavy Trucks:	61.3	49.4	40.3	41.6	51.7	51.8
<b>Vehicle Noise:</b>	<b>63.9</b>	<b>56.3</b>	<b>52.8</b>	<b>48.5</b>	<b>57.0</b>	<b>57.4</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	72
65 dBA	33
70 dBA	15
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

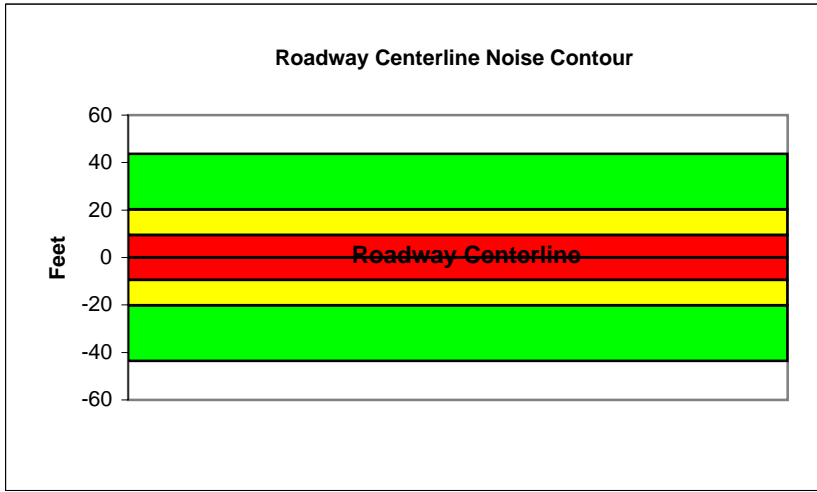
Project Name: Marymount College Facilities Expansion Project Scenario: Existing Plus Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Palos Verdes Drive East  
 Road Segment: Between Crest Drive and Palos Verdes Drive South

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	4356			
Receiver Barrier Dist:	0	Peak Hour Traffic:	435.6			
Centerline Dist. To Observer:	100	Vehicle Speed:	30			
Barrier Near Lane CL Dist:	0	Centerline Separation:	12.5			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	41.9	50.6	48.9	42.8	51.4	52.0
Medium Trucks:	52.5	44.4	38.0	36.4	44.9	45.2
Heavy Trucks:	58.1	46.2	37.1	38.3	48.5	48.6
<b>Vehicle Noise:</b>	<b>60.6</b>	<b>53.1</b>	<b>49.6</b>	<b>45.3</b>	<b>53.8</b>	<b>54.2</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	44
65 dBA	20
70 dBA	9
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

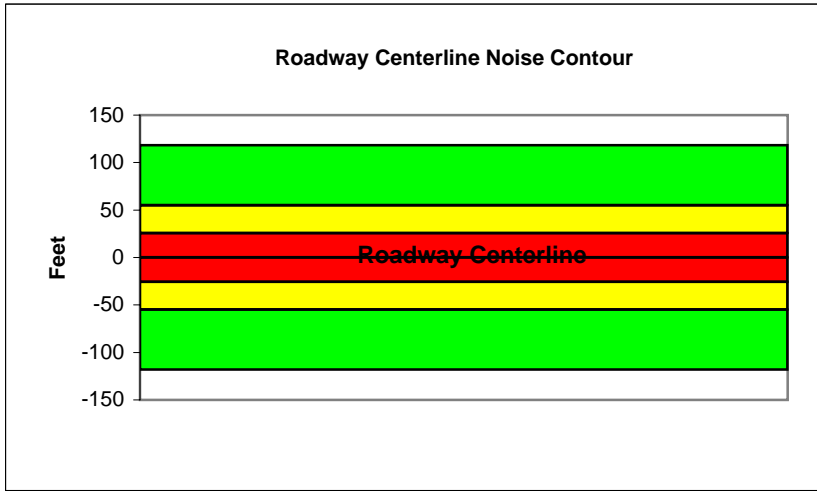
Project Name: Marymount College Facilities Expansion Project Scenario: Existing Plus Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Miraleste Drive  
 Road Segment: Between Palos Verdes Drive East and Via Colinita

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	13890			
Receiver Barrier Dist:	0	Peak Hour Traffic:	1389			
Centerline Dist. To Observer:	100	Vehicle Speed:	35			
Barrier Near Lane CL Dist:	0	Centerline Separation:	94			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	47.1	55.8	54.0	48.0	56.6	57.2
Medium Trucks:	56.8	48.7	42.3	40.7	49.2	49.5
Heavy Trucks:	62.0	50.1	41.0	42.2	52.1	52.2
<b>Vehicle Noise:</b>	<b>64.4</b>	<b>57.8</b>	<b>54.6</b>	<b>49.9</b>	<b>58.5</b>	<b>58.9</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	118
65 dBA	55
70 dBA	25
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

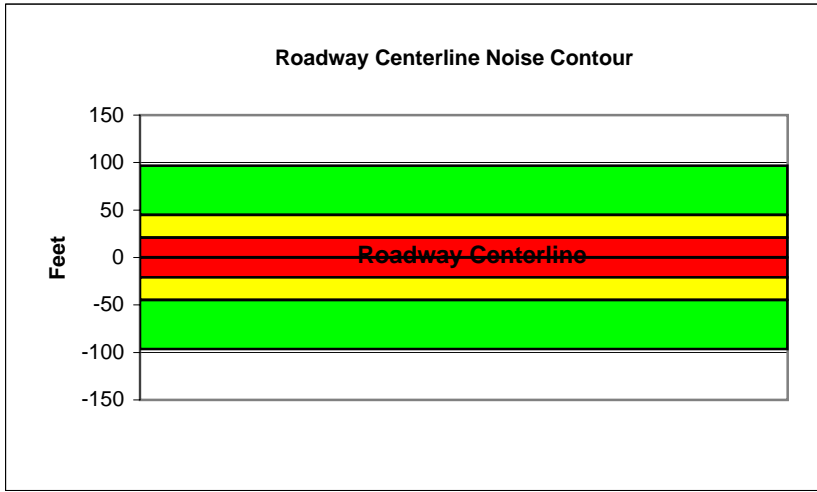
Project Name: Marymount College Facilities Expansion Project Scenario: Existing Plus Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Miraleste Drive  
 Road Segment: Between Via Colinita and First Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	10288			
Receiver Barrier Dist:	0	Peak Hour Traffic:	1028.8			
Centerline Dist. To Observer:	100	Vehicle Speed:	35			
Barrier Near Lane CL Dist:	0	Centerline Separation:	94			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	45.7	54.5	52.7	46.7	55.3	55.9
Medium Trucks:	55.5	47.4	41.0	39.4	47.9	48.2
Heavy Trucks:	60.7	48.7	39.7	40.9	50.8	50.9
<b>Vehicle Noise:</b>	<b>63.1</b>	<b>56.5</b>	<b>53.3</b>	<b>48.6</b>	<b>57.2</b>	<b>57.6</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	97
65 dBA	45
70 dBA	21
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

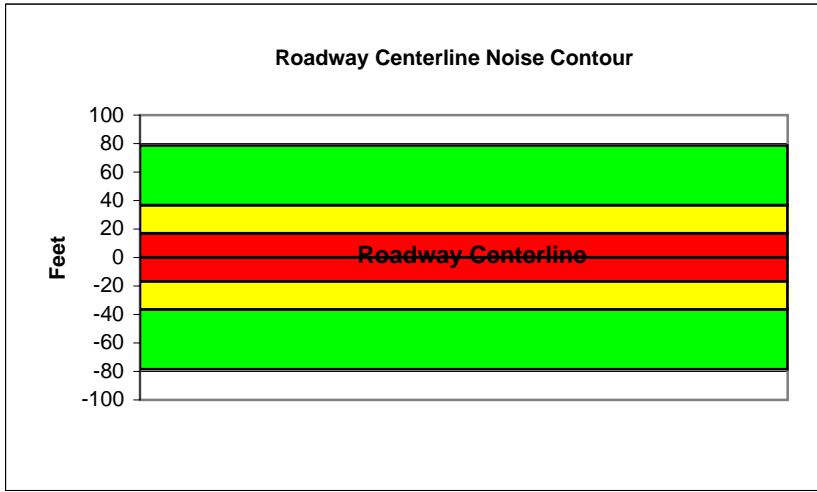
Project Name: Marymount College Facilities Expansion Project Scenario: Existing Plus Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Miraleste Drive  
 Road Segment: Between First Street and Western Avenue

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	7543			
Receiver Barrier Dist:	0	Peak Hour Traffic:	754.3			
Centerline Dist. To Observer:	100	Vehicle Speed:	35			
Barrier Near Lane CL Dist:	0	Centerline Separation:	28			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	45.8	54.5	52.7	46.7	55.3	55.9
Medium Trucks:	55.5	47.4	41.0	39.5	47.9	48.2
Heavy Trucks:	60.7	48.8	39.7	40.9	50.8	51.0
<b>Vehicle Noise:</b>	<b>63.1</b>	<b>56.5</b>	<b>53.3</b>	<b>48.6</b>	<b>57.2</b>	<b>57.6</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	79
65 dBA	37
70 dBA	17
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

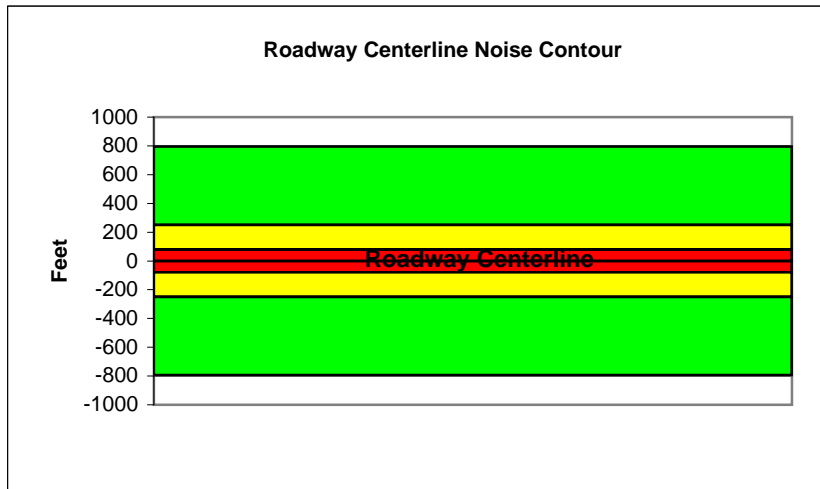
Project Name: Marymount College Facilities Expansion Project      Scenario: Existing Plus Project  
 Analyst: Michelle Dunn      Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between Toscanini Drive and Capitol Drive

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	35,540			
Receiver Barrier Dist:	0	Peak Hour Traffic:	3554			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	48			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions <b>HARD SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	0	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.695	0.129	0.096	0.92
Rt View:      90      Lft View:      -90		Med. Truck	0.0144	0.0006	0.015	0.03
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.24	0.001	0.025	0.05
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	56.8	64.9	63.6	57.5	65.9	66.6
Medium Trucks:	65.8	42.1	34.3	43.5	49.7	49.7
Heavy Trucks:	70.6	61.4	43.6	52.8	62.8	62.8
<b>Vehicle Noise:</b>	<b>73.0</b>	<b>67.0</b>	<b>63.6</b>	<b>59.2</b>	<b>67.7</b>	<b>68.2</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	795
65 dBA	251
70 dBA	79
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

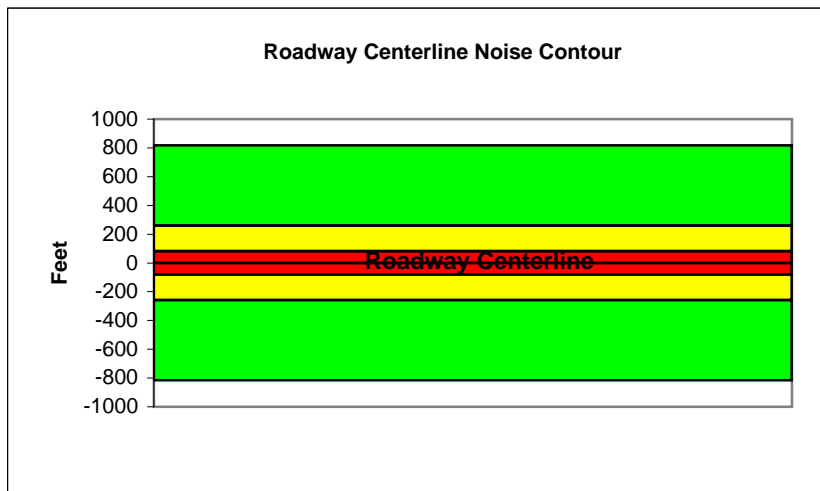
Project Name: Marymount College Facilities Expansion Project      Scenario: Existing Plus Project  
 Analyst: Michelle Dunn      Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between Capitol Drive and Crestwood Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	36,501			
Receiver Barrier Dist:	0	Peak Hour Traffic:	3650.1			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	48			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions <b>HARD SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	0	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.695	0.129	0.096	0.92
Rt View:      90      Lft View:      -90		Med. Truck	0.0144	0.0006	0.015	0.03
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.24	0.001	0.025	0.05
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	56.9	65.0	63.7	57.6	66.0	66.7
Medium Trucks:	65.9	42.2	34.4	43.6	49.8	49.8
Heavy Trucks:	70.7	61.5	43.7	52.9	62.9	62.9
<b>Vehicle Noise:</b>	<b>73.1</b>	<b>67.1</b>	<b>63.7</b>	<b>59.4</b>	<b>67.8</b>	<b>68.3</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	817
65 dBA	258
70 dBA	82
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

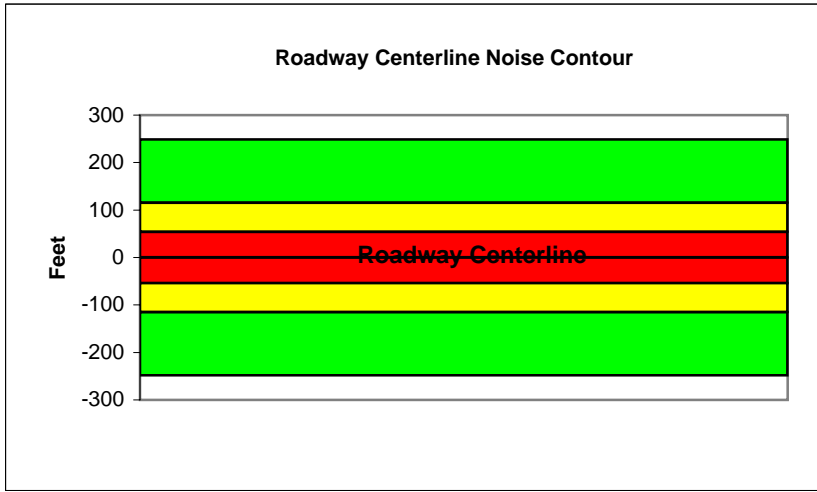
Project Name: Marymount College Facilities Expansion Project Scenario: Existing Plus Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between Crestwood Street and First Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	31203			
Receiver Barrier Dist:	0	Peak Hour Traffic:	3120.3			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	48			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	53.1	61.9	60.1	54.0	62.7	63.3
Medium Trucks:	62.1	54.0	47.6	46.0	54.5	54.8
Heavy Trucks:	66.9	55.0	45.9	47.1	56.9	57.0
<b>Vehicle Noise:</b>	<b>69.3</b>	<b>63.5</b>	<b>60.6</b>	<b>55.6</b>	<b>64.2</b>	<b>64.7</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	249
65 dBA	115
70 dBA	54
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

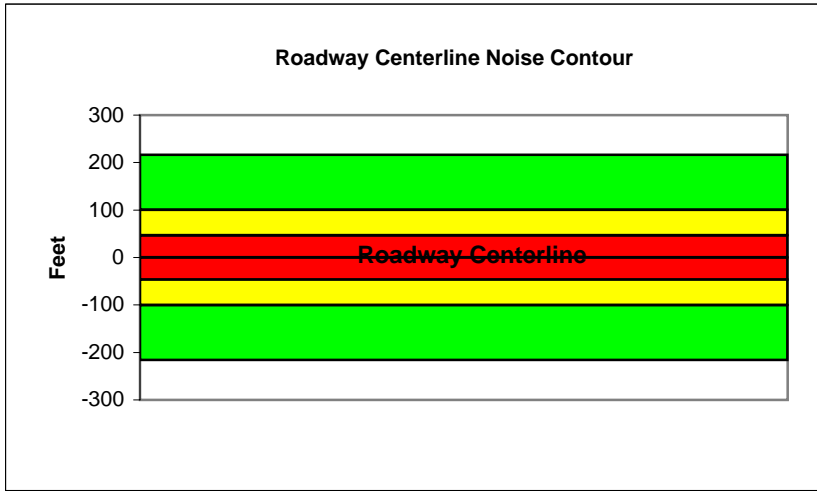
Project Name: Marymount College Facilities Expansion Project Scenario: Existing Plus Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between First Street and Ninth Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	25362			
Receiver Barrier Dist:	0	Peak Hour Traffic:	2536.2			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	48			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	52.2	61.0	59.2	53.1	61.8	62.4
Medium Trucks:	61.2	53.1	46.7	45.1	53.6	53.9
Heavy Trucks:	66.0	54.1	45.0	46.2	56.0	56.1
<b>Vehicle Noise:</b>	<b>68.4</b>	<b>62.6</b>	<b>59.7</b>	<b>54.7</b>	<b>63.3</b>	<b>63.8</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	217
65 dBA	101
70 dBA	47
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

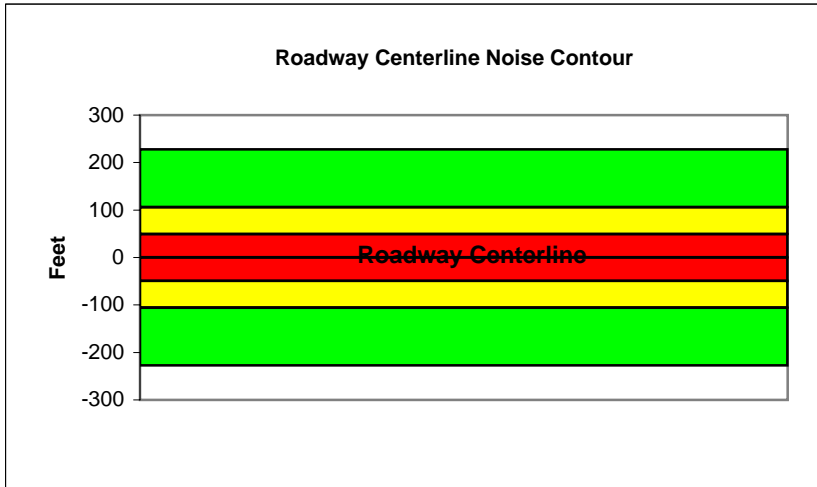
Project Name: Marymount College Facilities Expansion Project Scenario: Existing Plus Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between Ninth Street and West 25th Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	27331			
Receiver Barrier Dist:	0	Peak Hour Traffic:	2733.1			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	43			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	52.7	61.4	59.6	53.6	62.2	62.8
Medium Trucks:	61.6	53.5	47.2	45.6	54.1	54.3
Heavy Trucks:	66.5	54.5	45.5	46.7	56.4	56.5
<b>Vehicle Noise:</b>	<b>68.8</b>	<b>63.0</b>	<b>60.1</b>	<b>55.1</b>	<b>63.7</b>	<b>64.2</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	228
65 dBA	106
70 dBA	49
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

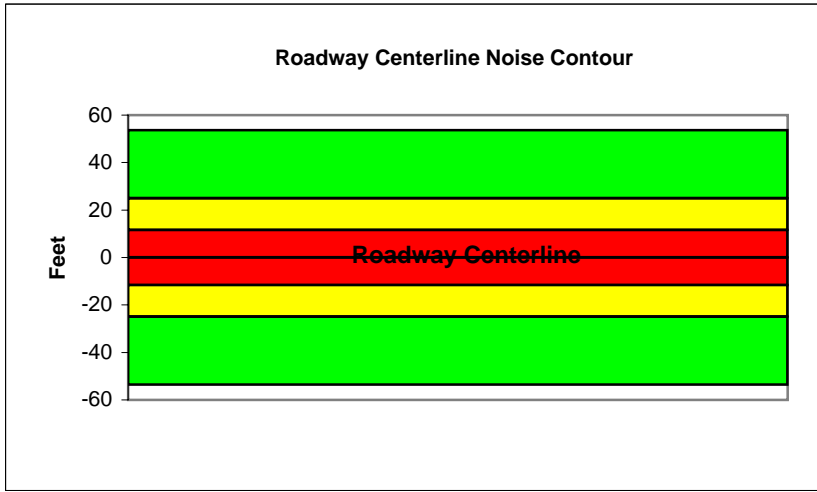
Project Name: Marymount College Facilities Expansion Project Scenario: Existing Plus Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: First Street  
 Road Segment: Between Miraleste Drive and Western Avenue

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	5933			
Receiver Barrier Dist:	0	Peak Hour Traffic:	593.3			
Centerline Dist. To Observer:	100	Vehicle Speed:	30			
Barrier Near Lane CL Dist:	0	Centerline Separation:	14			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	43.2	51.9	50.2	44.1	52.7	53.3
Medium Trucks:	53.8	45.7	39.3	37.7	46.2	46.5
Heavy Trucks:	59.4	47.5	38.4	39.6	49.8	49.9
<b>Vehicle Noise:</b>	<b>61.9</b>	<b>54.4</b>	<b>50.9</b>	<b>46.6</b>	<b>55.1</b>	<b>55.5</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	54
65 dBA	25
70 dBA	12
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

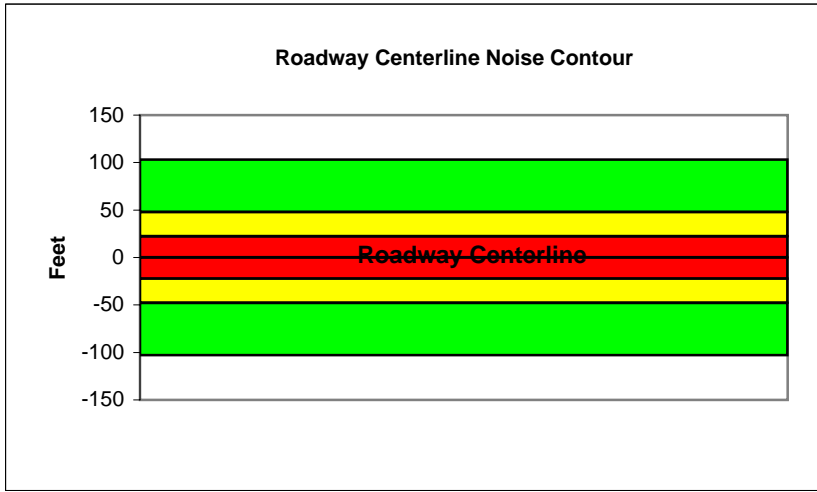
Project Name: Marymount College Facilities Expansion Project Scenario: Existing Plus Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Palos Verdes Drive South  
 Road Segment: Between Palos Verdes Drive East and Western Avenue

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	11281			
Receiver Barrier Dist:	0	Peak Hour Traffic:	1128.1			
Centerline Dist. To Observer:	100	Vehicle Speed:	35			
Barrier Near Lane CL Dist:	0	Centerline Separation:	32			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	47.4	56.2	54.4	48.3	57.0	57.6
Medium Trucks:	57.1	49.1	42.7	41.1	49.6	49.8
Heavy Trucks:	62.3	50.4	41.3	42.6	52.5	52.6
<b>Vehicle Noise:</b>	<b>64.8</b>	<b>58.1</b>	<b>55.0</b>	<b>50.3</b>	<b>58.8</b>	<b>59.3</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	103
65 dBA	48
70 dBA	22
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

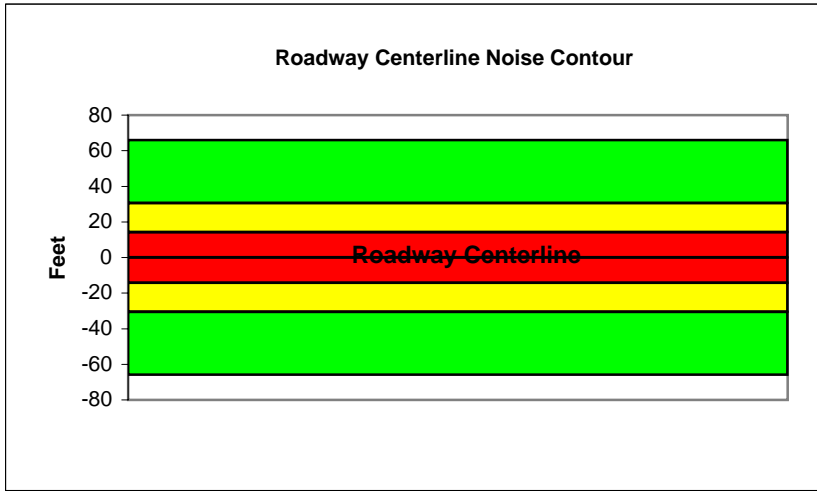
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 Without Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Palos Verdes Drive East  
 Road Segment: Between Via Colinita And Crest Drive

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	8063			
Receiver Barrier Dist:	0	Peak Hour Traffic:	806.3			
Centerline Dist. To Observer:	100	Vehicle Speed:	30			
Barrier Near Lane CL Dist:	0	Centerline Separation:	12.5			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	44.5	53.3	51.5	45.4	54.1	54.7
Medium Trucks:	55.1	47.1	40.7	39.1	47.6	47.8
Heavy Trucks:	60.8	48.8	39.8	41.0	51.1	51.3
<b>Vehicle Noise:</b>	<b>63.3</b>	<b>55.8</b>	<b>52.3</b>	<b>47.9</b>	<b>56.5</b>	<b>56.9</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	66
65 dBA	31
70 dBA	14
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

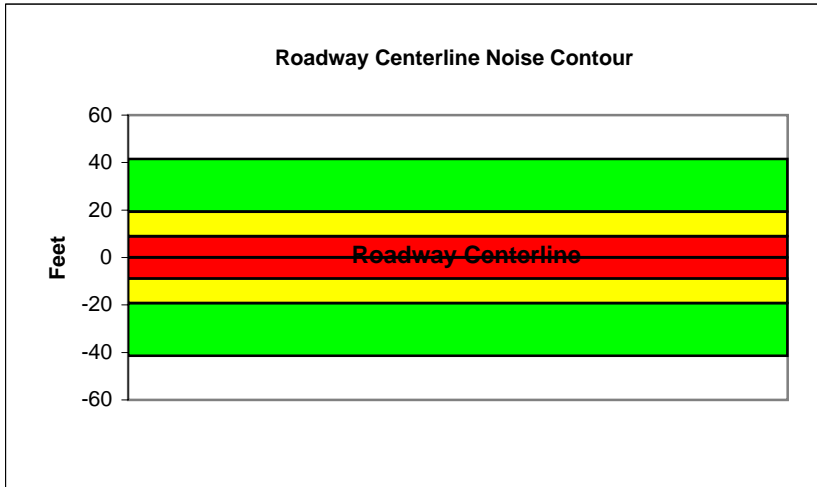
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 Without Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Palos Verdes Drive East  
 Road Segment: Between Crest Drive and Palos Verdes Drive South

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	4041			
Receiver Barrier Dist:	0	Peak Hour Traffic:	404.1			
Centerline Dist. To Observer:	100	Vehicle Speed:	30			
Barrier Near Lane CL Dist:	0	Centerline Separation:	12.5			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	41.5	50.3	48.5	42.4	51.1	51.7
Medium Trucks:	52.1	44.1	37.7	36.1	44.6	44.8
Heavy Trucks:	57.8	45.8	36.8	38.0	48.1	48.3
<b>Vehicle Noise:</b>	<b>60.3</b>	<b>52.8</b>	<b>49.3</b>	<b>44.9</b>	<b>53.5</b>	<b>53.9</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	42
65 dBA	19
70 dBA	9
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

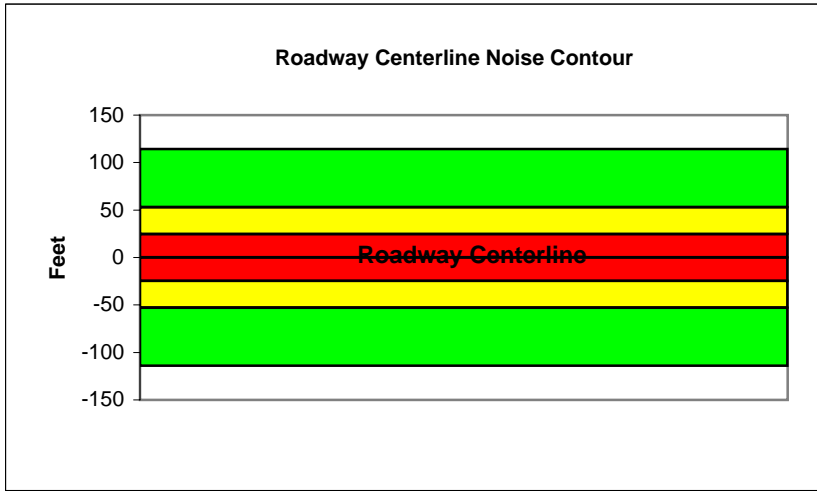
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 Without Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Miraleste Drive  
 Road Segment: Between Palos Verdes Drive East and Via Colinita

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	13195			
Receiver Barrier Dist:	0	Peak Hour Traffic:	1319.5			
Centerline Dist. To Observer:	100	Vehicle Speed:	35			
Barrier Near Lane CL Dist:	0	Centerline Separation:	94			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	46.8	55.6	53.8	47.7	56.4	57.0
Medium Trucks:	56.5	48.5	42.1	40.5	49.0	49.2
Heavy Trucks:	61.8	49.8	40.8	42.0	51.9	52.0
<b>Vehicle Noise:</b>	<b>64.2</b>	<b>57.6</b>	<b>54.4</b>	<b>49.7</b>	<b>58.3</b>	<b>58.7</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	114
65 dBA	53
70 dBA	25
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

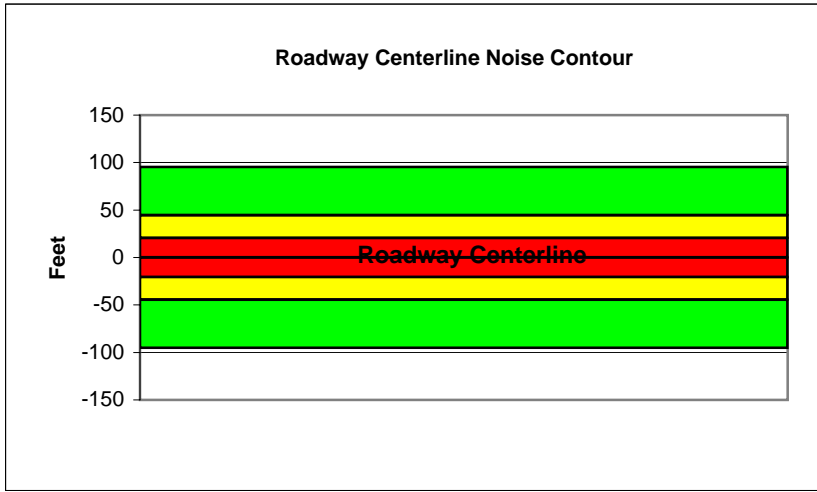
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 Without Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Miraleste Drive  
 Road Segment: Between Via Colinita and First Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	10085			
Receiver Barrier Dist:	0	Peak Hour Traffic:	1008.5			
Centerline Dist. To Observer:	100	Vehicle Speed:	35			
Barrier Near Lane CL Dist:	0	Centerline Separation:	94			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	45.7	54.4	52.7	46.6	55.2	55.8
Medium Trucks:	55.4	47.3	40.9	39.4	47.8	48.1
Heavy Trucks:	60.6	48.7	39.6	40.8	50.7	50.9
<b>Vehicle Noise:</b>	<b>63.0</b>	<b>56.4</b>	<b>53.2</b>	<b>48.5</b>	<b>57.1</b>	<b>57.5</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	95
65 dBA	44
70 dBA	21
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

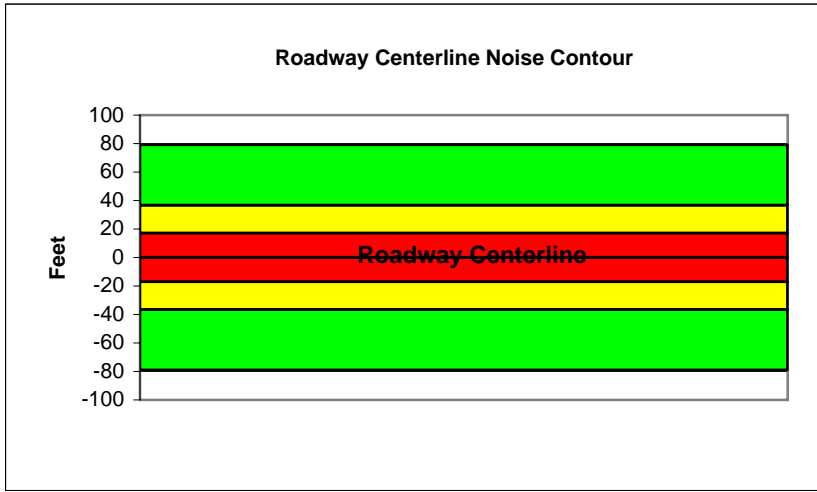
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 Without Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Miraleste Drive  
 Road Segment: Between First Street and Western Avenue

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	7586			
Receiver Barrier Dist:	0	Peak Hour Traffic:	758.6			
Centerline Dist. To Observer:	100	Vehicle Speed:	35			
Barrier Near Lane CL Dist:	0	Centerline Separation:	28			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	45.8	54.6	52.8	46.7	55.3	55.9
Medium Trucks:	55.5	47.4	41.0	39.5	48.0	48.2
Heavy Trucks:	60.7	48.8	39.7	40.9	50.8	51.0
<b>Vehicle Noise:</b>	<b>63.2</b>	<b>56.5</b>	<b>53.3</b>	<b>48.6</b>	<b>57.2</b>	<b>57.7</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	79
65 dBA	37
70 dBA	17
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

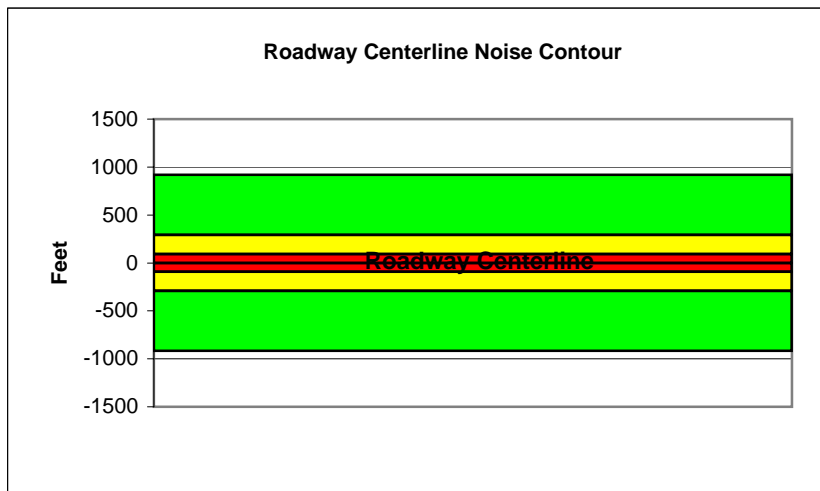
Project Name: Marymount College Facilities Expansion Project      Scenario: 2012 Without Project  
 Analyst: Michelle Dunn      Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between Toscanini Drive and Capitol Drive

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	41,170			
Receiver Barrier Dist:	0	Peak Hour Traffic:	4117			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	48			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions <b>HARD SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	0	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.695	0.129	0.096	0.92
Rt View:      90      Lft View:      -90		Med. Truck	0.0144	0.0006	0.015	0.03
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.24	0.001	0.025	0.05
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	57.4	65.5	64.2	58.1	66.6	67.2
Medium Trucks:	66.4	42.8	35.0	44.1	50.3	50.3
Heavy Trucks:	71.2	62.0	44.2	53.4	63.5	63.5
<b>Vehicle Noise:</b>	<b>73.6</b>	<b>67.6</b>	<b>64.3</b>	<b>59.9</b>	<b>68.4</b>	<b>68.8</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	921
65 dBA	291
70 dBA	92
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

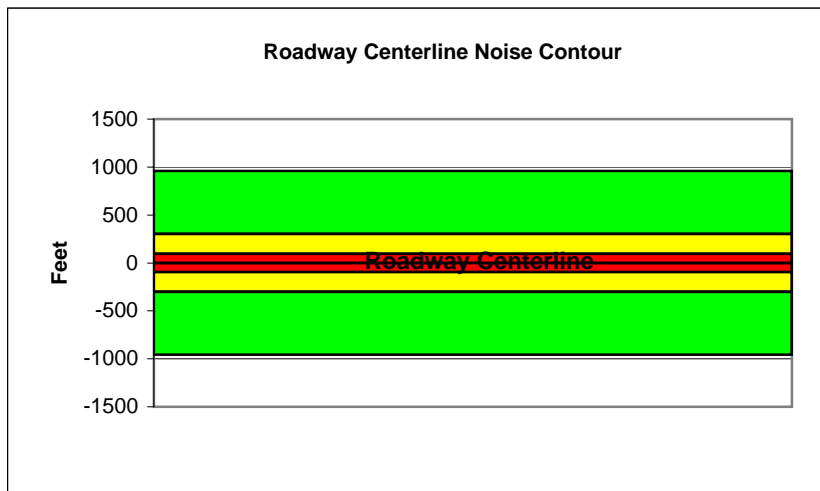
Project Name: Marymount College Facilities Expansion Project      Scenario: 2012 Without Project  
 Analyst: Michelle Dunn      Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between Capitol Drive and Crestwood Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	42,941			
Receiver Barrier Dist:	0	Peak Hour Traffic:	4294.1			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	48			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions <b>HARD SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	0	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.695	0.129	0.096	0.92
Rt View:      90      Lft View:      -90		Med. Truck	0.0144	0.0006	0.015	0.03
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.24	0.001	0.025	0.05
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	57.6	65.7	64.4	58.3	66.8	67.4
Medium Trucks:	66.6	42.9	35.1	44.3	50.5	50.5
Heavy Trucks:	71.4	62.2	44.4	53.6	63.6	63.7
<b>Vehicle Noise:</b>	<b>73.8</b>	<b>67.8</b>	<b>64.4</b>	<b>60.1</b>	<b>68.5</b>	<b>69.0</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	960
65 dBA	303
70 dBA	96
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

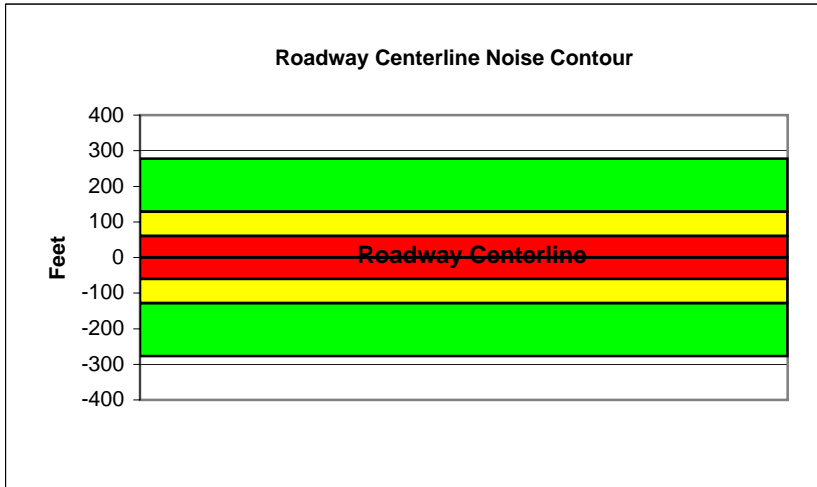
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 Without Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between Crestwood Street and First Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	36763			
Receiver Barrier Dist:	0	Peak Hour Traffic:	3676.3			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	48			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	53.8	62.6	60.8	54.7	63.4	64.0
Medium Trucks:	62.8	54.7	48.3	46.8	55.2	55.5
Heavy Trucks:	67.6	55.7	46.6	47.9	57.6	57.7
<b>Vehicle Noise:</b>	<b>70.0</b>	<b>64.2</b>	<b>61.3</b>	<b>56.3</b>	<b>64.9</b>	<b>65.4</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	278
65 dBA	129
70 dBA	60
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

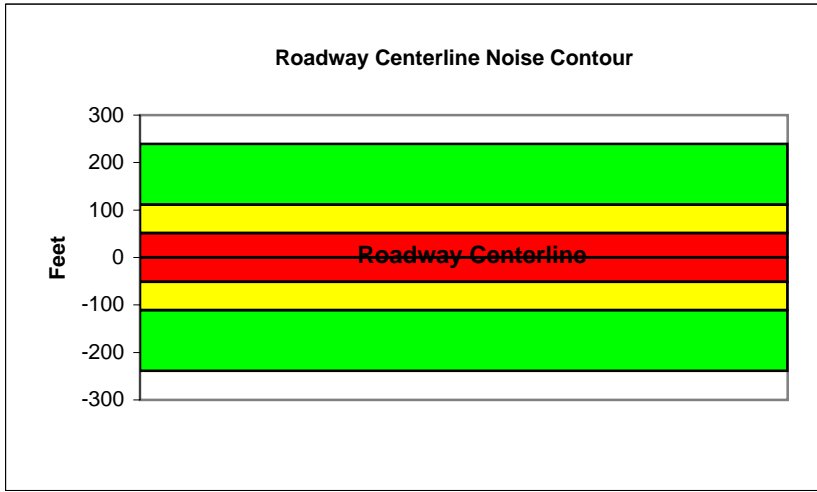
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 Without Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between First Street and Ninth Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	29360			
Receiver Barrier Dist:	0	Peak Hour Traffic:	2936			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	48			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	52.9	61.6	59.8	53.8	62.4	63.0
Medium Trucks:	61.8	53.7	47.4	45.8	54.3	54.5
Heavy Trucks:	66.7	54.7	45.7	46.9	56.6	56.7
<b>Vehicle Noise:</b>	<b>69.0</b>	<b>63.2</b>	<b>60.3</b>	<b>55.3</b>	<b>63.9</b>	<b>64.4</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	239
65 dBA	111
70 dBA	52
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

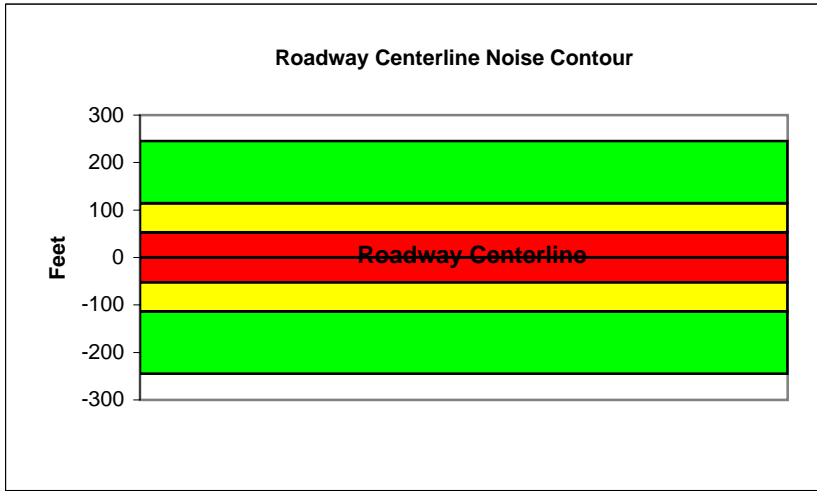
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 Without Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between Ninth Street and West 25th Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	30488			
Receiver Barrier Dist:	0	Peak Hour Traffic:	3048.8			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	43			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	53.1	61.9	60.1	54.0	62.7	63.3
Medium Trucks:	62.1	54.0	47.6	46.1	54.5	54.8
Heavy Trucks:	66.9	55.0	45.9	47.2	56.9	57.0
<b>Vehicle Noise:</b>	<b>69.3</b>	<b>63.5</b>	<b>60.6</b>	<b>55.6</b>	<b>64.2</b>	<b>64.7</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	245
65 dBA	114
70 dBA	53
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

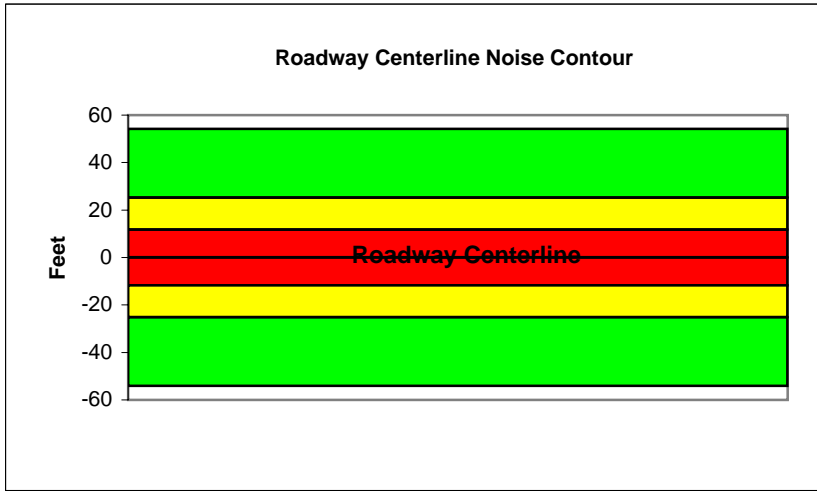
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 Without Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: First Street  
 Road Segment: Between Miraleste Drive and Western Avenue

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	6021			
Receiver Barrier Dist:	0	Peak Hour Traffic:	602.1			
Centerline Dist. To Observer:	100	Vehicle Speed:	30			
Barrier Near Lane CL Dist:	0	Centerline Separation:	14			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	43.2	52.0	50.2	44.1	52.8	53.4
Medium Trucks:	53.8	45.8	39.4	37.8	46.3	46.5
Heavy Trucks:	59.5	47.5	38.5	39.7	49.8	50.0
<b>Vehicle Noise:</b>	<b>62.0</b>	<b>54.5</b>	<b>50.9</b>	<b>46.6</b>	<b>55.2</b>	<b>55.6</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	54
65 dBA	25
70 dBA	12
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

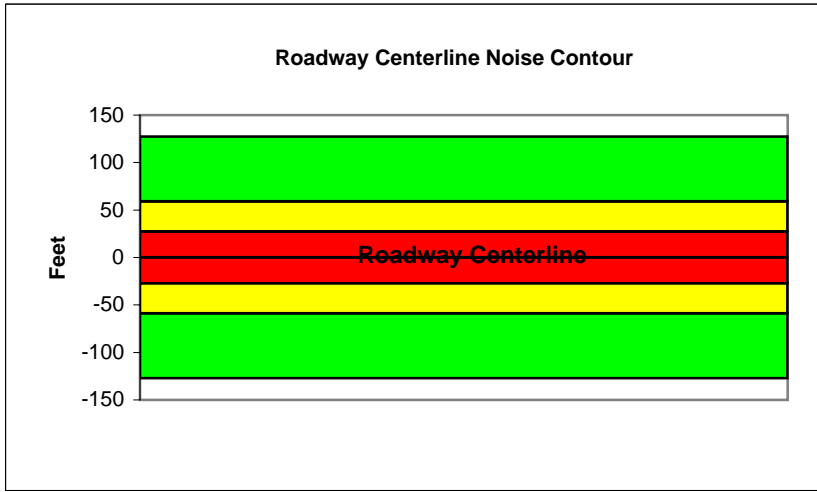
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 Without Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Palos Verdes Drive South  
 Road Segment: Between Palos Verdes Drive East and Western Avenue

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	15515			
Receiver Barrier Dist:	0	Peak Hour Traffic:	1551.5			
Centerline Dist. To Observer:	100	Vehicle Speed:	35			
Barrier Near Lane CL Dist:	0	Centerline Separation:	32			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	48.8	57.6	55.8	49.7	58.3	59.0
Medium Trucks:	58.5	50.4	44.1	42.5	51.0	51.2
Heavy Trucks:	63.7	51.8	42.7	44.0	53.9	54.0
<b>Vehicle Noise:</b>	<b>66.2</b>	<b>59.5</b>	<b>56.3</b>	<b>51.7</b>	<b>60.2</b>	<b>60.7</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	127
65 dBA	59
70 dBA	27
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

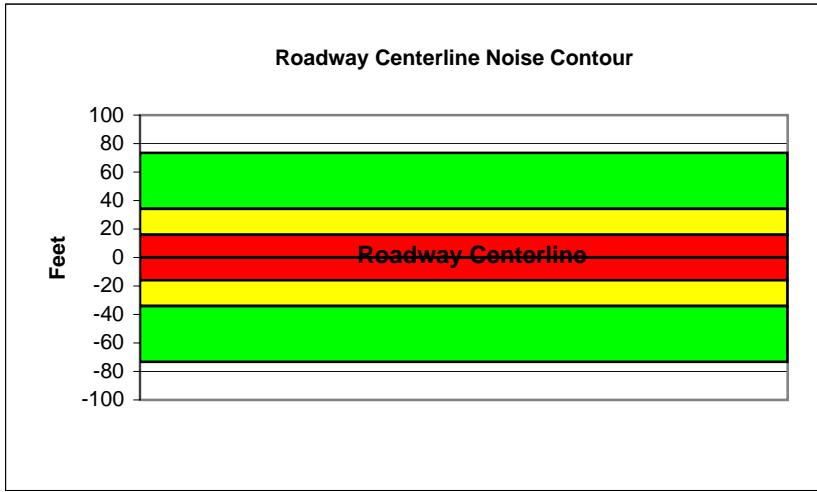
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 With Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Palos Verdes Drive East  
 Road Segment: Between Via Colinita And Crest Drive

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	9511			
Receiver Barrier Dist:	0	Peak Hour Traffic:	951.1			
Centerline Dist. To Observer:	100	Vehicle Speed:	30			
Barrier Near Lane CL Dist:	0	Centerline Separation:	12.5			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	45.3	54.0	52.2	46.2	54.8	55.4
Medium Trucks:	55.9	47.8	41.4	39.8	48.3	48.6
Heavy Trucks:	61.5	49.6	40.5	41.7	51.9	52.0
<b>Vehicle Noise:</b>	<b>64.0</b>	<b>56.5</b>	<b>53.0</b>	<b>48.6</b>	<b>57.2</b>	<b>57.6</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	73
65 dBA	34
70 dBA	16
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

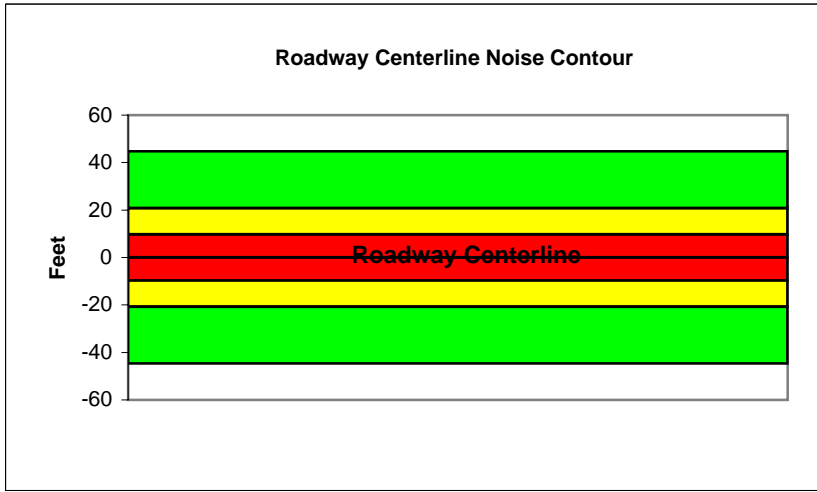
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 With Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Palos Verdes Drive East  
 Road Segment: Between Crest Drive and Palos Verdes Drive South

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	4524			
Receiver Barrier Dist:	0	Peak Hour Traffic:	452.4			
Centerline Dist. To Observer:	100	Vehicle Speed:	30			
Barrier Near Lane CL Dist:	0	Centerline Separation:	12.5			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	42.0	50.8	49.0	42.9	51.6	52.2
Medium Trucks:	52.6	44.6	38.2	36.6	45.1	45.3
Heavy Trucks:	58.3	46.3	37.3	38.5	48.6	48.8
<b>Vehicle Noise:</b>	<b>60.8</b>	<b>53.3</b>	<b>49.7</b>	<b>45.4</b>	<b>54.0</b>	<b>54.4</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	45
65 dBA	21
70 dBA	10
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

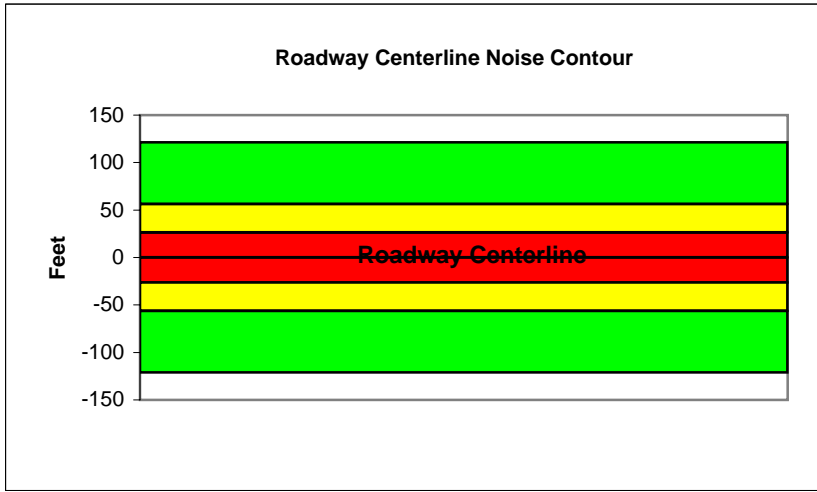
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 With Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Miraleste Drive  
 Road Segment: Between Palos Verdes Drive East and Via Colinita

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	14450			
Receiver Barrier Dist:	0	Peak Hour Traffic:	1445			
Centerline Dist. To Observer:	100	Vehicle Speed:	35			
Barrier Near Lane CL Dist:	0	Centerline Separation:	94			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	47.2	56.0	54.2	48.1	56.8	57.4
Medium Trucks:	56.9	48.9	42.5	40.9	49.4	49.6
Heavy Trucks:	62.2	50.2	41.2	42.4	52.3	52.4
<b>Vehicle Noise:</b>	<b>64.6</b>	<b>58.0</b>	<b>54.8</b>	<b>50.1</b>	<b>58.7</b>	<b>59.1</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	121
65 dBA	56
70 dBA	26
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

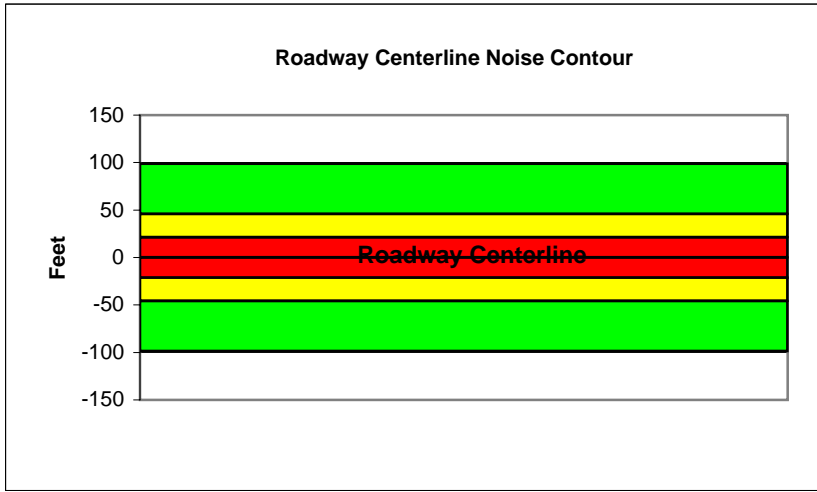
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 With Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Miraleste Drive  
 Road Segment: Between Via Colinita and First Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	10568			
Receiver Barrier Dist:	0	Peak Hour Traffic:	1056.8			
Centerline Dist. To Observer:	100	Vehicle Speed:	35			
Barrier Near Lane CL Dist:	0	Centerline Separation:	94			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	45.9	54.6	52.9	46.8	55.4	56.0
Medium Trucks:	55.6	47.5	41.1	39.6	48.0	48.3
Heavy Trucks:	60.8	48.9	39.8	41.0	50.9	51.1
<b>Vehicle Noise:</b>	<b>63.2</b>	<b>56.6</b>	<b>53.4</b>	<b>48.7</b>	<b>57.3</b>	<b>57.8</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	99
65 dBA	46
70 dBA	21
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

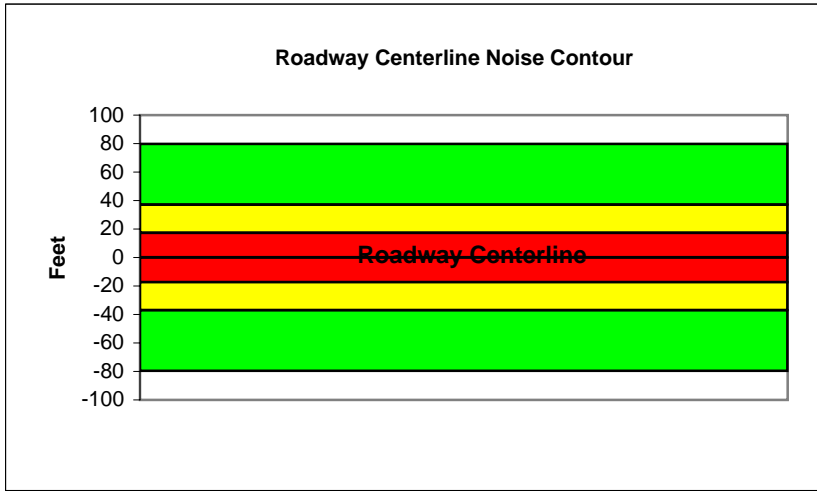
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 With Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Miraleste Drive  
 Road Segment: Between First Street and Western Avenue

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	7683			
Receiver Barrier Dist:	0	Peak Hour Traffic:	768.3			
Centerline Dist. To Observer:	100	Vehicle Speed:	35			
Barrier Near Lane CL Dist:	0	Centerline Separation:	28			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	45.8	54.6	52.8	46.7	55.4	56.0
Medium Trucks:	55.6	47.5	41.1	39.5	48.0	48.3
Heavy Trucks:	60.8	48.8	39.8	41.0	50.9	51.0
<b>Vehicle Noise:</b>	<b>63.2</b>	<b>56.6</b>	<b>53.4</b>	<b>48.7</b>	<b>57.3</b>	<b>57.7</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	80
65 dBA	37
70 dBA	17
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

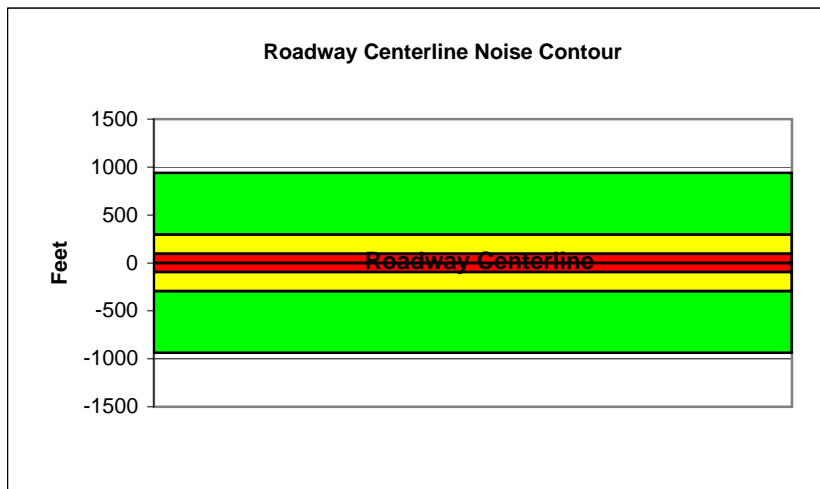
Project Name: Marymount College Facilities Expansion Project      Scenario: 2012 With Project  
Analyst: Michelle Dunn      Job #: 10-104089  
Roadway: Western Avenue  
Road Segment: Between Toscanini Drive and Capitol Drive

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	41,942			
Receiver Barrier Dist:	0	Peak Hour Traffic:	4194.2			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	48			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions <b>HARD SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	0	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.695	0.129	0.096	0.92
Rt View:      90      Lft View:      -90		Med. Truck	0.0144	0.0006	0.015	0.03
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.24	0.001	0.025	0.05
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	57.5	65.6	64.3	58.2	66.6	67.3
Medium Trucks:	66.5	42.8	35.0	44.2	50.4	50.4
Heavy Trucks:	71.3	62.1	44.3	53.5	63.5	63.5
<b>Vehicle Noise:</b>	<b>73.7</b>	<b>67.7</b>	<b>64.3</b>	<b>60.0</b>	<b>68.4</b>	<b>68.9</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	938
65 dBA	297
70 dBA	94
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

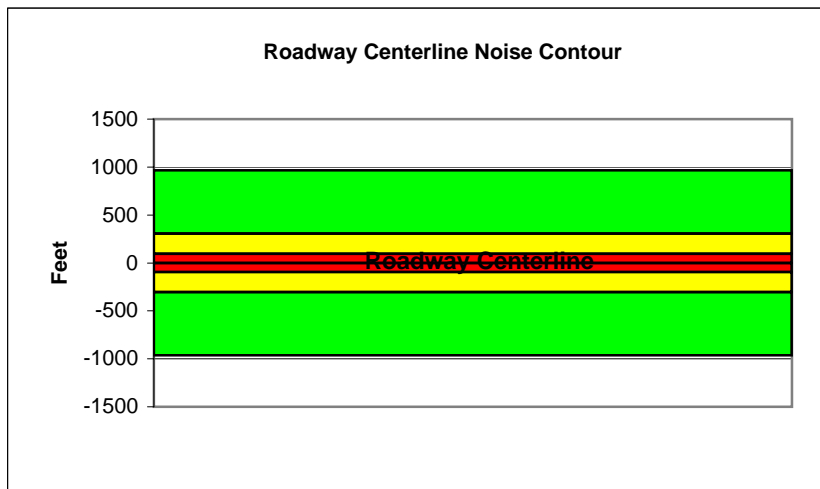
Project Name: Marymount College Facilities Expansion Project      Scenario: 2012 With Project  
Analyst: Michelle Dunn      Job #: 10-104089  
Roadway: Western Avenue  
Road Segment: Between Capitol Drive and Crestwood Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	43,231			
Receiver Barrier Dist:	0	Peak Hour Traffic:	4323.1			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	48			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions <b>HARD SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	0	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.695	0.129	0.096	0.92
Rt View:      90      Lft View:      -90		Med. Truck	0.0144	0.0006	0.015	0.03
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.24	0.001	0.025	0.05
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	57.7	65.7	64.4	58.3	66.8	67.4
Medium Trucks:	66.6	43.0	35.2	44.3	50.5	50.5
Heavy Trucks:	71.5	62.3	44.5	53.6	63.7	63.7
<b>Vehicle Noise:</b>	<b>73.8</b>	<b>67.8</b>	<b>64.5</b>	<b>60.1</b>	<b>68.6</b>	<b>69.0</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	966
65 dBA	306
70 dBA	97
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

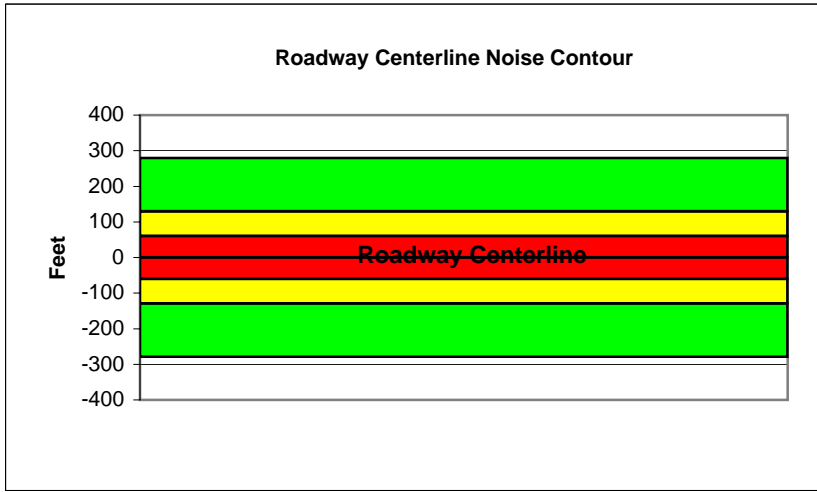
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 With Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between Crestwood Street and First Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	37053			
Receiver Barrier Dist:	0	Peak Hour Traffic:	3705.3			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	48			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	53.9	62.6	60.9	54.8	63.4	64.0
Medium Trucks:	62.8	54.8	48.4	46.8	55.3	55.5
Heavy Trucks:	67.7	55.7	46.7	47.9	57.6	57.7
<b>Vehicle Noise:</b>	<b>70.0</b>	<b>64.2</b>	<b>61.3</b>	<b>56.4</b>	<b>64.9</b>	<b>65.4</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	279
65 dBA	130
70 dBA	60
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

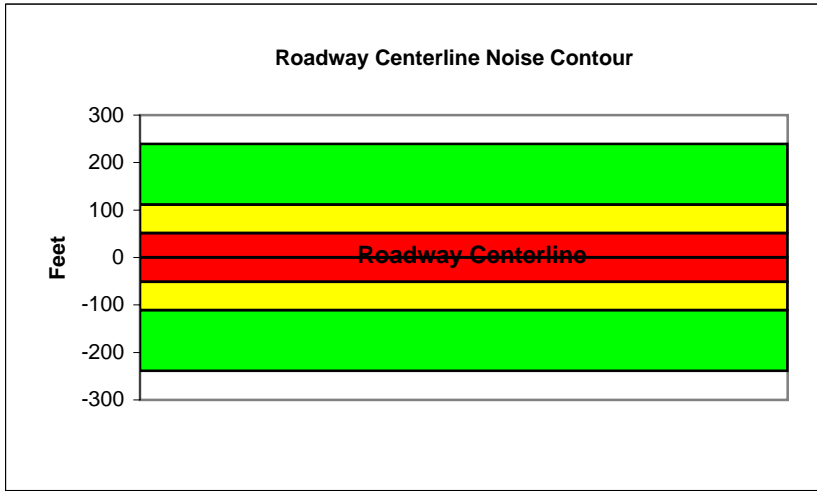
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 With Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between First Street and Ninth Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	29360			
Receiver Barrier Dist:	0	Peak Hour Traffic:	2936			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	48			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	52.9	61.6	59.8	53.8	62.4	63.0
Medium Trucks:	61.8	53.7	47.4	45.8	54.3	54.5
Heavy Trucks:	66.7	54.7	45.7	46.9	56.6	56.7
<b>Vehicle Noise:</b>	<b>69.0</b>	<b>63.2</b>	<b>60.3</b>	<b>55.3</b>	<b>63.9</b>	<b>64.4</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	239
65 dBA	111
70 dBA	52
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

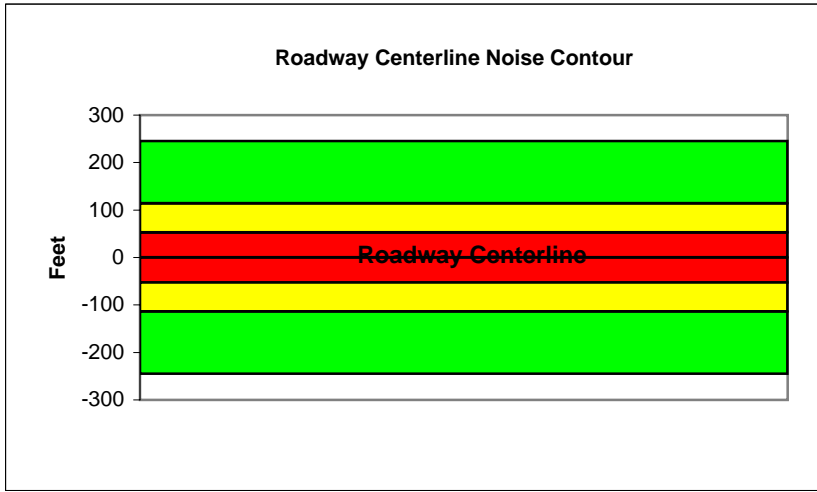
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 With Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between Ninth Street and West 25th Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	30488			
Receiver Barrier Dist:	0	Peak Hour Traffic:	3048.8			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	43			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	53.1	61.9	60.1	54.0	62.7	63.3
Medium Trucks:	62.1	54.0	47.6	46.1	54.5	54.8
Heavy Trucks:	66.9	55.0	45.9	47.2	56.9	57.0
<b>Vehicle Noise:</b>	<b>69.3</b>	<b>63.5</b>	<b>60.6</b>	<b>55.6</b>	<b>64.2</b>	<b>64.7</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	245
65 dBA	114
70 dBA	53
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

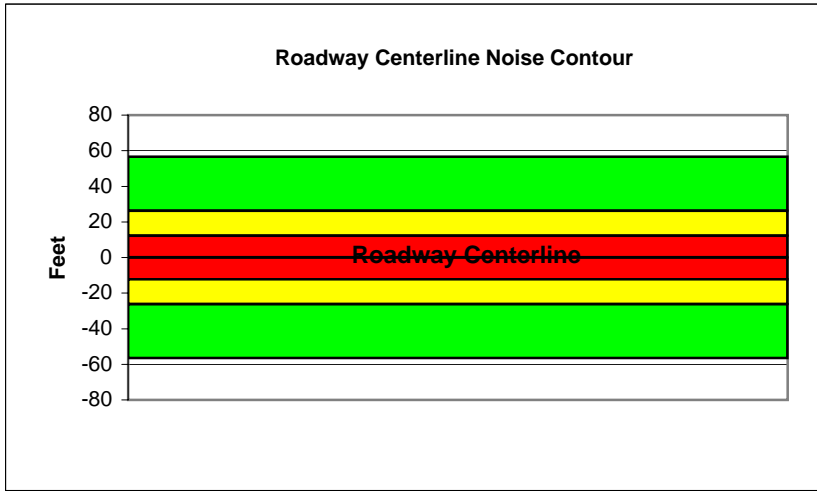
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 With Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: First Street  
 Road Segment: Between Miraleste Drive and Western Avenue

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	6407			
Receiver Barrier Dist:	0	Peak Hour Traffic:	640.7			
Centerline Dist. To Observer:	100	Vehicle Speed:	30			
Barrier Near Lane CL Dist:	0	Centerline Separation:	14			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	43.5	52.3	50.5	44.4	53.1	53.7
Medium Trucks:	54.1	46.0	39.6	38.1	46.6	46.8
Heavy Trucks:	59.7	47.8	38.8	40.0	50.1	50.2
<b>Vehicle Noise:</b>	<b>62.3</b>	<b>54.8</b>	<b>51.2</b>	<b>46.9</b>	<b>55.4</b>	<b>55.9</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	57
65 dBA	26
70 dBA	12
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

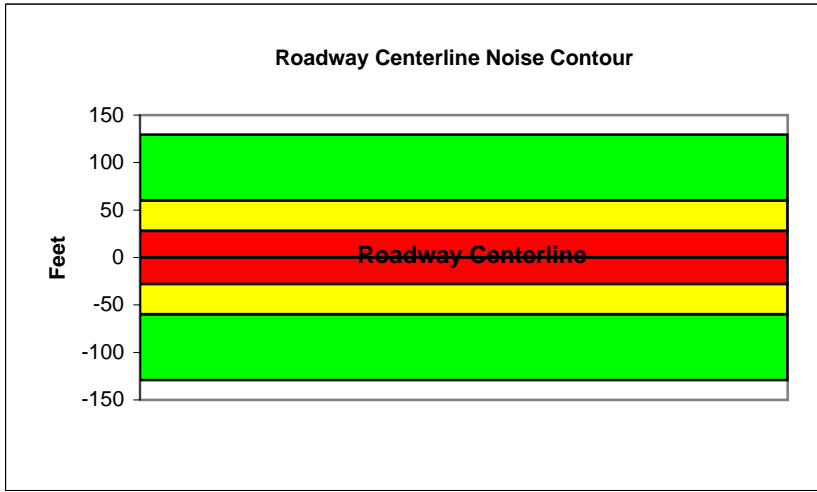
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 With Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Palos Verdes Drive South  
 Road Segment: Between Palos Verdes Drive East and Western Avenue

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	15901			
Receiver Barrier Dist:	0	Peak Hour Traffic:	1590.1			
Centerline Dist. To Observer:	100	Vehicle Speed:	35			
Barrier Near Lane CL Dist:	0	Centerline Separation:	32			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	48.9	57.7	55.9	49.8	58.5	59.1
Medium Trucks:	58.6	50.5	44.2	42.6	51.1	51.3
Heavy Trucks:	63.8	51.9	42.8	44.1	54.0	54.1
<b>Vehicle Noise:</b>	<b>66.3</b>	<b>59.6</b>	<b>56.4</b>	<b>51.8</b>	<b>60.3</b>	<b>60.8</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	130
65 dBA	60
70 dBA	28
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

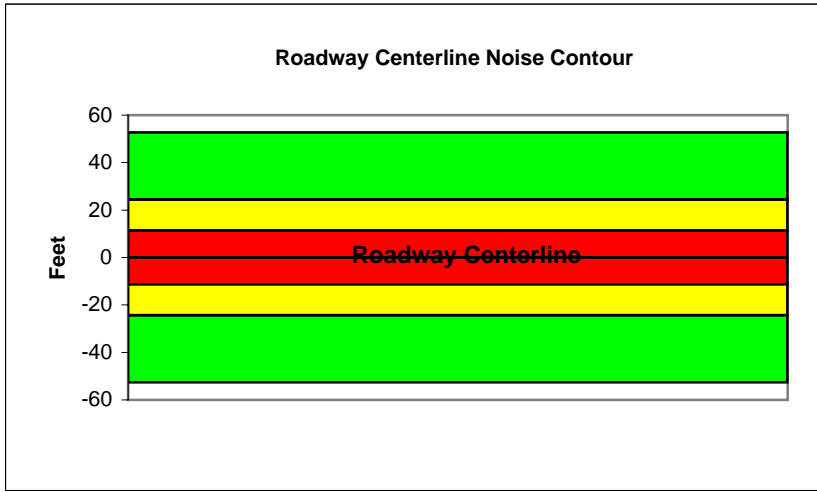
Project Name: Marymount College Facilities Expansion Project Scenario: Existing  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Palos Verdes Drive East  
 Road Segment: Between Via Colinita And Crest Drive

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	5766			
Receiver Barrier Dist:	0	Peak Hour Traffic:	576.6			
Centerline Dist. To Observer:	100	Vehicle Speed:	30			
Barrier Near Lane CL Dist:	0	Centerline Separation:	12.5			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	43.1	51.9	50.1	44.0	52.6	53.2
Medium Trucks:	53.7	45.6	39.2	37.7	46.1	46.4
Heavy Trucks:	59.3	47.4	38.3	39.6	49.7	49.8
<b>Vehicle Noise:</b>	<b>61.9</b>	<b>54.3</b>	<b>50.8</b>	<b>46.5</b>	<b>55.0</b>	<b>55.4</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	53
65 dBA	24
70 dBA	11
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

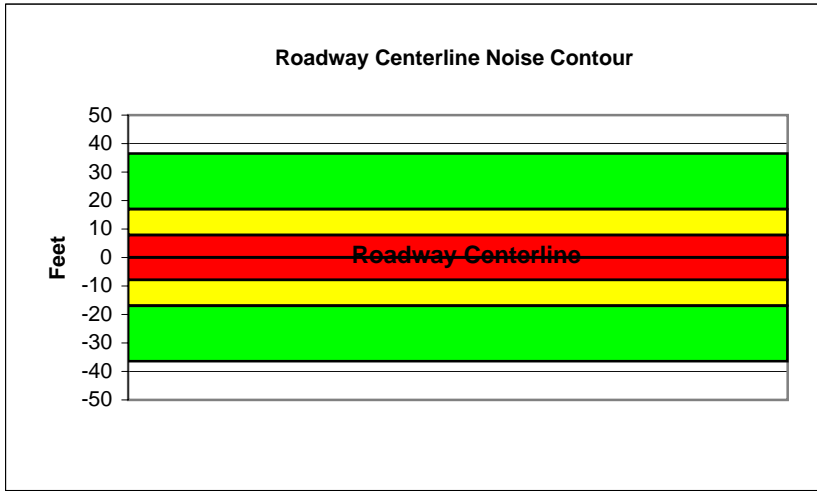
Project Name: Marymount College Facilities Expansion Project Scenario: Existing  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Palos Verdes Drive East  
 Road Segment: Between Crest Drive and Palos Verdes Drive South

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	3326			
Receiver Barrier Dist:	0	Peak Hour Traffic:	332.6			
Centerline Dist. To Observer:	100	Vehicle Speed:	30			
Barrier Near Lane CL Dist:	0	Centerline Separation:	12.5			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	40.7	49.5	47.7	41.6	50.3	50.9
Medium Trucks:	51.3	43.2	36.8	35.3	43.8	44.0
Heavy Trucks:	56.9	45.0	35.9	37.2	47.3	47.4
<b>Vehicle Noise:</b>	<b>59.5</b>	<b>52.0</b>	<b>48.4</b>	<b>44.1</b>	<b>52.6</b>	<b>53.1</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	36
65 dBA	17
70 dBA	8
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

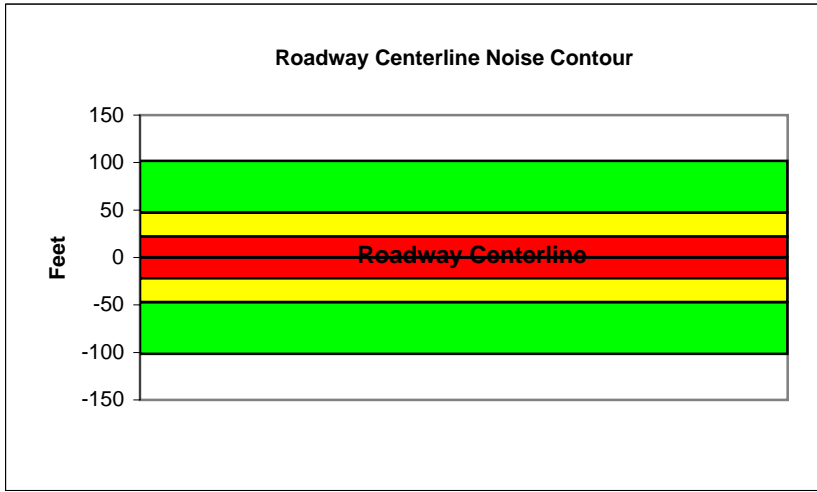
Project Name: Marymount College Facilities Expansion Project Scenario: Existing  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Miraleste Drive  
 Road Segment: Between Palos Verdes Drive East and Via Colinita

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	11103			
Receiver Barrier Dist:	0	Peak Hour Traffic:	1110.3			
Centerline Dist. To Observer:	100	Vehicle Speed:	35			
Barrier Near Lane CL Dist:	0	Centerline Separation:	94			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	46.1	54.9	53.1	47.0	55.6	56.2
Medium Trucks:	55.8	47.7	41.3	39.8	48.3	48.5
Heavy Trucks:	61.0	49.1	40.0	41.2	51.1	51.3
<b>Vehicle Noise:</b>	<b>63.5</b>	<b>56.8</b>	<b>53.6</b>	<b>48.9</b>	<b>57.5</b>	<b>58.0</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	102
65 dBA	47
70 dBA	22
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

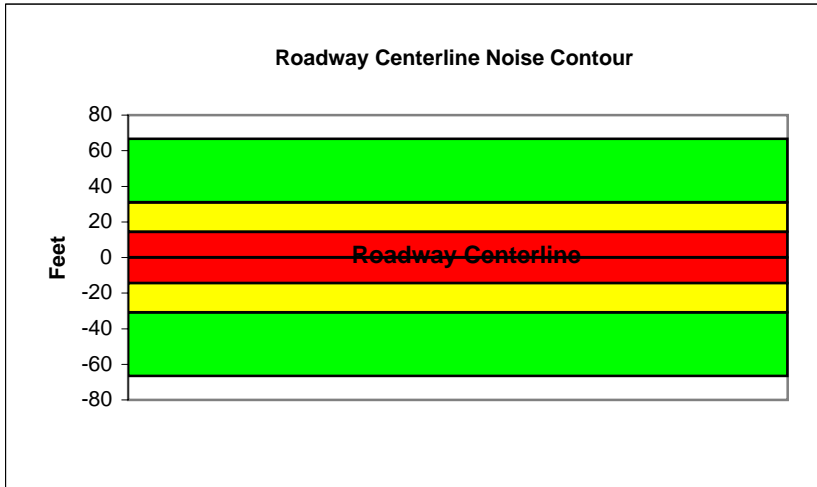
Project Name: Marymount College Facilities Expansion Project Scenario: Existing  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Miraleste Drive  
 Road Segment: Between Via Colinita and First Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	5880			
Receiver Barrier Dist:	0	Peak Hour Traffic:	588			
Centerline Dist. To Observer:	100	Vehicle Speed:	35			
Barrier Near Lane CL Dist:	0	Centerline Separation:	94			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	43.3	52.1	50.3	44.2	52.9	53.5
Medium Trucks:	53.0	45.0	38.6	37.0	45.5	45.7
Heavy Trucks:	58.3	46.3	37.3	38.5	48.4	48.5
<b>Vehicle Noise:</b>	<b>60.7</b>	<b>54.1</b>	<b>50.9</b>	<b>46.2</b>	<b>54.7</b>	<b>55.2</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	67
65 dBA	31
70 dBA	14
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

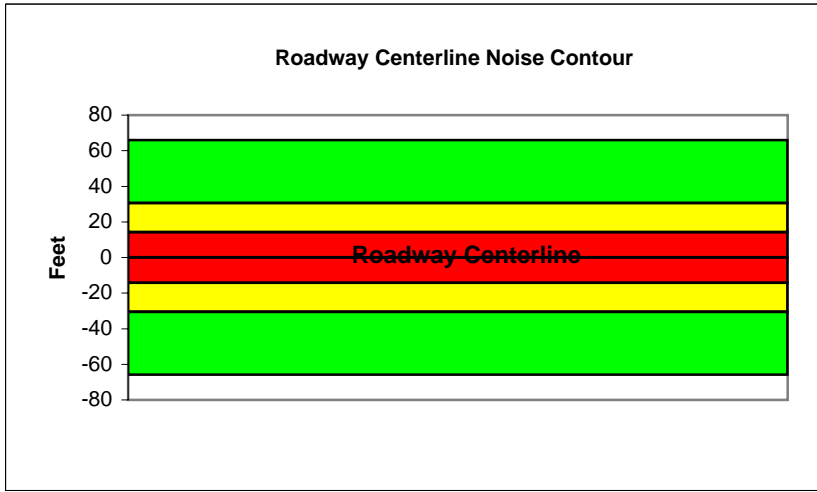
Project Name: Marymount College Facilities Expansion Project Scenario: Existing  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Miraleste Drive  
 Road Segment: Between First Street and Western Avenue

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	5798			
Receiver Barrier Dist:	0	Peak Hour Traffic:	579.8			
Centerline Dist. To Observer:	100	Vehicle Speed:	35			
Barrier Near Lane CL Dist:	0	Centerline Separation:	28			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	44.6	53.4	51.6	45.5	54.2	54.8
Medium Trucks:	54.3	46.3	39.9	38.3	46.8	47.0
Heavy Trucks:	59.5	47.6	38.6	39.8	49.7	49.8
<b>Vehicle Noise:</b>	<b>62.0</b>	<b>55.3</b>	<b>52.2</b>	<b>47.5</b>	<b>56.0</b>	<b>56.5</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	66
65 dBA	31
70 dBA	14
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

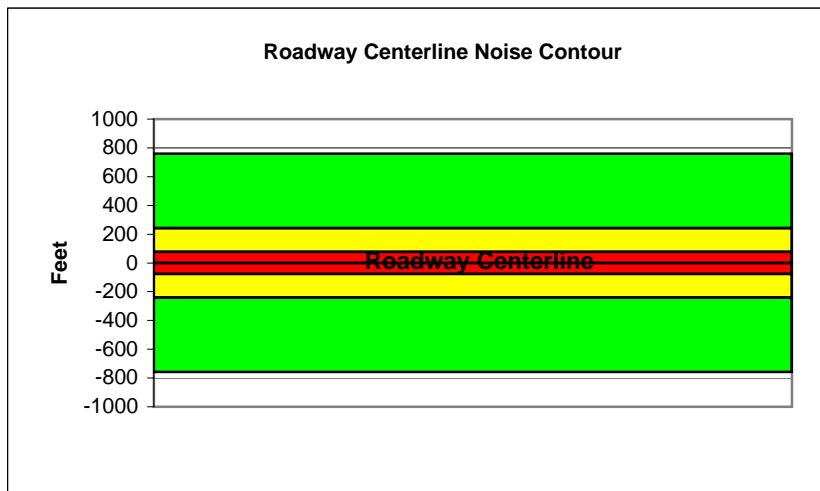
Project Name: Marymount College Facilities Expansion Project      Scenario: Existing  
 Analyst: Michelle Dunn      Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between Toscanini Drive and Capitol Drive

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	34,038			
Receiver Barrier Dist:	0	Peak Hour Traffic:	3403.8			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	48			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions <b>HARD SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	0	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.695	0.129	0.096	0.92
Rt View:      90      Lft View:      -90		Med. Truck	0.0144	0.0006	0.015	0.03
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.24	0.001	0.025	0.05
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	56.6	64.7	63.4	57.3	65.7	66.4
Medium Trucks:	65.6	41.9	34.1	43.3	49.5	49.5
Heavy Trucks:	70.4	61.2	43.4	52.6	62.6	62.6
<b>Vehicle Noise:</b>	<b>72.8</b>	<b>66.8</b>	<b>63.4</b>	<b>59.0</b>	<b>67.5</b>	<b>68.0</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	761
65 dBA	241
70 dBA	76
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

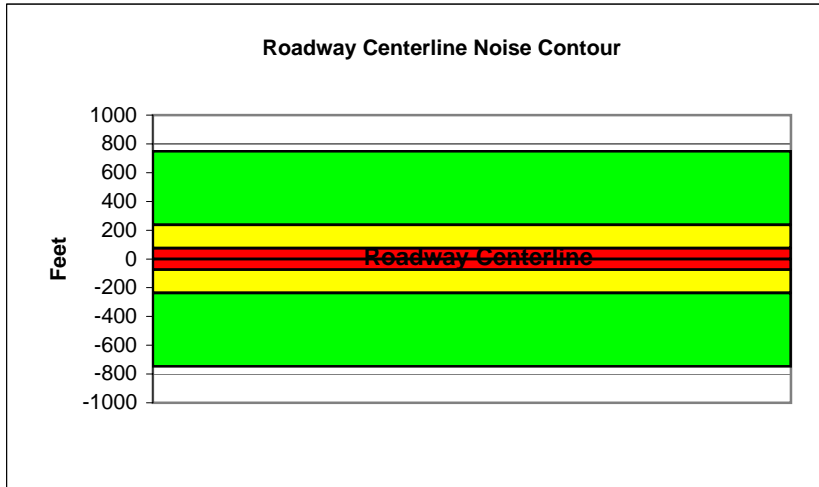
Project Name: Marymount College Facilities Expansion Project      Scenario: Existing  
 Analyst: Michelle Dunn      Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between Capitol Drive and Crestwood Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	33,493			
Receiver Barrier Dist:	0	Peak Hour Traffic:	3349.3			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	48			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions <b>HARD SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	0	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.695	0.129	0.096	0.92
Rt View:      90      Lft View:      -90		Med. Truck	0.0144	0.0006	0.015	0.03
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.24	0.001	0.025	0.05
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	56.6	64.6	63.3	57.2	65.7	66.3
Medium Trucks:	65.5	41.9	34.1	43.2	49.4	49.4
Heavy Trucks:	70.4	61.1	43.3	52.5	62.6	62.6
<b>Vehicle Noise:</b>	<b>72.7</b>	<b>66.7</b>	<b>63.4</b>	<b>59.0</b>	<b>67.5</b>	<b>67.9</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	748
65 dBA	237
70 dBA	75
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

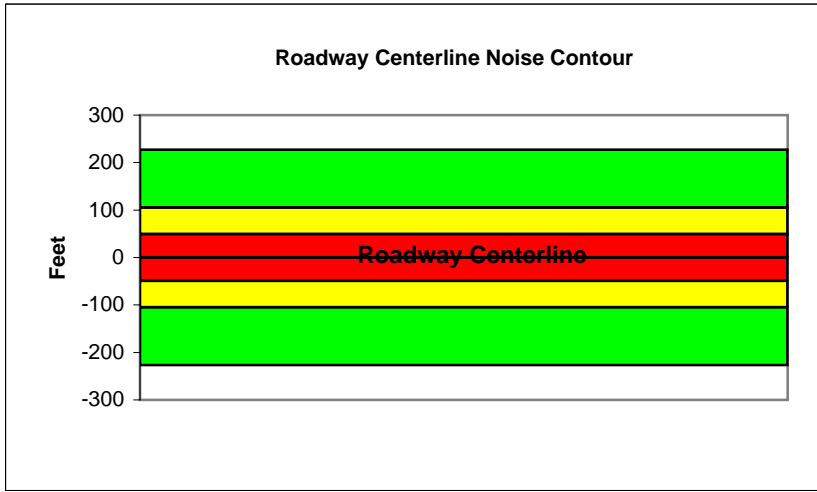
Project Name: Marymount College Facilities Expansion Project Scenario: Existing  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between Crestwood Street and First Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	27121			
Receiver Barrier Dist:	0	Peak Hour Traffic:	2712.1			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	48			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	52.5	61.3	59.5	53.4	62.1	62.7
Medium Trucks:	61.5	53.4	47.0	45.4	53.9	54.2
Heavy Trucks:	66.3	54.4	45.3	46.5	56.3	56.4
<b>Vehicle Noise:</b>	<b>68.7</b>	<b>62.9</b>	<b>59.9</b>	<b>55.0</b>	<b>63.6</b>	<b>64.1</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	227
65 dBA	105
70 dBA	49
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

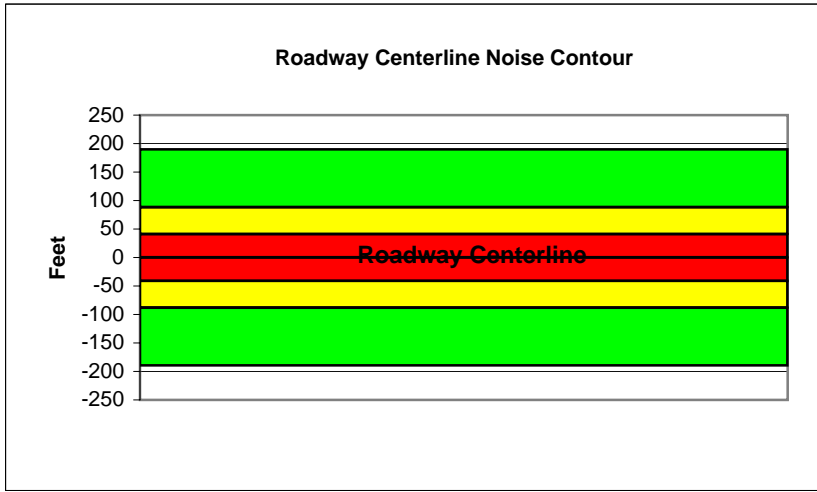
Project Name: Marymount College Facilities Expansion Project Scenario: Existing  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between First Street and Ninth Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	20749			
Receiver Barrier Dist:	0	Peak Hour Traffic:	2074.9			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	48			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	51.3	60.1	58.3	52.3	60.9	61.5
Medium Trucks:	60.3	52.2	45.9	44.3	52.8	53.0
Heavy Trucks:	65.1	53.2	44.2	45.4	55.1	55.2
<b>Vehicle Noise:</b>	<b>67.5</b>	<b>61.7</b>	<b>58.8</b>	<b>53.8</b>	<b>62.4</b>	<b>62.9</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	190
65 dBA	88
70 dBA	41
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

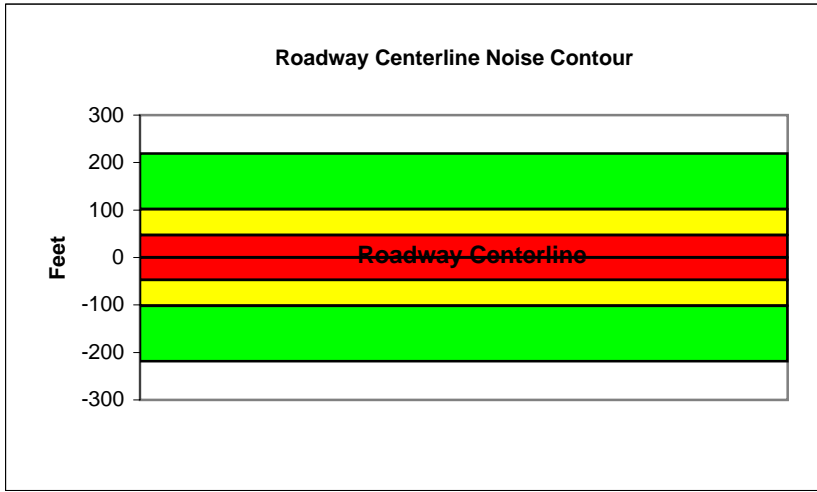
Project Name: Marymount College Facilities Expansion Project Scenario: Existing  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between Ninth Street and West 25th Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	25726			
Receiver Barrier Dist:	0	Peak Hour Traffic:	2572.6			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	43			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	52.4	61.2	59.4	53.3	62.0	62.6
Medium Trucks:	61.3	53.3	46.9	45.3	53.8	54.0
Heavy Trucks:	66.2	54.3	45.2	46.4	56.1	56.3
<b>Vehicle Noise:</b>	<b>68.6</b>	<b>62.8</b>	<b>59.8</b>	<b>54.9</b>	<b>63.5</b>	<b>63.9</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	219
65 dBA	102
70 dBA	47
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

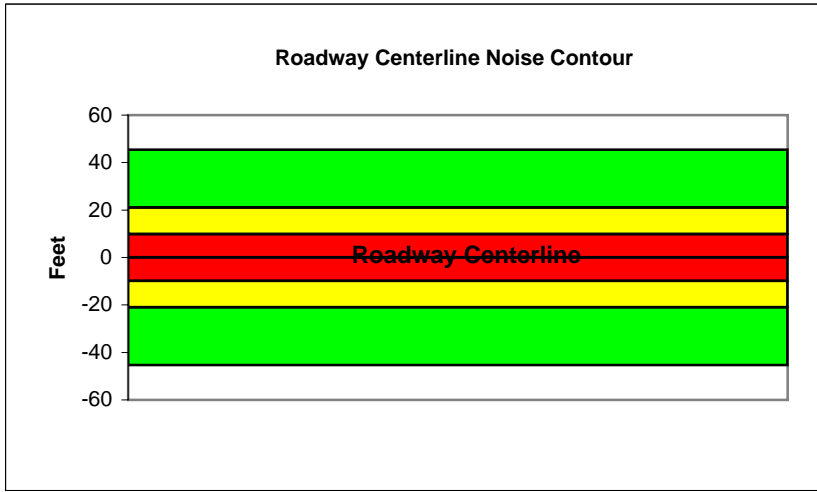
Project Name: Marymount College Facilities Expansion Project Scenario: Existing  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: First Street  
 Road Segment: Between Miraleste Drive and Western Avenue

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	4616			
Receiver Barrier Dist:	0	Peak Hour Traffic:	461.6			
Centerline Dist. To Observer:	100	Vehicle Speed:	30			
Barrier Near Lane CL Dist:	0	Centerline Separation:	14			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	42.1	50.9	49.1	43.0	51.6	52.2
Medium Trucks:	52.7	44.6	38.2	36.7	45.1	45.4
Heavy Trucks:	58.3	46.4	37.3	38.5	48.7	48.8
<b>Vehicle Noise:</b>	<b>60.9</b>	<b>53.3</b>	<b>49.8</b>	<b>45.5</b>	<b>54.0</b>	<b>54.4</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	45
65 dBA	21
70 dBA	10
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

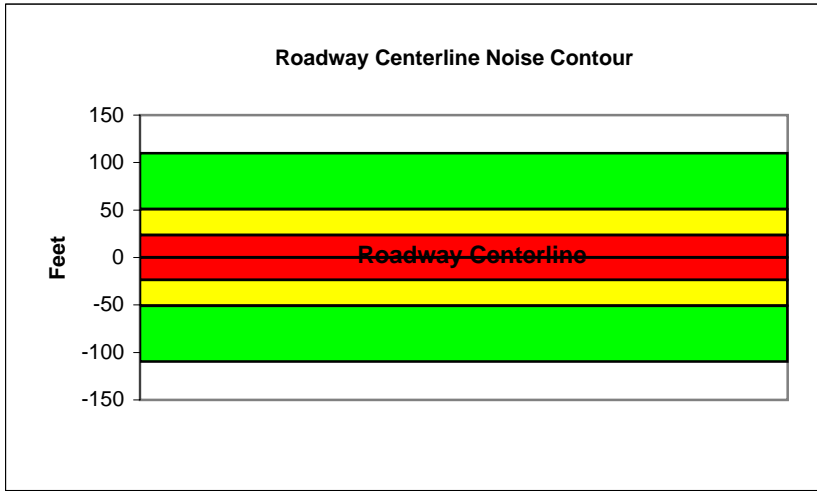
Project Name: Marymount College Facilities Expansion Project Scenario: Existing  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Palos Verdes Drive South  
 Road Segment: Between Palos Verdes Drive East and Western Avenue

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	12444			
Receiver Barrier Dist:	0	Peak Hour Traffic:	1244.4			
Centerline Dist. To Observer:	100	Vehicle Speed:	35			
Barrier Near Lane CL Dist:	0	Centerline Separation:	32			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	47.8	56.6	54.8	48.7	57.4	58.0
Medium Trucks:	57.5	49.5	43.1	41.5	50.0	50.2
Heavy Trucks:	62.8	50.8	41.8	43.0	52.9	53.0
<b>Vehicle Noise:</b>	<b>65.2</b>	<b>58.6</b>	<b>55.4</b>	<b>50.7</b>	<b>59.3</b>	<b>59.7</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	110
65 dBA	51
70 dBA	24
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

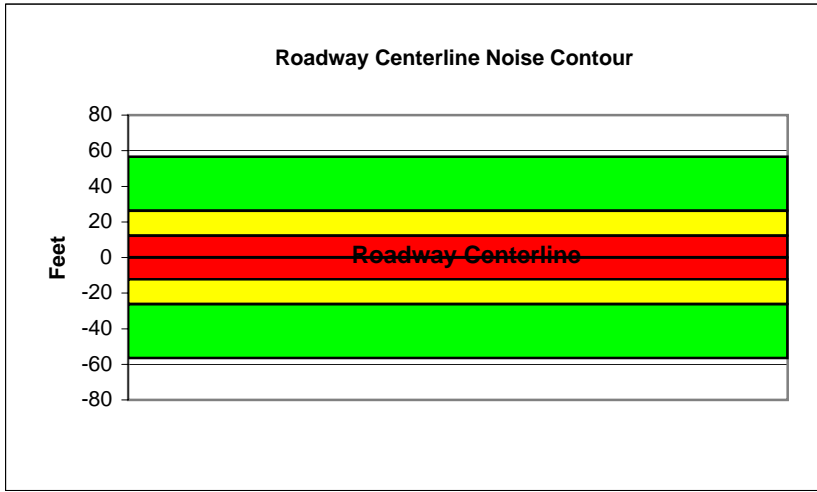
Project Name: Marymount College Facilities Expansion Project Scenario: Existing Plus Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Palos Verdes Drive East  
 Road Segment: Between Via Colinita And Crest Drive

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	6432			
Receiver Barrier Dist:	0	Peak Hour Traffic:	643.2			
Centerline Dist. To Observer:	100	Vehicle Speed:	30			
Barrier Near Lane CL Dist:	0	Centerline Separation:	12.5			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	43.6	52.3	50.5	44.5	53.1	53.7
Medium Trucks:	54.2	46.1	39.7	38.1	46.6	46.9
Heavy Trucks:	59.8	47.9	38.8	40.0	50.2	50.3
<b>Vehicle Noise:</b>	<b>62.3</b>	<b>54.8</b>	<b>51.3</b>	<b>46.9</b>	<b>55.5</b>	<b>55.9</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	57
65 dBA	26
70 dBA	12
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

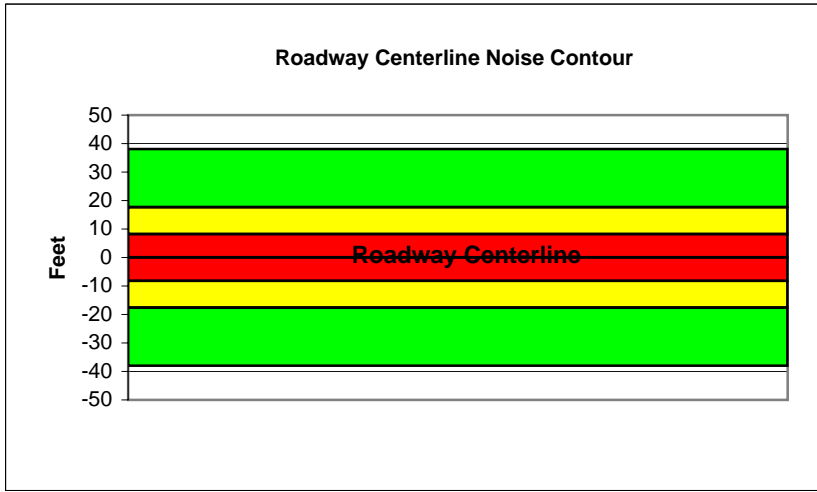
Project Name: Marymount College Facilities Expansion Project Scenario: Existing Plus Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Palos Verdes Drive East  
 Road Segment: Between Crest Drive and Palos Verdes Drive South

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	3548			
Receiver Barrier Dist:	0	Peak Hour Traffic:	354.8			
Centerline Dist. To Observer:	100	Vehicle Speed:	30			
Barrier Near Lane CL Dist:	0	Centerline Separation:	12.5			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	41.0	49.8	48.0	41.9	50.5	51.1
Medium Trucks:	51.6	43.5	37.1	35.6	44.0	44.3
Heavy Trucks:	57.2	45.3	36.2	37.4	47.6	47.7
<b>Vehicle Noise:</b>	<b>59.8</b>	<b>52.2</b>	<b>48.7</b>	<b>44.4</b>	<b>52.9</b>	<b>53.3</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	38
65 dBA	18
70 dBA	8
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

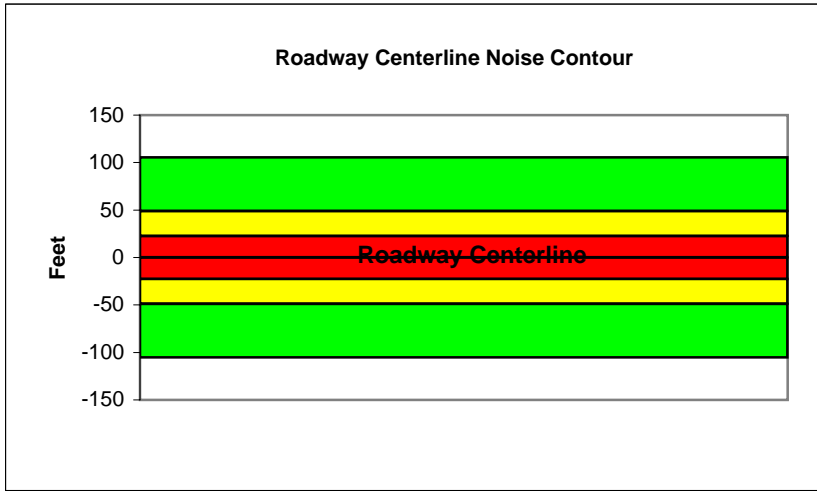
Project Name: Marymount College Facilities Expansion Project Scenario: Existing Plus Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Miraleste Drive  
 Road Segment: Between Palos Verdes Drive East and Via Colinita

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	11680			
Receiver Barrier Dist:	0	Peak Hour Traffic:	1168			
Centerline Dist. To Observer:	100	Vehicle Speed:	35			
Barrier Near Lane CL Dist:	0	Centerline Separation:	94			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	46.3	55.1	53.3	47.2	55.9	56.5
Medium Trucks:	56.0	48.0	41.6	40.0	48.5	48.7
Heavy Trucks:	61.2	49.3	40.2	41.5	51.4	51.5
<b>Vehicle Noise:</b>	<b>63.7</b>	<b>57.0</b>	<b>53.9</b>	<b>49.2</b>	<b>57.7</b>	<b>58.2</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	105
65 dBA	49
70 dBA	23
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

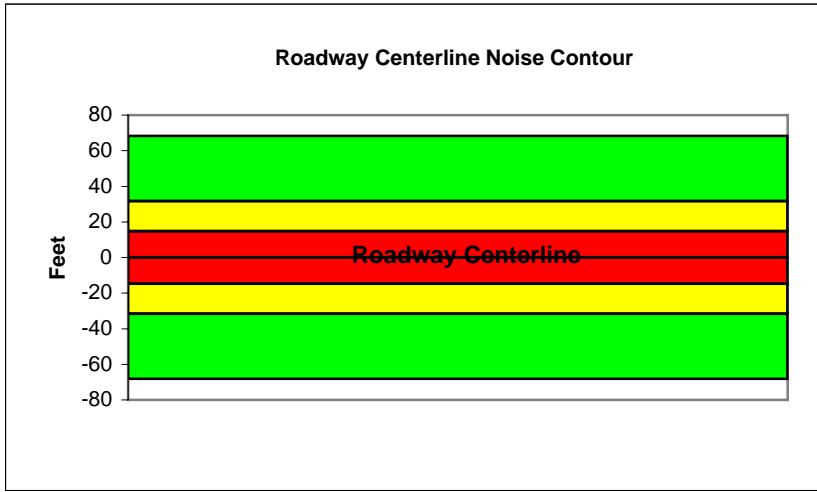
Project Name: Marymount College Facilities Expansion Project Scenario: Existing Plus Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Miraleste Drive  
 Road Segment: Between Via Colinita and First Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	6102			
Receiver Barrier Dist:	0	Peak Hour Traffic:	610.2			
Centerline Dist. To Observer:	100	Vehicle Speed:	35			
Barrier Near Lane CL Dist:	0	Centerline Separation:	94			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	43.5	52.3	50.5	44.4	53.0	53.6
Medium Trucks:	53.2	45.1	38.7	37.2	45.7	45.9
Heavy Trucks:	58.4	46.5	37.4	38.6	48.5	48.7
<b>Vehicle Noise:</b>	<b>60.9</b>	<b>54.2</b>	<b>51.0</b>	<b>46.3</b>	<b>54.9</b>	<b>55.4</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	68
65 dBA	32
70 dBA	15
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

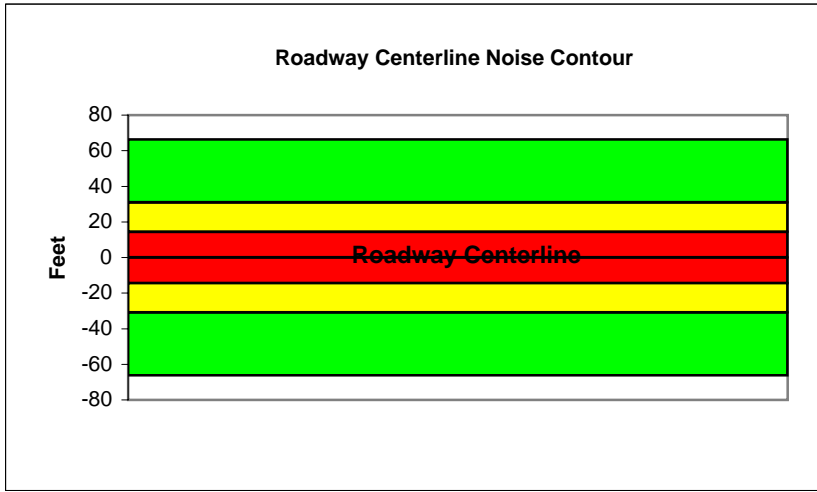
Project Name: Marymount College Facilities Expansion Project Scenario: Existing Plus Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Miraleste Drive  
 Road Segment: Between First Street and Western Avenue

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	5842			
Receiver Barrier Dist:	0	Peak Hour Traffic:	584.2			
Centerline Dist. To Observer:	100	Vehicle Speed:	35			
Barrier Near Lane CL Dist:	0	Centerline Separation:	28			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	44.6	53.4	51.6	45.6	54.2	54.8
Medium Trucks:	54.4	46.3	39.9	38.3	46.8	47.1
Heavy Trucks:	59.6	47.6	38.6	39.8	49.7	49.8
<b>Vehicle Noise:</b>	<b>62.0</b>	<b>55.4</b>	<b>52.2</b>	<b>47.5</b>	<b>56.1</b>	<b>56.5</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	66
65 dBA	31
70 dBA	14
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

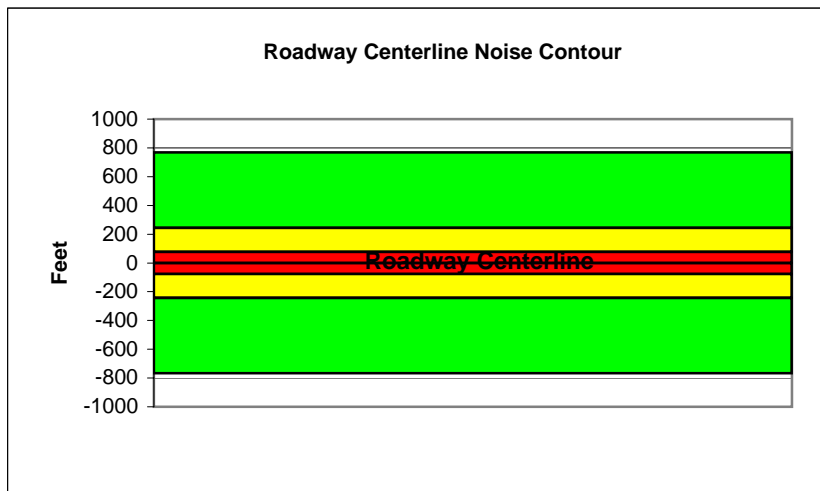
Project Name: Marymount College Facilities Expansion Project      Scenario: Existing Plus Project  
 Analyst: Michelle Dunn      Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between Toscanini Drive and Capitol Drive

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	34,393			
Receiver Barrier Dist:	0	Peak Hour Traffic:	3439.3			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	48			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions <b>HARD SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	0	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.695	0.129	0.096	0.92
Rt View:      90      Lft View:      -90		Med. Truck	0.0144	0.0006	0.015	0.03
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.24	0.001	0.025	0.05
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	56.7	64.7	63.4	57.3	65.8	66.4
Medium Trucks:	65.6	42.0	34.2	43.4	49.5	49.6
Heavy Trucks:	70.5	61.3	43.5	52.6	62.7	62.7
<b>Vehicle Noise:</b>	<b>72.8</b>	<b>66.8</b>	<b>63.5</b>	<b>59.1</b>	<b>67.6</b>	<b>68.0</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	769
65 dBA	243
70 dBA	77
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

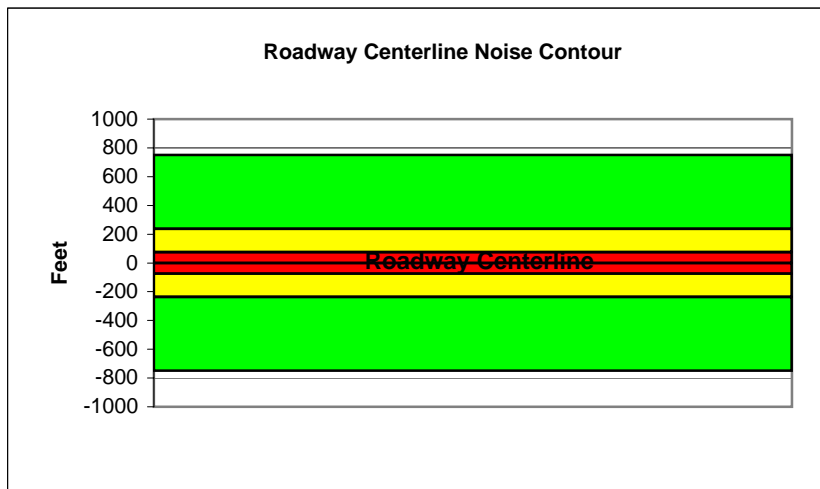
Project Name: Marymount College Facilities Expansion Project      Scenario: Existing Plus Project  
Analyst: Michelle Dunn      Job #: 10-104089  
Roadway: Western Avenue  
Road Segment: Between Capitol Drive and Crestwood Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	33,626			
Receiver Barrier Dist:	0	Peak Hour Traffic:	3362.6			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	48			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions <b>HARD SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	0	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.695	0.129	0.096	0.92
Rt View:      90      Lft View:      -90		Med. Truck	0.0144	0.0006	0.015	0.03
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.24	0.001	0.025	0.05
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	56.6	64.6	63.3	57.2	65.7	66.3
Medium Trucks:	65.5	41.9	34.1	43.3	49.4	49.5
Heavy Trucks:	70.4	61.2	43.4	52.5	62.6	62.6
<b>Vehicle Noise:</b>	<b>72.7</b>	<b>66.7</b>	<b>63.4</b>	<b>59.0</b>	<b>67.5</b>	<b>67.9</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	752
65 dBA	238
70 dBA	75
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

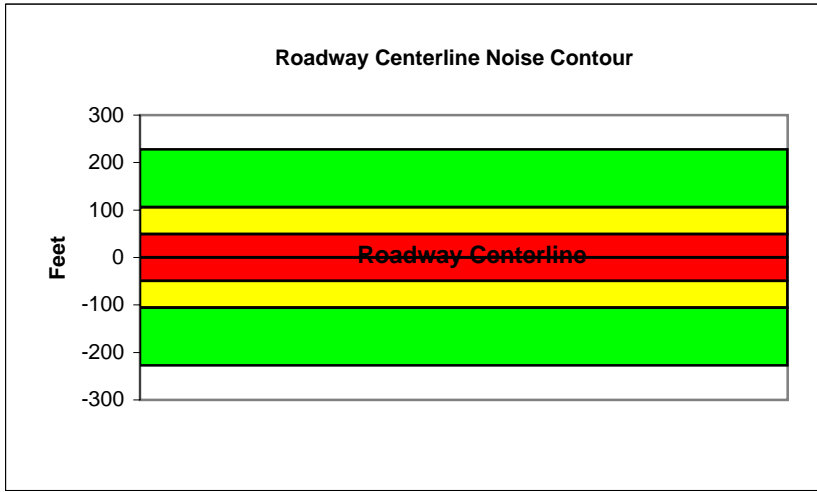
Project Name: Marymount College Facilities Expansion Project Scenario: Existing Plus Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between Crestwood Street and First Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	27254			
Receiver Barrier Dist:	0	Peak Hour Traffic:	2725.4			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	48			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	52.5	61.3	59.5	53.4	62.1	62.7
Medium Trucks:	61.5	53.4	47.0	45.5	53.9	54.2
Heavy Trucks:	66.3	54.4	45.3	46.6	56.3	56.4
<b>Vehicle Noise:</b>	<b>68.7</b>	<b>62.9</b>	<b>60.0</b>	<b>55.0</b>	<b>63.6</b>	<b>64.1</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	228
65 dBA	106
70 dBA	49
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

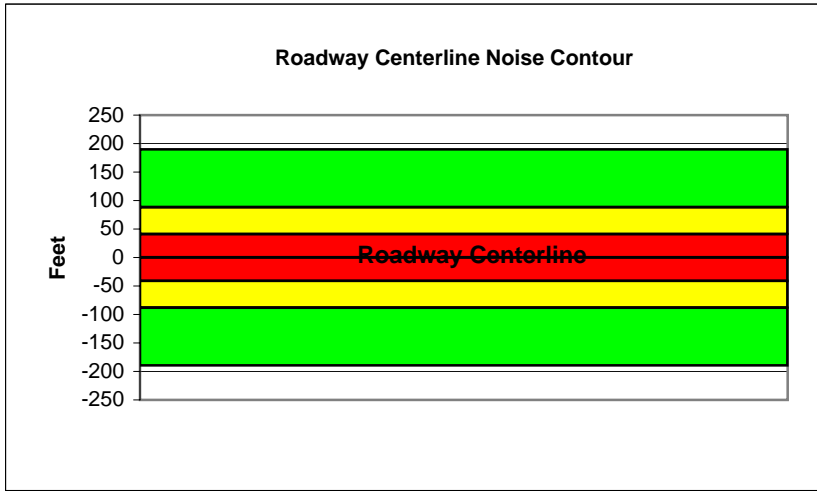
Project Name: Marymount College Facilities Expansion Project Scenario: Existing Plus Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between First Street and Ninth Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	20749			
Receiver Barrier Dist:	0	Peak Hour Traffic:	2074.9			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	48			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	51.3	60.1	58.3	52.3	60.9	61.5
Medium Trucks:	60.3	52.2	45.9	44.3	52.8	53.0
Heavy Trucks:	65.1	53.2	44.2	45.4	55.1	55.2
<b>Vehicle Noise:</b>	<b>67.5</b>	<b>61.7</b>	<b>58.8</b>	<b>53.8</b>	<b>62.4</b>	<b>62.9</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	190
65 dBA	88
70 dBA	41
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

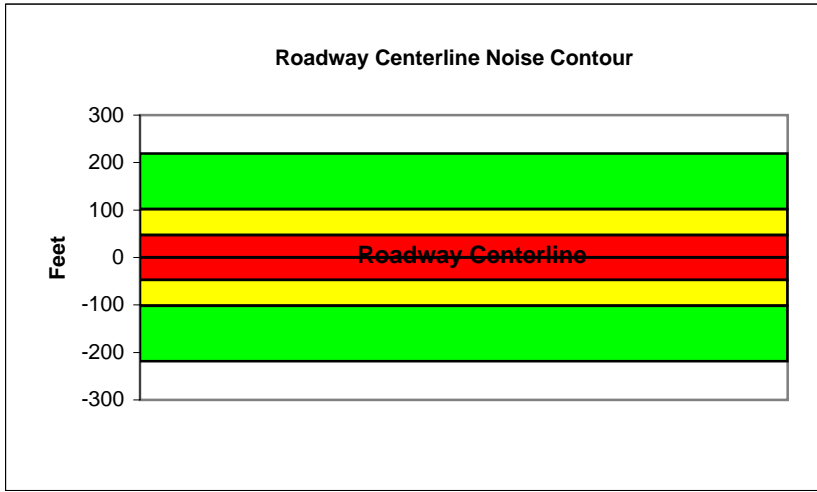
Project Name: Marymount College Facilities Expansion Project Scenario: Existing Plus Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between Ninth Street and West 25th Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	25726			
Receiver Barrier Dist:	0	Peak Hour Traffic:	2572.6			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	43			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	52.4	61.2	59.4	53.3	62.0	62.6
Medium Trucks:	61.3	53.3	46.9	45.3	53.8	54.0
Heavy Trucks:	66.2	54.3	45.2	46.4	56.1	56.3
<b>Vehicle Noise:</b>	<b>68.6</b>	<b>62.8</b>	<b>59.8</b>	<b>54.9</b>	<b>63.5</b>	<b>63.9</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	219
65 dBA	102
70 dBA	47
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

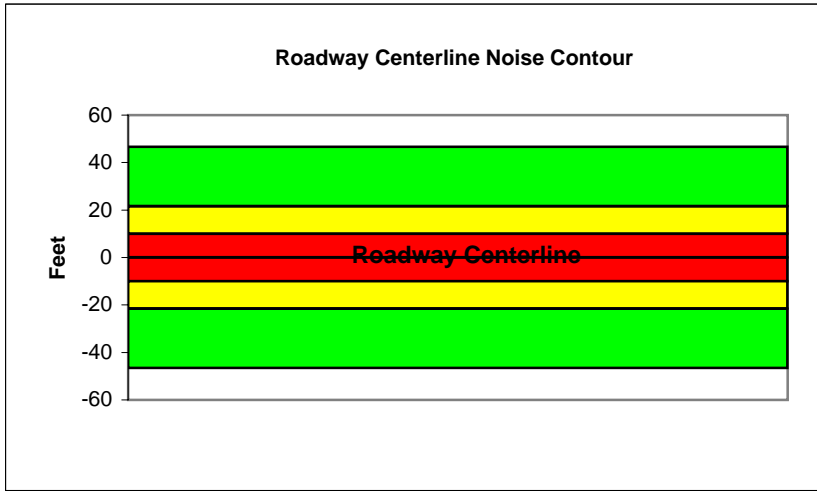
Project Name: Marymount College Facilities Expansion Project Scenario: Existing Plus Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: First Street  
 Road Segment: Between Miraleste Drive and Western Avenue

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	4794			
Receiver Barrier Dist:	0	Peak Hour Traffic:	479.4			
Centerline Dist. To Observer:	100	Vehicle Speed:	30			
Barrier Near Lane CL Dist:	0	Centerline Separation:	14			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	42.2	51.0	49.2	43.1	51.8	52.4
Medium Trucks:	52.8	44.8	38.4	36.8	45.3	45.5
Heavy Trucks:	58.5	46.5	37.5	38.7	48.8	49.0
<b>Vehicle Noise:</b>	<b>61.0</b>	<b>53.5</b>	<b>50.0</b>	<b>45.6</b>	<b>54.2</b>	<b>54.6</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	47
65 dBA	22
70 dBA	10
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

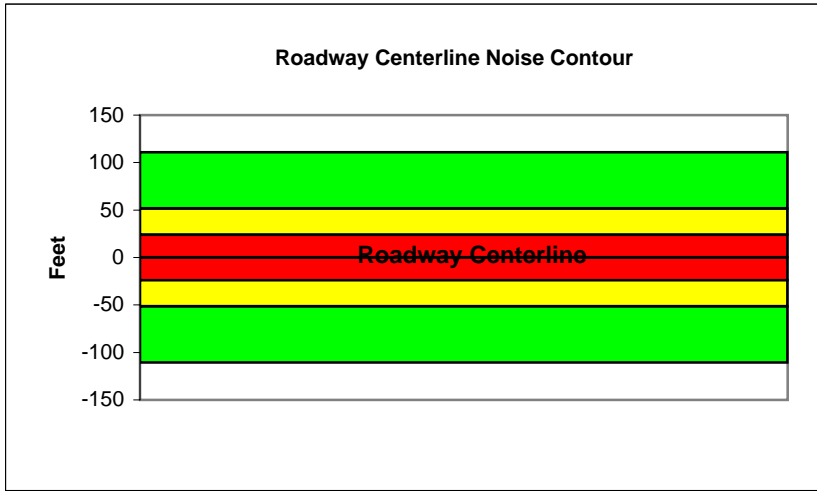
Project Name: Marymount College Facilities Expansion Project Scenario: Existing Plus Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Palos Verdes Drive South  
 Road Segment: Between Palos Verdes Drive East and Western Avenue

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	12622			
Receiver Barrier Dist:	0	Peak Hour Traffic:	1262.2			
Centerline Dist. To Observer:	100	Vehicle Speed:	35			
Barrier Near Lane CL Dist:	0	Centerline Separation:	32			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	47.9	56.7	54.9	48.8	57.5	58.1
Medium Trucks:	57.6	49.5	43.2	41.6	50.1	50.3
Heavy Trucks:	62.8	50.9	41.8	43.1	53.0	53.1
<b>Vehicle Noise:</b>	<b>65.3</b>	<b>58.6</b>	<b>55.4</b>	<b>50.8</b>	<b>59.3</b>	<b>59.8</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	111
65 dBA	51
70 dBA	24
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

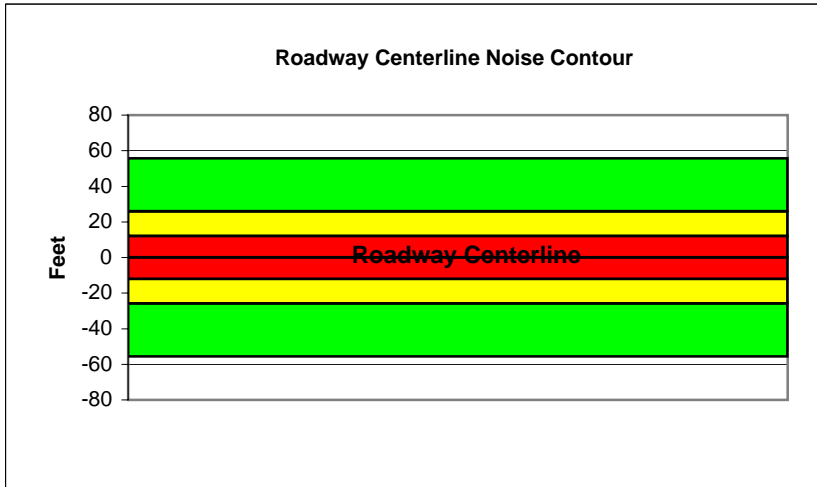
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 Without Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Palos Verdes Drive East  
 Road Segment: Between Via Colinita And Crest Drive

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	6282			
Receiver Barrier Dist:	0	Peak Hour Traffic:	628.2			
Centerline Dist. To Observer:	100	Vehicle Speed:	30			
Barrier Near Lane CL Dist:	0	Centerline Separation:	12.5			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	43.5	52.2	50.4	44.4	53.0	53.6
Medium Trucks:	54.1	46.0	39.6	38.0	46.5	46.8
Heavy Trucks:	59.7	47.8	38.7	39.9	50.1	50.2
<b>Vehicle Noise:</b>	<b>62.2</b>	<b>54.7</b>	<b>51.2</b>	<b>46.8</b>	<b>55.4</b>	<b>55.8</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	56
65 dBA	26
70 dBA	12
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

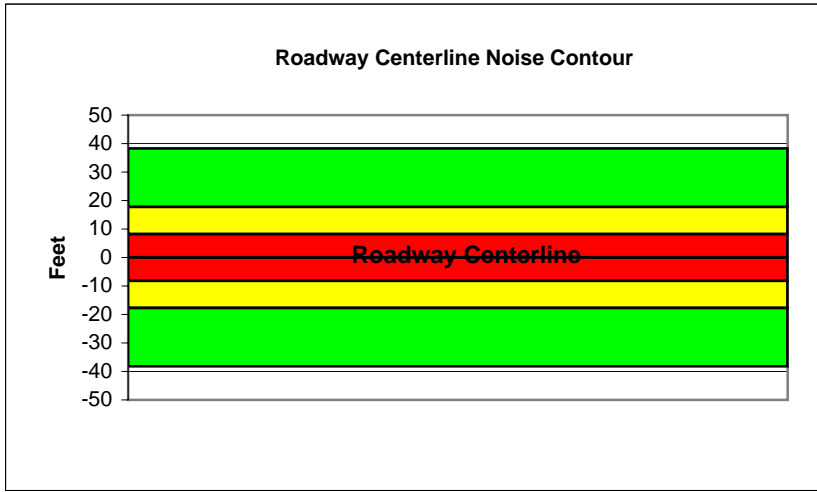
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 Without Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Palos Verdes Drive East  
 Road Segment: Between Crest Drive and Palos Verdes Drive South

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	3574			
Receiver Barrier Dist:	0	Peak Hour Traffic:	357.4			
Centerline Dist. To Observer:	100	Vehicle Speed:	30			
Barrier Near Lane CL Dist:	0	Centerline Separation:	12.5			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	41.0	49.8	48.0	41.9	50.6	51.2
Medium Trucks:	51.6	43.5	37.2	35.6	44.1	44.3
Heavy Trucks:	57.3	45.3	36.3	37.5	47.6	47.7
<b>Vehicle Noise:</b>	<b>59.8</b>	<b>52.3</b>	<b>48.7</b>	<b>44.4</b>	<b>52.9</b>	<b>53.4</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	38
65 dBA	18
70 dBA	8
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

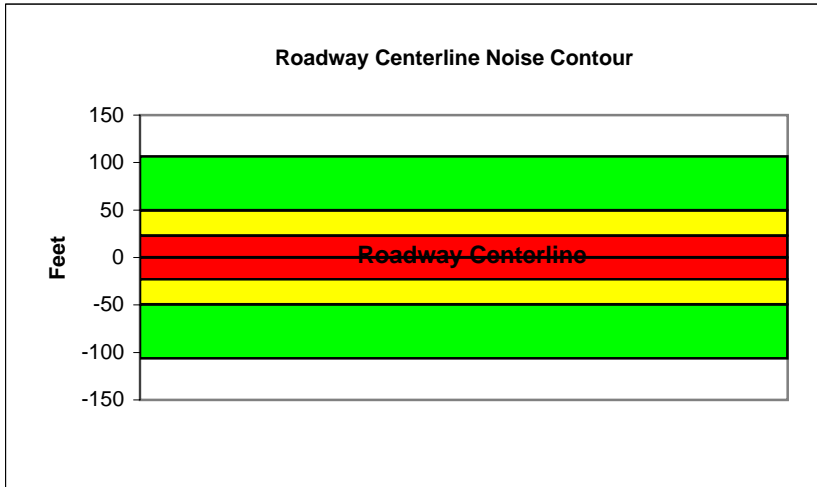
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 Without Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Miraleste Drive  
 Road Segment: Between Palos Verdes Drive East and Via Colinita

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	11887			
Receiver Barrier Dist:	0	Peak Hour Traffic:	1188.7			
Centerline Dist. To Observer:	100	Vehicle Speed:	35			
Barrier Near Lane CL Dist:	0	Centerline Separation:	94			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	46.4	55.2	53.4	47.3	55.9	56.5
Medium Trucks:	56.1	48.0	41.6	40.1	48.6	48.8
Heavy Trucks:	61.3	49.4	40.3	41.5	51.4	51.6
<b>Vehicle Noise:</b>	<b>63.8</b>	<b>57.1</b>	<b>53.9</b>	<b>49.2</b>	<b>57.8</b>	<b>58.3</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	107
65 dBA	50
70 dBA	23
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

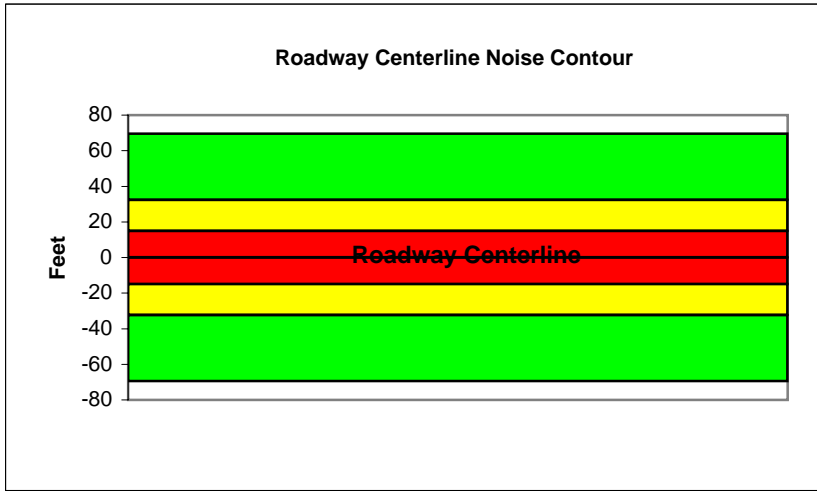
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 Without Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Miraleste Drive  
 Road Segment: Between Via Colinita and First Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	6272			
Receiver Barrier Dist:	0	Peak Hour Traffic:	627.2			
Centerline Dist. To Observer:	100	Vehicle Speed:	35			
Barrier Near Lane CL Dist:	0	Centerline Separation:	94			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	43.6	52.4	50.6	44.5	53.2	53.8
Medium Trucks:	53.3	45.3	38.9	37.3	45.8	46.0
Heavy Trucks:	58.5	46.6	37.5	38.8	48.7	48.8
<b>Vehicle Noise:</b>	<b>61.0</b>	<b>54.3</b>	<b>51.2</b>	<b>46.5</b>	<b>55.0</b>	<b>55.5</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	70
65 dBA	32
70 dBA	15
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

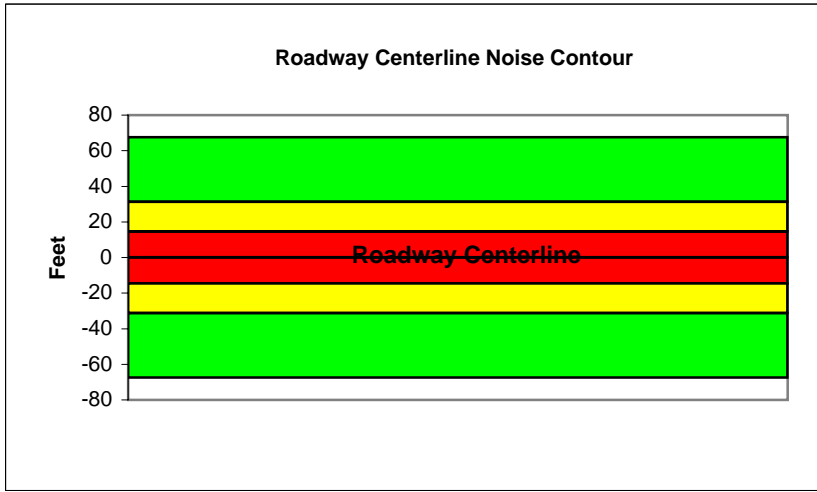
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 Without Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Miraleste Drive  
 Road Segment: Between First Street and Western Avenue

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	5994			
Receiver Barrier Dist:	0	Peak Hour Traffic:	599.4			
Centerline Dist. To Observer:	100	Vehicle Speed:	35			
Barrier Near Lane CL Dist:	0	Centerline Separation:	28			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	44.8	53.5	51.8	45.7	54.3	54.9
Medium Trucks:	54.5	46.4	40.0	38.5	46.9	47.2
Heavy Trucks:	59.7	47.8	38.7	39.9	49.8	50.0
<b>Vehicle Noise:</b>	<b>62.1</b>	<b>55.5</b>	<b>52.3</b>	<b>47.6</b>	<b>56.2</b>	<b>56.6</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	68
65 dBA	31
70 dBA	15
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

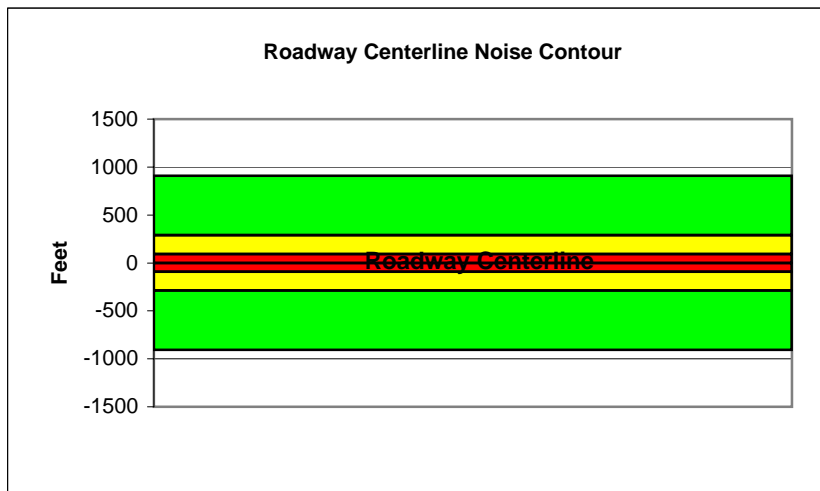
Project Name: Marymount College Facilities Expansion Project      Scenario: 2012 Without Project  
 Analyst: Michelle Dunn      Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between Toscanini Drive and Capitol Drive

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	40,650			
Receiver Barrier Dist:	0	Peak Hour Traffic:	4065			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	48			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions <b>HARD SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	0	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.695	0.129	0.096	0.92
Rt View:      90      Lft View:      -90		Med. Truck	0.0144	0.0006	0.015	0.03
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.24	0.001	0.025	0.05
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	57.4	65.5	64.1	58.1	66.5	67.1
Medium Trucks:	66.3	42.7	34.9	44.1	50.2	50.3
Heavy Trucks:	71.2	62.0	44.2	53.4	63.4	63.4
<b>Vehicle Noise:</b>	<b>73.6</b>	<b>67.5</b>	<b>64.2</b>	<b>59.8</b>	<b>68.3</b>	<b>68.7</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	910
65 dBA	288
70 dBA	91
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

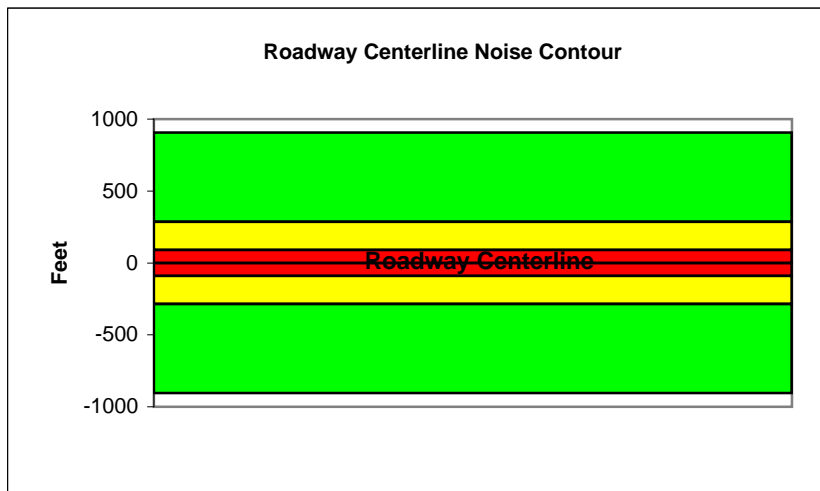
Project Name: Marymount College Facilities Expansion Project      Scenario: 2012 Without Project  
Analyst: Michelle Dunn      Job #: 10-104089  
Roadway: Western Avenue  
Road Segment: Between Capitol Drive and Crestwood Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	40,529			
Receiver Barrier Dist:	0	Peak Hour Traffic:	4052.9			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	48			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions <b>HARD SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	0	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.695	0.129	0.096	0.92
Rt View:      90      Lft View:      -90		Med. Truck	0.0144	0.0006	0.015	0.03
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.24	0.001	0.025	0.05
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	57.4	65.4	64.1	58.0	66.5	67.1
Medium Trucks:	66.3	42.7	34.9	44.1	50.2	50.3
Heavy Trucks:	71.2	62.0	44.2	53.3	63.4	63.4
<b>Vehicle Noise:</b>	<b>73.6</b>	<b>67.5</b>	<b>64.2</b>	<b>59.8</b>	<b>68.3</b>	<b>68.7</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	906
65 dBA	287
70 dBA	91
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

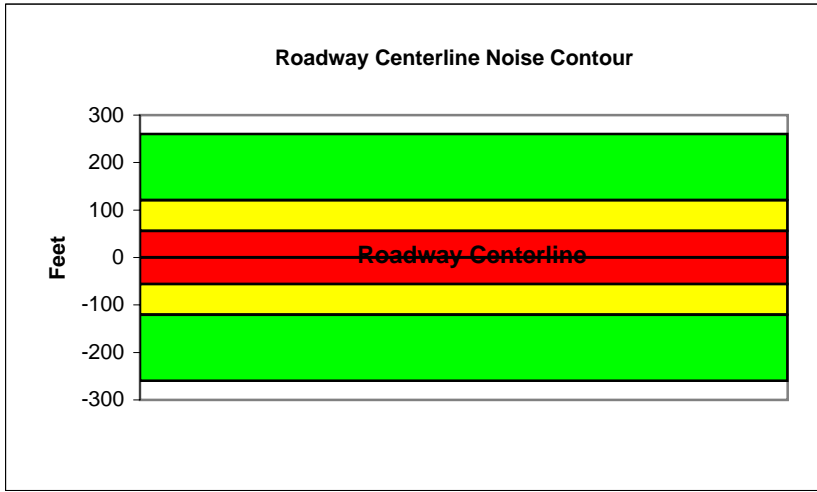
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 Without Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between Crestwood Street and First Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	33382			
Receiver Barrier Dist:	0	Peak Hour Traffic:	3338.2			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	48			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	53.4	62.2	60.4	54.3	63.0	63.6
Medium Trucks:	62.4	54.3	47.9	46.3	54.8	55.1
Heavy Trucks:	67.2	55.3	46.2	47.4	57.2	57.3
<b>Vehicle Noise:</b>	<b>69.6</b>	<b>63.8</b>	<b>60.9</b>	<b>55.9</b>	<b>64.5</b>	<b>65.0</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	261
65 dBA	121
70 dBA	56
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

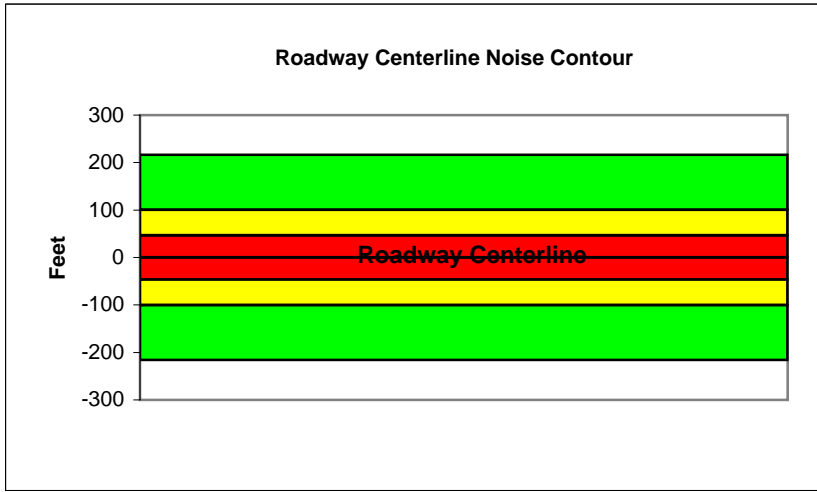
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 Without Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between First Street and Ninth Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	25320			
Receiver Barrier Dist:	0	Peak Hour Traffic:	2532			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	48			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	52.2	61.0	59.2	53.1	61.8	62.4
Medium Trucks:	61.2	53.1	46.7	45.1	53.6	53.9
Heavy Trucks:	66.0	54.1	45.0	46.2	56.0	56.1
<b>Vehicle Noise:</b>	<b>68.4</b>	<b>62.6</b>	<b>59.7</b>	<b>54.7</b>	<b>63.3</b>	<b>63.8</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	217
65 dBA	101
70 dBA	47
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

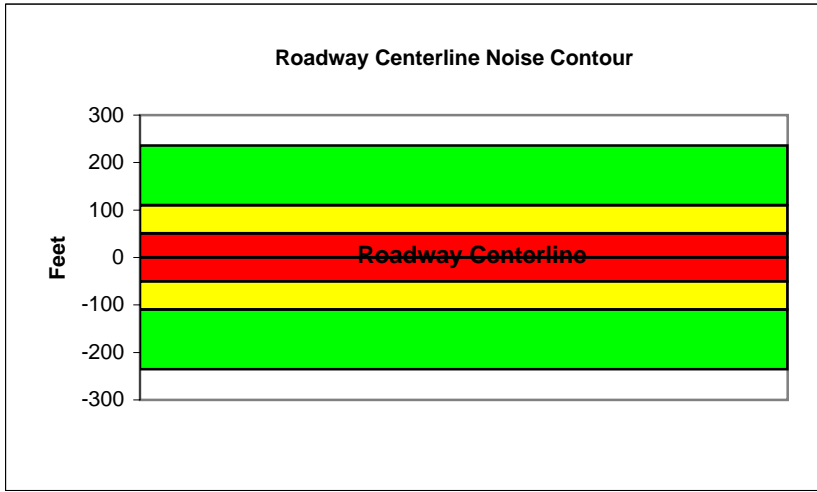
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 Without Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between Ninth Street and West 25th Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	28784			
Receiver Barrier Dist:	0	Peak Hour Traffic:	2878.4			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	43			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	52.9	61.7	59.9	53.8	62.4	63.0
Medium Trucks:	61.8	53.8	47.4	45.8	54.3	54.5
Heavy Trucks:	66.7	54.7	45.7	46.9	56.6	56.7
<b>Vehicle Noise:</b>	<b>69.1</b>	<b>63.2</b>	<b>60.3</b>	<b>55.4</b>	<b>63.9</b>	<b>64.4</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	236
65 dBA	110
70 dBA	51
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

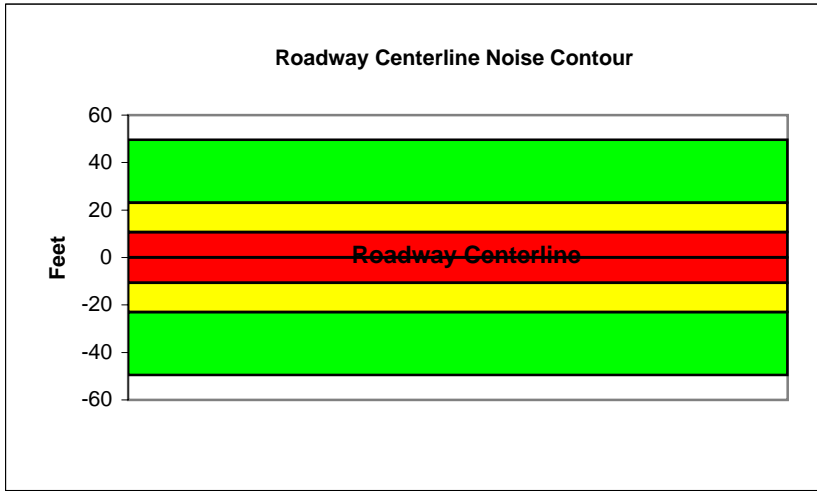
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 Without Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: First Street  
 Road Segment: Between Miraleste Drive and Western Avenue

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	5262			
Receiver Barrier Dist:	0	Peak Hour Traffic:	526.2			
Centerline Dist. To Observer:	100	Vehicle Speed:	30			
Barrier Near Lane CL Dist:	0	Centerline Separation:	14			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	42.6	51.4	49.6	43.6	52.2	52.8
Medium Trucks:	53.2	45.2	38.8	37.2	45.7	45.9
Heavy Trucks:	58.9	47.0	37.9	39.1	49.2	49.4
<b>Vehicle Noise:</b>	<b>61.4</b>	<b>53.9</b>	<b>50.4</b>	<b>46.0</b>	<b>54.6</b>	<b>55.0</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	50
65 dBA	23
70 dBA	11
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

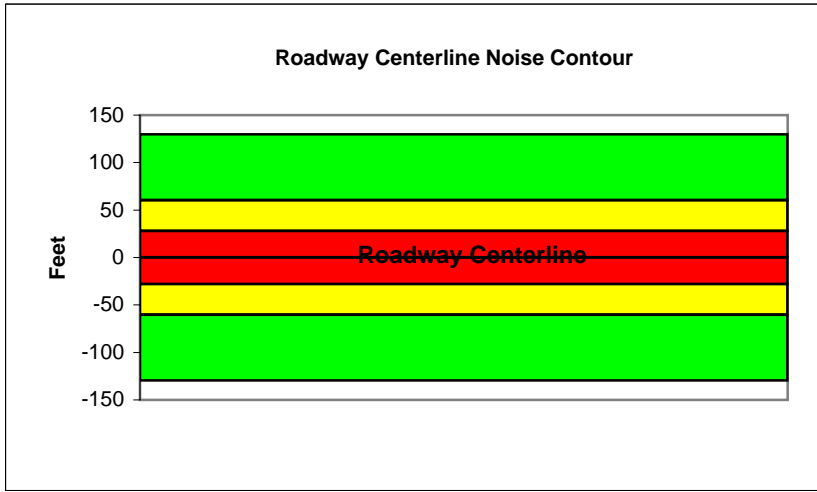
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 Without Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Palos Verdes Drive South  
 Road Segment: Between Palos Verdes Drive East and Western Avenue

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	15998			
Receiver Barrier Dist:	0	Peak Hour Traffic:	1599.8			
Centerline Dist. To Observer:	100	Vehicle Speed:	35			
Barrier Near Lane CL Dist:	0	Centerline Separation:	32			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	48.9	57.7	55.9	49.8	58.5	59.1
Medium Trucks:	58.6	50.6	44.2	42.6	51.1	51.3
Heavy Trucks:	63.9	51.9	42.9	44.1	54.0	54.1
<b>Vehicle Noise:</b>	<b>66.3</b>	<b>59.7</b>	<b>56.5</b>	<b>51.8</b>	<b>60.4</b>	<b>60.8</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	130
65 dBA	60
70 dBA	28
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

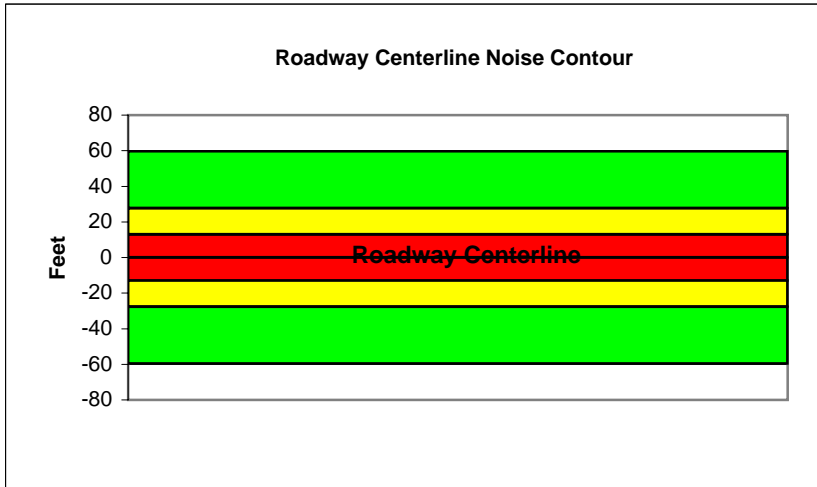
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 With Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Palos Verdes Drive East  
 Road Segment: Between Via Colinita And Crest Drive

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	6948			
Receiver Barrier Dist:	0	Peak Hour Traffic:	694.8			
Centerline Dist. To Observer:	100	Vehicle Speed:	30			
Barrier Near Lane CL Dist:	0	Centerline Separation:	12.5			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	43.9	52.7	50.9	44.8	53.5	54.1
Medium Trucks:	54.5	46.4	40.0	38.5	47.0	47.2
Heavy Trucks:	60.1	48.2	39.1	40.4	50.5	50.6
<b>Vehicle Noise:</b>	<b>62.7</b>	<b>55.2</b>	<b>51.6</b>	<b>47.3</b>	<b>55.8</b>	<b>56.3</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	60
65 dBA	28
70 dBA	13
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

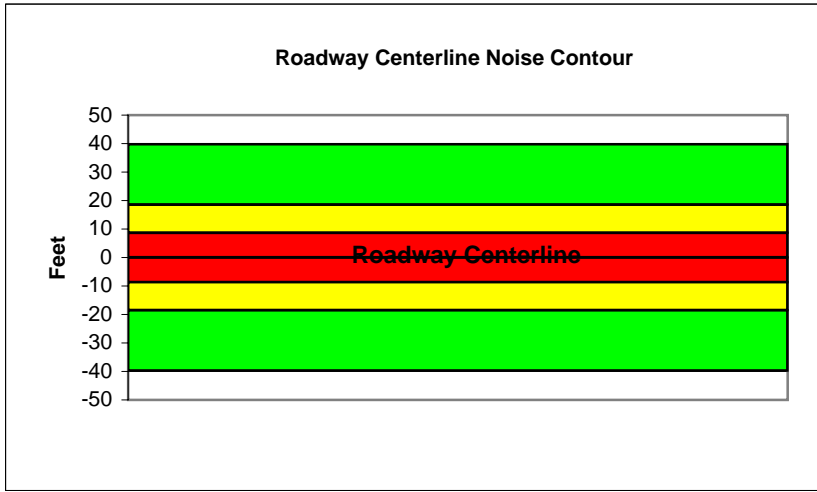
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 With Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Palos Verdes Drive East  
 Road Segment: Between Crest Drive and Palos Verdes Drive South

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	3796			
Receiver Barrier Dist:	0	Peak Hour Traffic:	379.6			
Centerline Dist. To Observer:	100	Vehicle Speed:	30			
Barrier Near Lane CL Dist:	0	Centerline Separation:	12.5			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	41.3	50.0	48.3	42.2	50.8	51.4
Medium Trucks:	51.9	43.8	37.4	35.8	44.3	44.6
Heavy Trucks:	57.5	45.6	36.5	37.7	47.9	48.0
<b>Vehicle Noise:</b>	<b>60.0</b>	<b>52.5</b>	<b>49.0</b>	<b>44.7</b>	<b>53.2</b>	<b>53.6</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	40
65 dBA	19
70 dBA	9
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

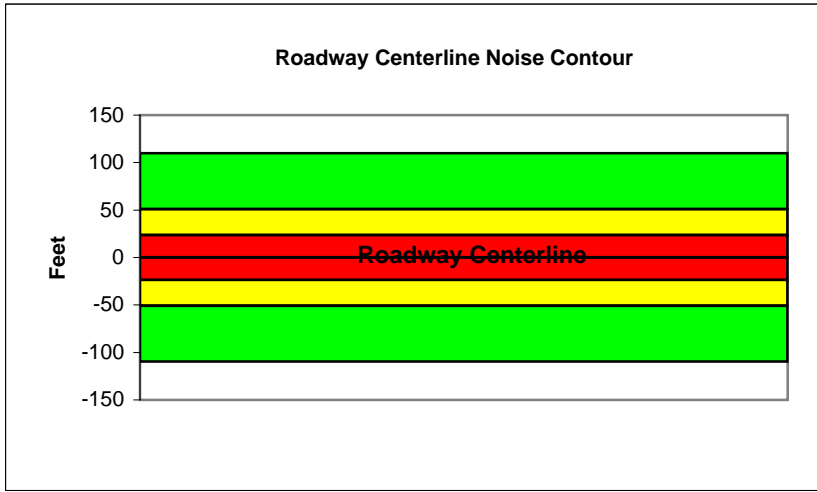
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 With Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Miraleste Drive  
 Road Segment: Between Palos Verdes Drive East and Via Colinita

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	12464			
Receiver Barrier Dist:	0	Peak Hour Traffic:	1246.4			
Centerline Dist. To Observer:	100	Vehicle Speed:	35			
Barrier Near Lane CL Dist:	0	Centerline Separation:	94			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	46.6	55.4	53.6	47.5	56.1	56.7
Medium Trucks:	56.3	48.2	41.9	40.3	48.8	49.0
Heavy Trucks:	61.5	49.6	40.5	41.7	51.7	51.8
<b>Vehicle Noise:</b>	<b>64.0</b>	<b>57.3</b>	<b>54.1</b>	<b>49.4</b>	<b>58.0</b>	<b>58.5</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	110
65 dBA	51
70 dBA	24
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

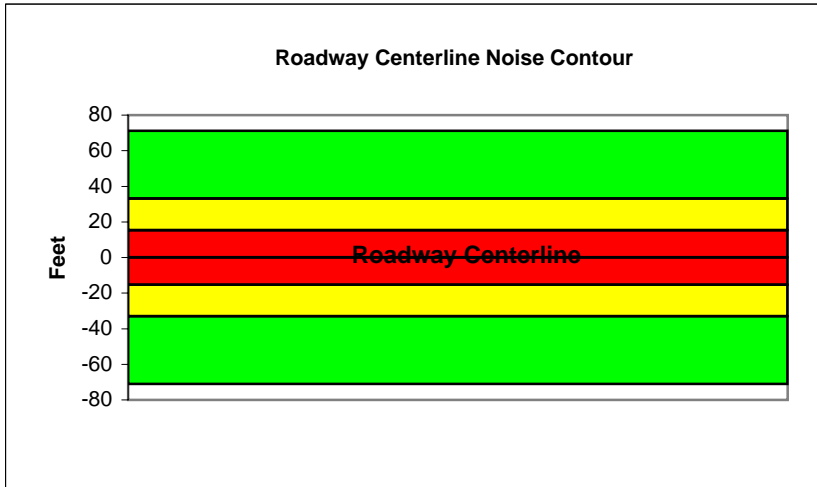
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 With Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Miraleste Drive  
 Road Segment: Between Via Colinita and First Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	6494			
Receiver Barrier Dist:	0	Peak Hour Traffic:	649.4			
Centerline Dist. To Observer:	100	Vehicle Speed:	35			
Barrier Near Lane CL Dist:	0	Centerline Separation:	94			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	43.8	52.5	50.7	44.7	53.3	53.9
Medium Trucks:	53.5	45.4	39.0	37.4	45.9	46.2
Heavy Trucks:	58.7	46.8	37.7	38.9	48.8	48.9
<b>Vehicle Noise:</b>	<b>61.1</b>	<b>54.5</b>	<b>51.3</b>	<b>46.6</b>	<b>55.2</b>	<b>55.6</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	71
65 dBA	33
70 dBA	15
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

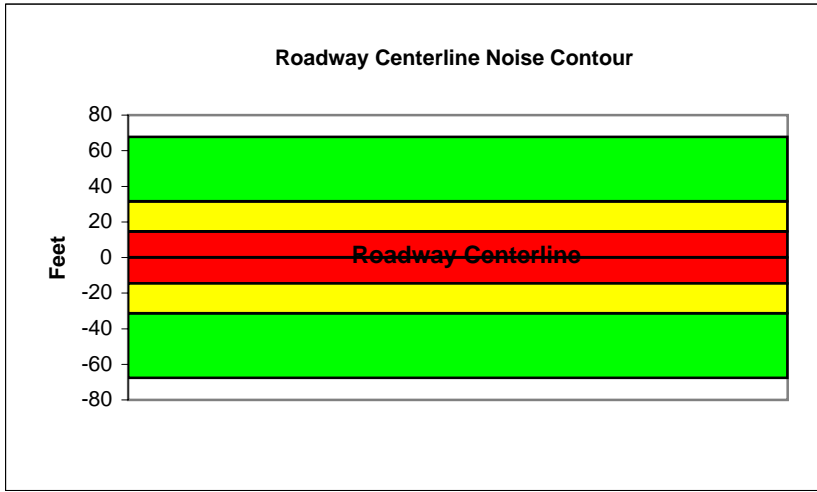
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 With Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Miraleste Drive  
 Road Segment: Between First Street and Western Avenue

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	6038			
Receiver Barrier Dist:	0	Peak Hour Traffic:	603.8			
Centerline Dist. To Observer:	100	Vehicle Speed:	35			
Barrier Near Lane CL Dist:	0	Centerline Separation:	28			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	44.8	53.6	51.8	45.7	54.3	55.0
Medium Trucks:	54.5	46.4	40.1	38.5	47.0	47.2
Heavy Trucks:	59.7	47.8	38.7	40.0	49.9	50.0
<b>Vehicle Noise:</b>	<b>62.2</b>	<b>55.5</b>	<b>52.3</b>	<b>47.7</b>	<b>56.2</b>	<b>56.7</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	68
65 dBA	31
70 dBA	15
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

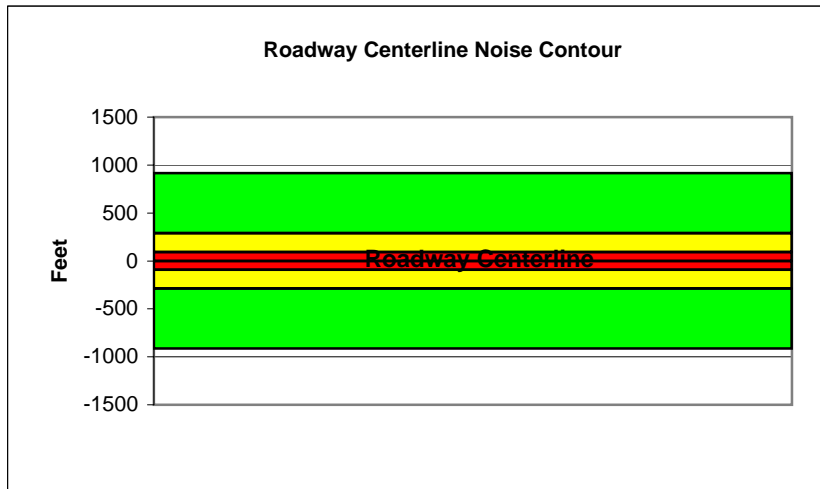
Project Name: Marymount College Facilities Expansion Project      Scenario: 2012 With Project  
Analyst: Michelle Dunn      Job #: 10-104089  
Roadway: Western Avenue  
Road Segment: Between Toscanini Drive and Capitol Drive

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	41,005			
Receiver Barrier Dist:	0	Peak Hour Traffic:	4100.5			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	48			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions <b>HARD SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	0	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.695	0.129	0.096	0.92
Rt View:      90      Lft View:      -90		Med. Truck	0.0144	0.0006	0.015	0.03
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.24	0.001	0.025	0.05
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	57.4	65.5	64.2	58.1	66.6	67.2
Medium Trucks:	66.4	42.7	34.9	44.1	50.3	50.3
Heavy Trucks:	71.2	62.0	44.2	53.4	63.4	63.5
<b>Vehicle Noise:</b>	<b>73.6</b>	<b>67.6</b>	<b>64.2</b>	<b>59.9</b>	<b>68.3</b>	<b>68.8</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	917
65 dBA	290
70 dBA	92
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

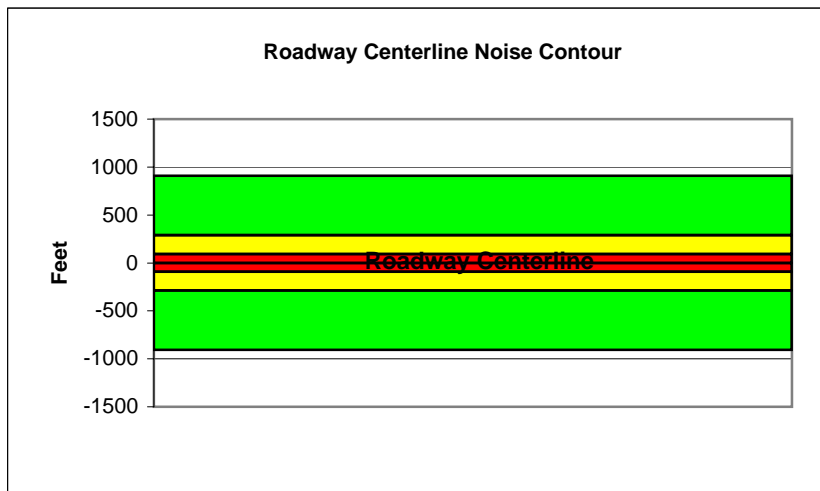
Project Name: Marymount College Facilities Expansion Project      Scenario: 2012 With Project  
Analyst: Michelle Dunn      Job #: 10-104089  
Roadway: Western Avenue  
Road Segment: Between Capitol Drive and Crestwood Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	40,662			
Receiver Barrier Dist:	0	Peak Hour Traffic:	4066.2			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	48			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions <b>HARD SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	0	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.695	0.129	0.096	0.92
Rt View:      90      Lft View:      -90		Med. Truck	0.0144	0.0006	0.015	0.03
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.24	0.001	0.025	0.05
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	57.4	65.5	64.1	58.1	66.5	67.1
Medium Trucks:	66.3	42.7	34.9	44.1	50.2	50.3
Heavy Trucks:	71.2	62.0	44.2	53.4	63.4	63.4
<b>Vehicle Noise:</b>	<b>73.6</b>	<b>67.5</b>	<b>64.2</b>	<b>59.8</b>	<b>68.3</b>	<b>68.7</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	910
65 dBA	288
70 dBA	91
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

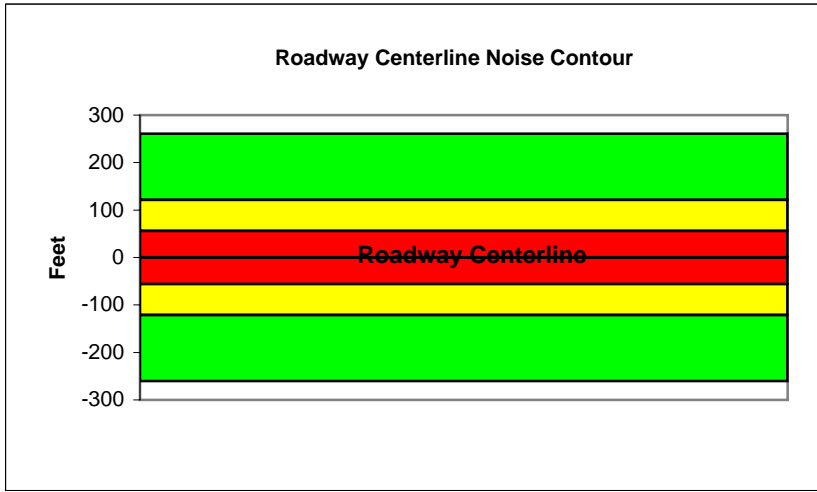
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 With Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between Crestwood Street and First Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	33515			
Receiver Barrier Dist:	0	Peak Hour Traffic:	3351.5			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	48			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	53.4	62.2	60.4	54.3	63.0	63.6
Medium Trucks:	62.4	54.3	47.9	46.4	54.8	55.1
Heavy Trucks:	67.2	55.3	46.2	47.5	57.2	57.3
<b>Vehicle Noise:</b>	<b>69.6</b>	<b>63.8</b>	<b>60.9</b>	<b>55.9</b>	<b>64.5</b>	<b>65.0</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	261
65 dBA	121
70 dBA	56
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

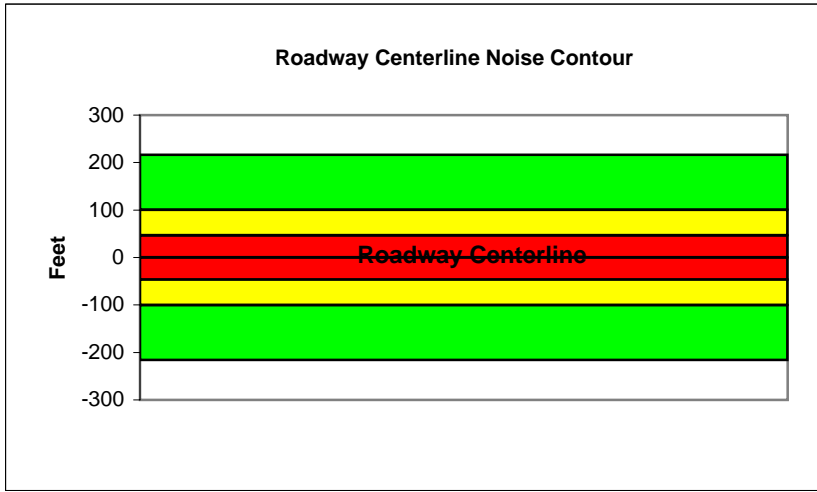
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 With Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between First Street and Ninth Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	25320			
Receiver Barrier Dist:	0	Peak Hour Traffic:	2532			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	48			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	52.2	61.0	59.2	53.1	61.8	62.4
Medium Trucks:	61.2	53.1	46.7	45.1	53.6	53.9
Heavy Trucks:	66.0	54.1	45.0	46.2	56.0	56.1
<b>Vehicle Noise:</b>	<b>68.4</b>	<b>62.6</b>	<b>59.7</b>	<b>54.7</b>	<b>63.3</b>	<b>63.8</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	217
65 dBA	101
70 dBA	47
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

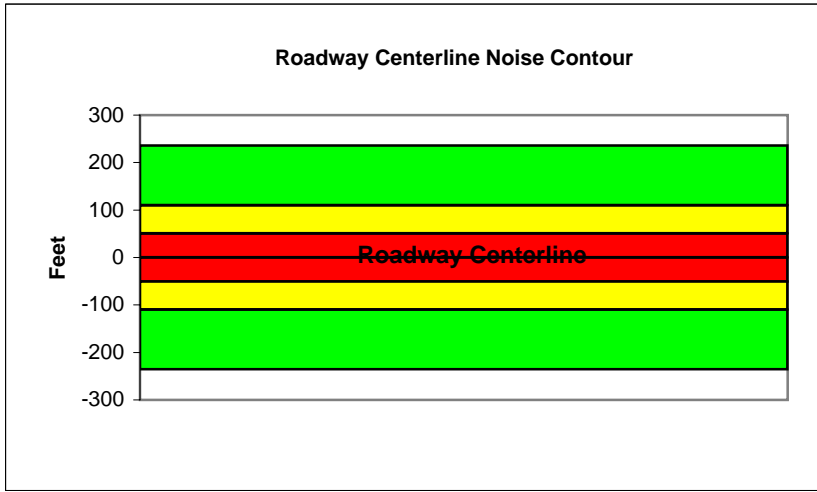
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 With Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Western Avenue  
 Road Segment: Between Ninth Street and West 25th Street

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	28784			
Receiver Barrier Dist:	0	Peak Hour Traffic:	2878.4			
Centerline Dist. To Observer:	100	Vehicle Speed:	40			
Barrier Near Lane CL Dist:	0	Centerline Separation:	43			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	52.9	61.7	59.9	53.8	62.4	63.0
Medium Trucks:	61.8	53.8	47.4	45.8	54.3	54.5
Heavy Trucks:	66.7	54.7	45.7	46.9	56.6	56.7
<b>Vehicle Noise:</b>	<b>69.1</b>	<b>63.2</b>	<b>60.3</b>	<b>55.4</b>	<b>63.9</b>	<b>64.4</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	236
65 dBA	110
70 dBA	51
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

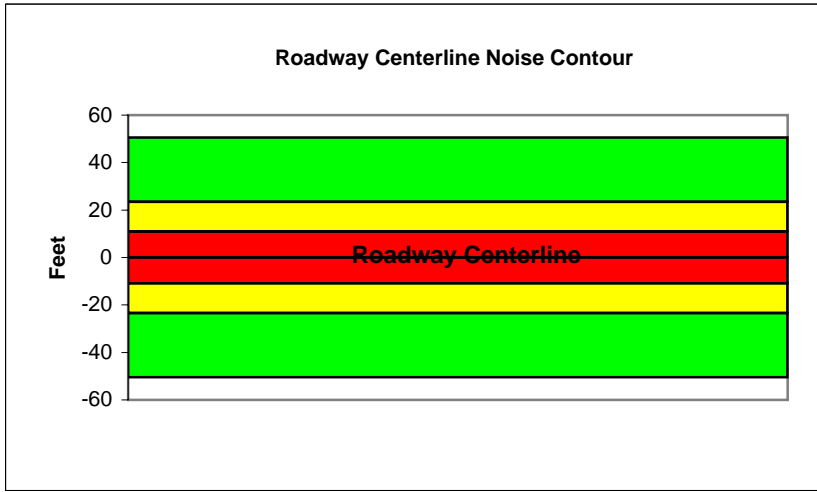
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 With Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: First Street  
 Road Segment: Between Miraleste Drive and Western Avenue

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	5440			
Receiver Barrier Dist:	0	Peak Hour Traffic:	544			
Centerline Dist. To Observer:	100	Vehicle Speed:	30			
Barrier Near Lane CL Dist:	0	Centerline Separation:	14			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	42.8	51.6	49.8	43.7	52.3	53.0
Medium Trucks:	53.4	45.3	38.9	37.4	45.9	46.1
Heavy Trucks:	59.0	47.1	38.0	39.3	49.4	49.5
<b>Vehicle Noise:</b>	<b>61.6</b>	<b>54.0</b>	<b>50.5</b>	<b>46.2</b>	<b>54.7</b>	<b>55.1</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	51
65 dBA	23
70 dBA	11
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Federal Highway Administration RD-77-108  
Traffic Noise Prediction Model (CALVENO)**

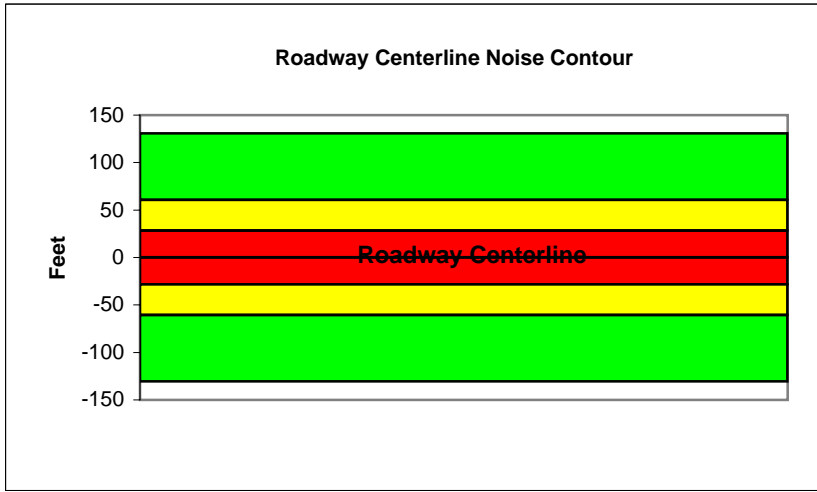
Project Name: Marymount College Facilities Expansion Project Scenario: 2012 With Project  
 Analyst: Michelle Dunn Job #: 10-104089  
 Roadway: Palos Verdes Drive South  
 Road Segment: Between Palos Verdes Drive East and Western Avenue

PROJECT DATA		SITE DATA				
Centerline Dist to Barrier:	0	Road Grade:	0			
Barrier (0=wall, 1= berm):	0	Average Daily Traffic:	16176			
Receiver Barrier Dist:	0	Peak Hour Traffic:	1617.6			
Centerline Dist. To Observer:	100	Vehicle Speed:	35			
Barrier Near Lane CL Dist:	0	Centerline Separation:	32			
Barrier Far lane CL Dist:	0	<b>NOISE INPUTS</b>				
Pad Elevation:	0.5	Site conditions: <b>SOFT SITE</b>				
Road Elevation:	0	<b>FLEET MIX</b>				
Observer Height (above grade):	5.5	Type	Day	Evening	Night	Daily
Barrier Height:	0	Auto	0.775	0.129	0.096	0.9742
Rt View: 90 Lft View: -90		Med. Truck	0.848	0.049	0.103	0.0184
<b>NOISE SOURCE ELEVATIONS (Feet)</b>		Heavy Truck	0.865	0.027	0.108	0.0074
Autos:	0					
Medium Trucks:	2.3					
Heavy Trucks:	8					

UNMITIGATED NOISE LEVELS (No topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:	49.0	57.7	56.0	49.9	58.5	59.1
Medium Trucks:	58.7	50.6	44.2	42.7	51.2	51.4
Heavy Trucks:	63.9	52.0	42.9	44.1	54.0	54.2
<b>Vehicle Noise:</b>	<b>66.3</b>	<b>59.7</b>	<b>56.5</b>	<b>51.8</b>	<b>60.4</b>	<b>60.9</b>

MITIGATED NOISE LEVELS (With topographic or barrier attenuation)						
Vehicle Type	Peak Leq	Leq Day	Leq Evening	Leq Night	Ldn	CNEL
Autos:						
Medium Trucks:						
Heavy Trucks:						
Vehicle Noise:						

CENTERLINE NOISE CONTOUR	
Unmitigated	
60 dBA	131
65 dBA	61
70 dBA	28
Mitigated	
60 dBA	
65 dBA	
70 dBA	



**Appendix D-4**  
**Mira Vista Neighborhood**  
**Residential Street Segment Review**

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## MEMORANDUM

To: Ara Mihranian, AICP JN 10104089

From: Paul Martin – RBF Consulting

Date: January 19, 2010

Subject: Mira Vista Neighborhood Residential Street Segment Review

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This technical memorandum updates the *Marymount College – Traffic Count Data Analysis Memorandum*, dated May 18, 2008, prepared by Joanne Itagaki, City of Rancho Palos Verdes Consultant Traffic Engineer, and the *Mira Vista Neighborhood Residential Street Segment Review (RBF Consulting, May 19, 2009)*, to account for the following modifications to the Marymount College project:

- Designation of up to 250 students from an Associates of Arts degree program (AA Program) to a Bachelor of Arts degree program (BA Program).

For purposes of this analysis, the AA Program and BA Program are defined as follows:

- **AA Program** – For traffic purposes defined as educational programs under the Junior/Community College land use, which usually last about two-years, but may vary depending on the student; and
- **BA Program** – For traffic purposes defined as educational programs under the University land use, which usually last about four-years, but may vary depending on the student.

The *Mira Vista Neighborhood Residential Street Segment Review (May 19, 2009)*, included analysis for the following modifications:

- Removal of residence halls;
- Revised number of existing student seats from 578 to 648;
- Reduce net new seats from 131 to 5 seats;
- Reduce new employees/faculty from 12 to 7 staff; and
- Increase weekend student enrollment from 83 to 150 students.

This analysis reviews traffic operation of roadway segment within the Mira Vista neighborhood.

Traffic counts were collected on Tuesday, April 22, 2008, on the following streets:

- **Enrose Avenue between**
  1. Summerland Street and General Street
  2. General Street and Fairhill Drive
  3. Fairhill Drive and Crestwood Street
  4. Crestwood Street and Nobel View Drive
- **General Street between**
  5. Bayend Drive and Bernice Drive (W)
  6. Bayend Drive and Wycliff Avenue
  7. Fairhill Drive and Enrose Avenue
- **Trudie Drive between**
  8. Western Avenue and Highmore Avenue
  9. Homeworth Drive and Bayend Drive
  10. Bayend Drive and Trotwood Avenue
- **Via Colinita between**
  11. Enrose Avenue and Miraleste Drive

The trip distribution utilized in this analysis is consistent with that identified in the *Marymount College Facilities Expansion Project Traffic & Parking Impact Analysis (September 28, 2007)* and is assumed to remain unchanged with project description modifications. As indicated in the EIR, 40 percent of project trips are forecast to travel through the residential neighborhood. When accounting for modifications to the project description, which consists of designation of up to 250 students from an Associates of Arts degree program (AA Program) to a Bachelor of Arts degree program (BA Program), the project is forecast to generate the following number of trips during various periods of the day.

- AM Peak Hour Trips = 200
- Mid-day Peak Hour Trips = 204
- Afternoon Peak Hour Trips = 187
- PM Peak Hour Trips = 175
- Total Daily Trips = 1,931

Tables 1 through 4 identify the increase in hourly traffic volumes on the residential streets in question. The Tables reflect the trip generation of the four peak periods (AM peak, Mid-day peak, Afternoon peak, and PM peak) analyzed in the EIR. The distribution pattern from the EIR indicates that there would be no additional trips on General Street.

The City uses the Los Angeles County Traffic Impact Analysis Report Guidelines dated January 1, 1997 to determine significant traffic impacts. This document has defined a significant traffic impact on two-lane roadway occurs when a project adds the following percentages based on Level of Service (LOS) of the preproject conditions.

<b>TWO-LANE ROADWAYS</b>				
<b>Directional Split</b>	<b>Total Capacity (PCPH)</b>	<b>Percentages Increase in Passenger Car Per Hour (PCPH) by Project</b>		
		<b>Preproject LOS</b>		
		<b>C</b>	<b>D</b>	<b>E/F</b>
50/50	2,800	4	2	1
60/40	2,650	4	2	1
70/30	2,500	4	2	1
80/20	2,300	4	2	1
90/10	2,100	4	2	1
100/0	2,000	4	2	1

**Source:** Los Angeles County Traffic Impact Analysis Report Guidelines, January 1, 1997.

In comparing the County of Los Angeles criteria with Tables 1 to 4, the roadway segments analyzed are operating at an acceptable LOS "A" during all four periods. With the addition of proposed project trips accounting for project description modifications, the roadway segments continue to operate at an acceptable LOS "A".

When accounting for modifications to the project description and associated trip generation, the results of the Mira Vista Neighborhood review remain unchanged. Please contact me with any questions.

#### Attachments

Table 1 – Peak Hour Analysis: AM Peak Hour

Table 2 – Peak Hour Analysis: Mid-day Peak Hour

Table 3 – Peak Hour Analysis: Afternoon Peak Hour

Table 4 – Peak Hour Analysis: PM Peak Hour

**MARYMOUNT COLLEGE**  
**Peak Hour Analysis**  
**Table 1 - AM PEAK HOUR**

AM Peak Hour between 7 AM and 10 AM

AM Peak Hour Total Trips = 200

40% = 80 trips

	Location		AM Peak Hour	Traffic Volume			Preproject		Project Traffic Trips	Total Trips	With Project	
				NB/EB	SB/WB	Total	V/C	LOS			V/C	LOS
Enrose Avenue between												
Traffic Volume Directional	60/40 split	Summerland St. and General St.	8:00	220	143	363	0.14	A	80	443	0.17	A
	60/40 split	General St. and Fairhill Dr.	8:00	74	112	186	0.07	A	80	266	0.10	A
	70/30 split	Fairhill Dr. and Crestwood St.	8:00	121	52	173	0.07	A	80	253	0.10	A
	60/40/split	Crestwood St. and Nobel View Dr.	7:00	100	74	174	0.07	A	80	254	0.10	A
General Street between												
Traffic Volume Directional	60/40 split	Bayend Dr. and Bernice Dr. (W)	8:00	113	148	261	0.10	A	0	261	0.10	A
	60/40 split	Bayend Dr. and Wycliff Ave.	8:00	210	151	361	0.14	A	0	361	0.14	A
	60/40 split	Fairhill Dr. and Enrose Ave.	8:00	101	65	166	0.06	A	0	166	0.06	A
Trudie Drive between												
Traffic Volume Directional	60/40 split	Western Ave. and Highmore Ave.	8:00	381	127	508	0.19	A	80	588	0.22	A
	50/50 split	Homeworth Dr. and Bayend Dr.	8:00	154	162	316	0.11	A	80	396	0.14	A
	60/40 split	Bayend Dr. and Trotwood Ave.	8:00	83	56	139	0.05	A	80	219	0.08	A
Via Colinita between												
	50/50 split	Enrose Ave. and Miraleste Dr.	7:00	190	213	403	0.14	A	80	483	0.17	A

Capacity: pcph = passenger cars per hour

50/50 split = 2,800 pcph

60/40 split = 2,650 pcph

70/30 split = 2,500 pcph

**MARYMOUNT COLLEGE**  
**Peak Hour Analysis**  
**Table 2 - MID-DAY PEAK HOUR**

Mid-Day Peak Hour between 11 AM and 1 PM

Mid-day Peak Hour Total Trips = 204

40% = 82 trips

	Location	Mid-day Hour	Traffic Volume			Preproject		Project Traffic Trips	Total Trips	With Project		
			NB/EB	SB/WB	Total	V/C	LOS			V/C	LOS	
Enrose Avenue between												
Traffic Volume Directional	50/50 split	Summerland St. and General St.	1:00	145	124	269	0.10	A	82	351	0.13	A
	50/50 split	General St. and Fairhill Dr.	1:00	57	67	124	0.04	A	82	206	0.07	A
	60/40 split	Fairhill Dr. and Crestwood St.	1:00	65	48	113	0.04	A	82	195	0.07	A
	50/50 split	Crestwood St. and Nobel View Dr.	1:00	45	53	98	0.04	A	82	180	0.06	A
General Street between												
Traffic Volume Directional	50/50 split	Bayend Dr. and Bernice Dr. (W)	1:00	129	147	276	0.10	A	0	276	0.10	A
	50/50 split	Bayend Dr. and Wycliff Ave.	1:00	158	142	300	0.11	A	0	300	0.11	A
	50/50 split	Fairhill Dr. and Enrose Ave.	1:00	76	76	152	0.05	A	0	152	0.05	A
Trudie Drive between												
Traffic Volume Directional	60/40 split	Western Ave. and Highmore Ave.	1:00	216	124	340	0.13	A	82	422	0.16	A
	50/50 split	Homeworth Dr. and Bayend Dr.	1:00	109	100	209	0.07	A	82	291	0.10	A
	70/30 split	Bayend Dr. and Trotwood Ave.	12:00	59	29	88	0.04	A	82	170	0.07	A
Via Colinita between												
	50/50 split	Enrose Ave. and Miraleste Dr.	1:00	126	135	261	0.09	A	82	343	0.12	A

Capacity: pcph = passenger cars per hour

50/50 split = 2,800 pcph

60/40 split = 2,650 pcph

70/30 split = 2,500 pcph

**MARYMOUNT COLLEGE**  
**Peak Hour Analysis**  
**Table 3 - AFTERNOON PEAK HOUR**

Afternoon Peak Hour between 2 PM and 4 PM

Afternoon Peak Hour Total Trips = 187

40% = 75 trips

	Location	Afternoon Hour	Traffic Volume			Preproject		Project Traffic Trips	Total Trips	With Project		
			NB/EB	SB/WB	Total	V/C	LOS			V/C	LOS	
<b>Enrose Avenue between</b>												
Traffic Volume Directional	60/40 split	Summerland St. and General St.	3:00	223	139	362	0.14	A	75	437	0.16	A
	60/40 split	General St. and Fairhill Dr.	3:00	84	146	230	0.09	A	75	305	0.12	A
	70/30 split	Fairhill Dr. and Crestwood St.	3:00	150	68	218	0.09	A	75	293	0.12	A
	70/30 split	Crestwood St. and Nobel View Dr.	3:00	70	139	209	0.08	A	75	284	0.11	A
<b>General Street between</b>												
Traffic Volume Directional	50/50 split	Bayend Dr. and Bernice Dr. (W)	3:00	114	109	223	0.08	A	0	223	0.08	A
	50/50 split	Bayend Dr. and Wycliff Ave.	3:00	144	115	259	0.09	A	0	259	0.09	A
	50/50 split	Fairhill Dr. and Enrose Ave.	3:00	66	62	128	0.05	A	0	128	0.05	A
<b>Trudie Drive between</b>												
Traffic Volume Directional	70/30 split	Western Ave. and Highmore Ave.	3:00	313	162	475	0.19	A	75	550	0.22	A
	60/40 split	Homeworth Dr. and Bayend Dr.	3:00	126	157	283	0.11	A	75	358	0.14	A
	60/40 split	Bayend Dr. and Trotwood Ave.	3:00	108	62	170	0.06	A	75	245	0.09	A
<b>Via Colinita between</b>												
	60/40 split	Enrose Ave. and Miraleste Dr.	3:00	204	133	337	0.13	A	75	412	0.16	A

Capacity: pcph = passenger cars per hour

50/50 split = 2,800 pcph

60/40 split = 2,650 pcph

70/30 split = 2,500 pcph

**MARYMOUNT COLLEGE**  
**Peak Hour Analysis**  
**Table 4 - PM PEAK HOUR**

PM Peak Hour between 4 PM and 6 PM

PM Peak Hour Total Trips = 175

40% = 70 trips

	Location	PM Peak Hour	Traffic Volume			Preproject		Project Traffic Trips	Total Trips	With Project		
			NB/EB	SB/WB	Total	V/C	LOS			V/C	LOS	
<b>Enrose Avenue between</b>												
Traffic Volume Directional	50/50 split	Summerland St. and General St.	5:00	128	132	260	0.09	A	70	330	0.12	A
	50/50 split	General St. and Fairhill Dr.	5:00	76	72	148	0.05	A	70	218	0.08	A
	50/50 split	Fairhill Dr. and Crestwood St.	5:00	82	67	149	0.05	A	70	219	0.08	A
	50/50 split	Crestwood St. and Nobel View Dr.	5:00	67	78	145	0.05	A	70	215	0.08	A
<b>General Street between</b>												
Traffic Volume Directional	60/40 split	Bayend Dr. and Bernice Dr. (W)	5:00	101	133	234	0.09	A	0	234	0.09	A
	50/50 split	Bayend Dr. and Wycliff Ave.	5:00	107	134	241	0.09	A	0	241	0.09	A
	60/40 split	Fairhill Dr. and Enrose Ave.	5:00	42	58	100	0.04	A	0	100	0.04	A
<b>Trudie Drive between</b>												
Traffic Volume Directional	60/40 split	Western Ave. and Highmore Ave.	5:00	164	235	399	0.15	A	70	469	0.18	A
	70/30 split	Homeworth Dr. and Bayend Dr.	5:00	64	179	243	0.10	A	70	313	0.13	A
	60/40 split	Bayend Dr. and Trotwood Ave.	5:00	59	77	136	0.05	A	70	206	0.08	A
<b>Via Colinita between</b>												
	50/50 split	Enrose Ave. and Miraleste Dr.	5:00	127	132	259	0.09	A	70	329	0.12	A

Capacity: pcph = passenger cars per hour

50/50 split = 2,800 pcph

60/40 split = 2,650 pcph

70/30 split = 2,500 pcph